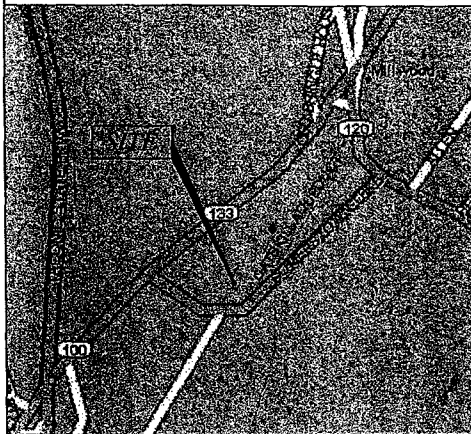


EXHIBIT E

LOCATION MAP

SCALE: 1" = 2000'



As Agent For
OMNIPOINT FACILITIES NETWORK 2, LLC

SITE NUMBER:
NY-09-215-A
SITE NAME:
CON-ED TOWER E-142

PROJECT DESCRIPTION

INSTALLATION OF THREE (3) UNMANNED EQUIPMENT CABINETS AT GRADE & THREE (3) PANEL ANTENNAS ON AN EXISTING CON-ED UTILITY TOWER.

PROJECT SUMMARY

SITE NUMBER: NY-09-215-A
SITE NAME: CON-ED TOWER E-142
SITE ADDRESS: STATION PLACE
TOWN OF NEW CASTLE, NY
COUNTY: WESTCHESTER
PROPERTY OWNER: CONSOLIDATED EDISON
OF NEW YORK, INC.
4 IRVING PLACE
NEW YORK, NY 10003
APPLICANT: OMNIPOINT COMMUNICATIONS, INC.
AS AGENT FOR
OMNIPOINT NETWORK FACILITIES 2, LLC.
76 PROGRESS DRIVE, 2ND FLR
STAMFORD, CT 06902
PHONE: (203) 328-8900

ENGINEER/SURVEYOR/STRUCTURAL ENGR: TECTONIC ENGINEERING
CONSULTANTS P.C.
955 LITTLE BRITAIN ROAD
NEW WINDSOR, NY 12553
CONTACT: EDWARD PIETRASZ
PHONE: (845) 567-6656

ELECTRICAL ENGR: XENON COMPANY
2 KILMER ROAD
EDISON, NJ 08817
CONTACT: ASOK GOSWAMI
PHONE: (732) 339-1400

SITE ACQUISITION: OMNIPOINT WIRELESS
76 PROGRESS DRIVE, 2ND FLR
STAMFORD, CT 06902
CONTACT: ESME LOMBARD
PHONE: (914) 980-2148

RF ENGINEER: OMNIPOINT WIRELESS
76 PROGRESS DRIVE, 2ND FLR
STAMFORD, CT 06902
CONTACT: JONATHAN LINK
PHONE: (201) 757-0477

CONSTRUCTION MGR.: OMNIPOINT WIRELESS
76 PROGRESS DRIVE, 2ND FLR
STAMFORD, CT 06902
CONTACT: RICK FRAZIER
PHONE: (516) 244-9560

CONSOLIDATED EDISON CONTACTS:
EDWARD BUSSE
TRANSMISSION LINE MAINTENANCE
(914) 789-6623
WILLIAM STACHOWIAK, P.E.
CENTRAL ENGINEERING DEPARTMENT
(212) 460-2932
AMIT MUKHOPADHYAY, P.E.
TRANSMISSION ENGINEERING
(212) 460-3685

EQUIPMENT LOCATION: OUTDOOR ☒
INDOOR ☐
ANTENNA LOCATION: GUY TOWER ☐
SELF SUPPORT TWR ☐
EX. LATTICE TOWER ☐
MONOPOLE ☐
ROOFTOP ☐
EX. SILO ☐
EX. UTILITY TOWER ☒

SITE DIRECTIONS

TAKE TACONIC PARKWAY NORTH TO RT 100 EXIT. MAKE LEFT OFF EXIT RAMP ONTO CAMPFIRE ROAD. MAKE RIGHT ONTO RT 100/SAWMILL RIVER ROAD. ACCESS ROAD TO SITE WILL BE ON RIGHT.

SHEET INDEX

SHEET NO	DESCRIPTION	REV NO	REVISION DATE
T-1	TITLE SHEET	+	11/10/03
T-2	GENERAL NOTES	+	11/10/03
T-3	ABBREVIATION LIST & LEGEND	+	11/10/03
B-1	BILL OF MATERIALS	+	11/10/03
B-2	RF CABLE SCHEMATIC	+	11/10/03
C-1	NOTES		
C-2	PARTIAL SITE PLAN	+	11/10/03
C-3	SITE DETAIL PLAN	+	11/10/03
C-4	SITE DETAILS	+	11/10/03
C-5	ELEVATION & ANTENNA PLAN	+	11/10/03
C-6	ANTENNA DETAIL & NOTES	+	11/10/03
C-7	CABINET DETAILS	+	11/10/03
C-8	NOTES & DETAILS	+	11/10/03
S-1	STEEL NOTES	+	11/10/03
S-2	CONCRETE NOTES	+	11/10/03
S-3	EQUIPMENT PLAN & DETAILS	+	11/10/03
E-1	ELECTRICAL NOTES	+	11/10/03
E-2	GROUNDING & ELECTRICAL PLAN	+	11/10/03
E-3	ONE-LINE DIAGRAM	+	11/10/03
E-4	GROUNDING DIAGRAM	+	11/10/03
E-5	GROUNDING DETAILS	+	11/10/03
E-6	GROUNDING DETAILS & NOTES	+	11/10/03

THIS SET OF PLANS SHALL NOT BE UTILIZED AS CONSTRUCTION DOCUMENTS UNTIL ALL ITEMS OF CONCERN HAVE BEEN ADDRESSED AND EACH OF THE DRAWINGS HAS BEEN REVISED AND ISSUED "FOR CONSTRUCTION".

THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF OMNIPOINT FACILITIES NETWORK 2, LLC. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED.

UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT

TECTONIC Engineering & Surveying
Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-6656
Fax: (845) 567-6703

OMNIPOINT COMMUNICATIONS INC

As Agent For
OMNIPOINT FACILITIES NETWORK 2, LLC

OMNIPOINT WIRELESS
76 PROGRESS DRIVE, 2ND FLR
STAMFORD, CT 06902
OFFICE: (203) 328-8900
FAX: (203) 328-8953

APPROVALS

OMNIPOINT
LANDLORD
RF
CONSTRUCTION

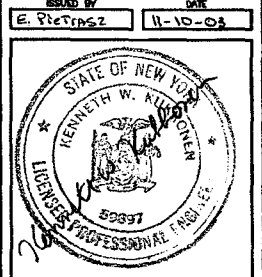
PROJECT NUMBER
235L04215A

DESIGNED BY
EP

REV	DATE	REVISION	DRAWN BY
Δ	4/16/03	FOR COMMENT	JFL
Δ	6/5/03	FOR PERMIT	JFL
Δ	7/24/03	REVISED ANTENNAS	CA
Δ	8/28/03	FOR COMMENT	CC
Δ	11/10/03	FOR CONSTRUCTION	BRK

ISSUED BY
E. Pietrasz

DATE
11-10-03



SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

TITLE SHEET

SHEET NUMBER

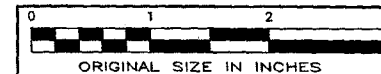
T-1

GENERAL NOTES

1. ALL MATERIAL SHALL BE FURNISHED AND WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE APPLICABLE SECTIONS OF OCI 'STANDARD CONSTRUCTION SPECIFICATIONS'. IN CASE OF A CONFLICT BETWEEN THE CONSTRUCTION SPECIFICATIONS AND THE DRAWINGS, THE DRAWINGS SHALL GOVERN. IN CASE OF A CONFLICT BETWEEN OCI STANDARD PRACTICES AND EITHER THE CONSTRUCTION SPECIFICATIONS OR THE DRAWINGS, THE OCI STANDARD PRACTICE SHALL GOVERN.
2. ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE NEW YORK STATE BUILDING CODE AND ALL OTHER APPLICABLE CODES AND ORDINANCES.
3. CONTRACTOR SHALL VISIT THE JOB SITE AND FAMILIARIZE HIMSELF WITH ALL CONDITIONS AFFECTING THE PROPOSED WORK AND MAKE PROVISIONS AS TO THE COST THEREOF. CONTRACTOR SHALL BE RESPONSIBLE FOR FAMILIARIZING HIMSELF WITH ALL CONTRACT DOCUMENTS, FIELD CONDITIONS AND DIMENSIONS AND CONFIRMING THAT THE WORK MAY BE ACCOMPLISHED AS SHOWN PRIOR TO PROCEEDING WITH CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO THE COMMENCEMENT OF WORK.
4. PLANS ARE NOT TO BE SCALED. THESE PLANS ARE INTENDED TO BE A DIAGRAMMATIC OUTLINE ONLY, UNLESS OTHERWISE NOTED. THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO EFFECT ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
5. DIMENSIONS SHOWN ARE TO FINISH SURFACES, UNLESS OTHERWISE NOTED. SPACING BETWEEN EQUIPMENT IS REQUIRED CLEARANCE. THEREFORE, IT IS CRITICAL TO FIELD VERIFY DIMENSIONS. SHOULD THERE BE ANY QUESTIONS REGARDING THE CONTRACT DOCUMENTS, EXISTING CONDITIONS AND/OR DESIGN INTENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING A CLARIFICATION FROM THE CARRIER'S AUTHORIZED REPRESENTATIVE OR THE ENGINEER PRIOR TO PROCEEDING WITH THE WORK.
6. DETAILS ARE INTENDED TO SHOW END RESULT OF DESIGN. MINOR MODIFICATIONS MAY BE REQUIRED TO SUIT JOB DIMENSIONS OR CONDITIONS, AND SUCH MODIFICATIONS SHALL BE INCLUDED AS PART OF THE WORK.
7. CONTRACTOR SHALL RECEIVE CLARIFICATION IN WRITING, AND SHALL RECEIVE IN WRITING AUTHORIZATION TO PROCEED BEFORE STARTING WORK ON ANY ITEMS NOT CLEARLY DEFINED OR IDENTIFIED BY THE CONTRACT DOCUMENTS.
8. CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER OF ALL PRODUCTS OR ITEMS NOTED AS "EXISTING" WHICH ARE NOT FOUND TO BE IN THE FIELD.
9. CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK USING THE BEST CONSTRUCTION SKILLS AND ATTENTION. CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER CONTRACT, UNLESS OTHERWISE NOTED.
10. ERECTION SHALL BE DONE IN A WORKMANLIKE MANNER BY COMPETENT EXPERIENCED WORKMEN IN ACCORDANCE WITH APPLICABLE CODES AND THE BEST ACCEPTED PRACTICE. ALL MEMBERS SHALL BE LAID PLUMB AND TRUE AS INDICATED ON THE DRAWINGS.
11. CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY OF THE WORK AREA, ADJACENT AREAS, AND BUILDING OCCUPANTS THAT ARE LIKELY TO BE AFFECTED BY THE WORK UNDER THIS CONTRACT. WORK SHALL CONFORM TO ALL OSHA REQUIREMENTS.
12. CONTRACTOR SHALL COORDINATE HIS WORK AND SCHEDULE HIS ACTIVITIES AND WORKING HOURS IN ACCORDANCE WITH THE REQUIREMENTS OF THE OWNER.
13. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING HIS WORK WITH THE WORK OF OTHERS AS IT MAY RELATE TO RADIO EQUIPMENT, ANTENNAS AND ANY OTHER PORTIONS OF THE WORK.
14. CONTRACTOR SHALL MAINTAIN LIABILITY INSURANCE TO PROTECT THE OWNER.
15. INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY INDICATED OR WHERE LOCAL CODES OR REGULATIONS TAKE PRECEDENCE.
16. MAKE NECESSARY PROVISIONS TO PROTECT EXISTING SURFACES, EQUIPMENT, IMPROVEMENTS, PIPING, ANTENNA AND ANTENNA CABLES. REPAIR ANY DAMAGE THAT OCCURS DURING CONSTRUCTION.
17. REPAIR ALL EXISTING SURFACES DAMAGED DURING CONSTRUCTION SUCH THAT THEY MATCH AND BLEND WITH ADJACENT SURFACES.
18. KEEP CONTRACT AREA CLEAN, HAZARD FREE, AND DISPOSE OF ALL DEBRIS AND RUBBISH. EQUIPMENT NOT SPECIFIED AS REMAINING ON THE PROPERTY OF THE OWNER SHALL BE REMOVED. LEAVE PREMISES IN CLEAN CONDITION AND FREE FROM PAINT SPOTS, DUST, OR SMUDGES OF ANY NATURE. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL ITEMS UNTIL COMPLETION OF CONSTRUCTION.
19. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE ENGINEER.
20. CONTRACTOR SHALL PROVIDE 48 HOURS WRITTEN NOTICE TO THE ENGINEER PRIOR TO THE COMMENCEMENT OF WORK.
21. CONTRACTOR IS TO REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. CONTRACTOR SHOULD COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUBCONTRACTORS AND ALL RELATED PARTIES. THE SUBCONTRACTORS SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
22. CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON SITE AT ALL TIMES AND INSURE THE DISTRIBUTION OF THE NEW DRAWINGS TO SUBCONTRACTOR AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE. ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONSTRUCTION AREA. CONTRACTOR SHALL FURNISH 'AS-BUILT' SET OF DRAWINGS TO THE OWNER UPON COMPLETION OF THE PROJECT.
23. CONTRACTOR TO CONTACT "CALL BEFORE YOU DIG" AT 1-800-922-4455 TO VERIFY AND IDENTIFY THE EXACT LOCATIONS OF ALL UNDERGROUND UTILITIES AND OBSTRUCTIONS IDENTIFIED PRIOR TO COMMENCING WORK IN THE CONTRACT AREA.

PROJECT INFORMATION

JURISDICTION: TOWN OF NEW CASTLE
 CURRENT ZONING: B-R: RETAIL BUSINESS
 SECTION, BLOCK, & LOT: SECTION 91.8, BLOCK 3, LOT 1
 QUADRANGLE MAP: OSSINING
 DECLINATION: 12'±
 LATITUDE: (NAD 27) 41° 11' 16" N
 LONGITUDE: (NAD 27) 73° 48' 5" W



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
 • ENGINEERING • CONSTRUCTION MANAGEMENT
 TECTONIC Engineering & Surveying Consultants P.C.

955 Little Britain Road
 New Windsor, NY 12553
 Phone: (845) 567-6666
 Fax: (845) 567-8703

OMNIPONT COMMUNICATIONS INC.

As Agent For
 OMNIPONT FACILITIES NETWORK, L.L.C.
 OMNIPONT WIRELESS
 75 PROGRESS ROAD 2ND FLOOR
 STAMFORD, CT 06907
 OFFICE (203) 328-8600
 FAX (203) 328-4961

APPROVALS			
OWNER:			
LANDLORD:			
IF:			
CONSTRUCTION:			
PROJECT NUMBER:	2308.09215A		
DESIGNED BY:	EP		
REV.	DATE	REVISION	ORIGIN BY
Δ	4/16/03	FOR COMMENT	JFL
Δ	5/5/03	FOR PERMIT	CA
Δ	7/24/03	REVISED ANTENNAS	CC
Δ	8/28/03	FOR COMMENT	CC
Δ	11/10/03	FOR CONSTRUCTION	EPK

ISSUED BY: E. PETERSON DATE: 11-10-03



SITE INFORMATION
 NY-09-215-A
 CON-ED TOWER E-142
 STATION PLACE
 TOWN OF
 NEW CASTLE, NY

SHEET TITLE
 GENERAL NOTES

SHEET NUMBER
 T-2

ABBREVIATION LIST

L	ANGLE	GA.	GAUGE	REV	REVISION(S), REVISED
CL	CENTERLINE	GALV.	GALVANIZED	RF.	RADIO FREQUENCY
AT	AT	GC	GENERAL CONTRACTOR	RO.	ROUGH OPENING
PLATE	PLATE	GEN	GENERATOR	RM.	ROOM
DIAMETER	DIAMETER	GFI	GROUND FAULT INTERRUPTER	S.C.	SOLID CORE
PLUS OR MINUS	PLUS OR MINUS	GND	GROUND	SCHED.	SCHEDULE
NUMBER	NUMBER	GPS	GLOBAL POSITIONING SYSTEM	SIM.	SIMILAR
AND	AND	GRC	GALVANIZED RIGID CONDUIT	SPECS.	SPECIFICATIONS
		G.W.B.	GYPSON WALLBOARD	SS.	STAINLESS STEEL
AC	AIR CONDITIONING	H.C.	HANDICAPPED	STD.	STANDARD
ACT	ACOUSTICAL CEILING TILE	HORIZ.	HORIZONTAL	STL.	STEEL
ADJ	ADJUSTABLE	H.P.	HORSEPOWER	STOR.	STORAGE
AIC	AMPS INTERRUPTING CAPACITY	HGT.	HEIGHT	STR.	STRUCTURAL
AFF	ABOVE FINISH FLOOR	H.W.	HOT WATER	T/O	TOP OF
ALUM	ALUMINUM	I.D.	INSIDE DIAMETER	TEL.	TELEPHONE
ARCH	ARCHITECT(URAL)	IGR	INTERIOR GROUND RING (HALO)	TEMP.	TEMPERED, TEMPORARY
ATTEN	ATTENUATION	IN.	INCHES(S)	TYP	TYPICAL
AWG	AMERICAN WIRE GAUGE	JT.	JOINT	T&B.	TOP AND BOTTOM
B/O	BOTTOM OF	KVA	KILO-VOLT-AMPERES	U/G	UNDERGROUND
BCW	BARE COPPER WIRE	KW	KILO-WATT	U.L.	UNDERWRITERS LABORATORIES
BLDG	BUILDING	LB	POUNDS	UNO	UNLESS NOTED OTHERWISE
BLKG.	BLOCKING	MAX	MAXIMUM	VCT.	VINYL COMPOSITION TILE
BTS	BASE TRANSMISSION SYSTEM	MCB	MAIN CIRCUIT BREAKER	VERT.	VERTICAL
C.F.M.	CUBIC FEET PER MINUTE	MECH.	MECHANICAL	VIF	VERIFY IN FIELD
C.T.	CERAMIC TILE	MEZZ.	MEZZANINE	WD	WOOD
CL.	CLOSET	MFTG.	MANUFACTURING	WF.	WIDE FLANGE
CLG.	CEILING	MGB	MASTER GROUND BAR	WH.	WATER HEATER
CLR.	CLEAR(ANCE)	MIN.	MINIMUM	WWM	WELDED WIRE MESH
CMU.	CONCRETE MASONRY UNIT	MR.	MOISTURE RESISTANT	WP	WEATHERPROOF
COL.	COLUMN	MTG.	MOUNTING		
COMM.	COMMUNICATION	MTL.	METAL		
CONC.	CONCRETE	MPH.	MILES PER HOUR		
CONT.	CONTINUOUS	N/A	NOT APPLICABLE		
DET	DETAIL	N.I.C.	NOT IN CONTRACT		
DIA	DIAMETER	N.T.S.	NOT TO SCALE		
DIM	DIMENSION	O.C.	ON CENTER		
DN	DOWN	O.D.	OUTSIDE DIAMETER		
DWG	DRAWING	O/H	OVERHEAD		
EA.	EACH	PCS	PERSONAL COMMUNICATION SYSTEM		
EF	EXHAUST FAN	PT.	PRESSURE TREATED		
EL	ELEVATION (DATUM)	PL.	PLATE		
ELEC.	ELECTRICAL	PPVC	PERFORATED POLYVINYL CHLORIDE		
ELEV.	ELEVATION	PLYWD.	PLYWOOD		
EMER.	EMERGENCY	PM.	PRESSED METAL		
EMT	ELECTRICAL METALLIC TUBING	PVC.	POLYVINYL CHLORIDE		
EQ.	EQUAL	PSF.	POUNDS PER SQUARE FOOT		
EQUIP.	EQUIPMENT	R.	RADIUS		
EXIST.	EXISTING	R.C.P.	REFLECTED CEILING PLAN		
F.A.	FIRE ALARM	RD.	ROOF DRAIN		
F.C.	(GWB) FIRE CODE	REF.	REFERENCE		
F.C.O.	FLOOR CLEAN OUT	REINF.	REINFORCE(D), (ING)		
F.D.	FLOOR DRAIN	REP.	REPRESENTATIVE		
F.O.	FACE OF	REQD	REQUIRED		
FDN.	FOUNDATION				
FIN.	FINISH(ED)				
FL.	FLOOR				
F.T.	FOOT/FEET				

LEGEND

	CONTOUR LINE
	INDEX CONTOUR LINE
	EDGE OF PAVEMENT
	CURB
	CHAIN LINK FENCE
	PROPERTY LINE
	ADJOINING PROPERTY LINE
	SWALE LINE
	PROPOSED U/G UTILITIES
	EASEMENT LINE
	STONE WALL
	SILT FENCE
	TREE LINE
	STRUCTURE
	UTILITY POLE
	FIELD INLET
	WELL CAP
	IRON ROD FOUND
	TREES
	ZONE BOUNDARY LINE
	REVISION
	SECTION OR DETAIL NUMBER
	SHEET WHERE DETAIL OR SECTION OCCURS

0 1 2 3
ORIGINAL SIZE IN INCHES

UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 587-8554
Fax: (845) 587-8703

OMNIPONT COMMUNICATIONS INC.

An Agent for
OMNIPONT FACILITIES NETWORK 2, LLC
OMNIPONT WIRELESS 78 PROGRESS DRIVE, 2ND FLOOR STAMFORD, CT 06902 OFFICE: (203) 328-5840 FAX: (203) 328-5843

APPROVALS

OMNIPONT
LANDLORD
RF
CONSTRUCTION

PROJECT NUMBER 2358.002154 DESIGNED BY EP

REV	DATE	REVISION	DESIGNED BY
1	4/18/03	FOR COMMENT	JFL
2	8/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/10/03	FOR CONSTRUCTION	BFK

ISSUED BY E. Pacheco DATE 11-16-03



SHEET INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

ABBREVIATION LIST & LEGEND

SHEET NUMBER

T-3

BILL OF MATERIALS

CON-ED TOWER E-142

SITE NO.: NY-09-215-A

ITEM NO.	DESCRIPTION	QUANTITY	LENGTH (FT)	TOTAL LENGTH
1	<u>ANTENNA SECTORS:</u> ALPHA : ADFD1820-6565B-XD W/MECHANICAL DOWNTILT KIT BETA : ADFD1820-6565B-XD W/MECHANICAL DOWNTILT KIT GAMMA : ADFD1820-6565B-XD W/MECHANICAL DOWNTILT KIT	1 1 1	N/A N/A N/A	N/A N/A N/A
2	<u>JUMPER CABLES:</u> 6'-0" - 1/2" CABLEWAVE COAXIAL TOP JUMPER W/ CONNECTORS: (ANTENNA) 6'-0" - 1/2" CABLEWAVE COAXIAL BOTTOM JUMPER W/ CONNECTORS: (SUPERFLEX) 3'-0" - 1/2" CABLEWAVE COAXIAL AMPLIFIER JUMPER W/ CONNECTORS	12 12 -	- - -	N/A N/A N/A
3	<u>MAIN CABLES:</u> 1 5/8" DIA. HELIAX COAXIAL 1/2" DIA. HELIAX COAXIAL (GPS)	12 1	CUT TO LENGTH IN FIELD	1500'±* 25'±*
4	<u>AMPLIFIERS:</u> MHA	-	N/A	N/A
5	<u>GROUNDING KITS & 2- HOLE LUGS FOR:</u> 1 5/8" CABLE 1/2" CABLE (GPS)	36 3	N/A N/A	N/A N/A
6	<u>CONNECTORS - UNATTACHED FOR MAIN CABLES:</u> FEMALE "DIN" 1 5/8" FEMALE "DIN" 1/2" (GPS)	24 2	N/A N/A	N/A N/A
7	NORTELL: S12000 CABINETS	3	N/A	N/A
8	CCI BOOSTER	3	N/A	N/A
9	VIC-100 GPS ANTENNA	1	N/A	N/A

NOTES :

1. ITEMS LISTED ABOVE ARE TO BE SUPPLIED BY OCI AND INSTALLED BY CONTRACTOR.

*2. COAXIAL CABLE LENGTHS ARE ESTIMATED AND TO BE FIELD VERIFIED BY CONTRACTOR PRIOR TO ORDERING MATERIALS.

ANTENNA SCHEDULE

SECTOR	RADIATION CENTER (FT)	MECHANICAL DOWNTILT (DEG)	ORIENTATION CONFIGURATION	ANTENNAS PER SECTOR
ALPHA	96'-9"±	0°	0°	1
BETA	96'-9"±	0°	120°	1
GAMMA	96'-9"±	0°	240°	1

CABLE SCHEDULE

ANTICIPATED COAXIAL CABLE RUN (X) IN FEET	CABLE DIAMETER SIZE
0 < X < 6'	1/2"
6 < X < 100'	7/8"
100 < X < 200'	1-5/8"

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION MANAGEMENT
TECTONIC Engineering & Surveying Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-8858
Fax: (845) 567-8753

OMNIPONT COMMUNICATIONS INC

As Agent For
OMNIPONT FACILITIES NETWORK 2, LLC

OMNIPONT WIRELESS 18 PROGRESS BLVD, 2ND FLOOR OFFICE: (203) 328-8800
STAMFORD, CT 06903 FAX: (203) 328-8853

APPROVALS

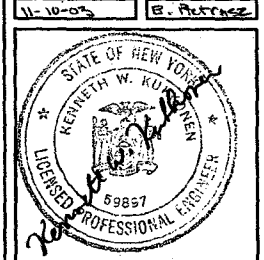
OMNIPONT: _____
LANDLORD: _____
BY: _____
CONSTRUCTION: _____

PROJECT NUMBER: 2356-09215A DESIGNED BY: EP

REV	DATE	REVISION	DRAWN BY
1	4/18/03	FOR COMMENT	JFL
2	6/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/10/03	FOR CONSTRUCTION	BRK

ISSUED BY: _____ DATE: _____

11-10-03 E. Retrevez



SITE INFORMATION

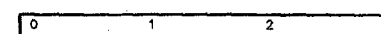
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

BILL OF MATERIALS

SHEET NUMBER

B-1



ORIGINAL SIZE IN INCHES

UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

CABINETS

CABINETS
JUMPERS

COAX CABLES

AMPLIFIERS
JUMPERSAMPLIFIERS
JUMPERSAMPLIFIERS
JUMPERS

ANTENNA

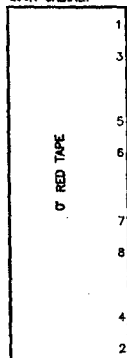
NOTES

1. ALL CABLE CONNECTIONS TO BE WEATHERPROOFED, INCLUDING ELECTRICAL TAPE, VAPOR SEAL (BUTYL TAPE), THREE LAYERS OF ELECTRICAL TAPE AND SCOTCH COAT.
2. CONTRACTOR TO PROVIDE DRIP LOOPS IN CABLES AND JUMPERS WHERE NECESSARY.
3. CABLE & JUMPER TAGGING:

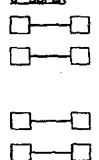
A. COLOR ELECTRICAL TAPE SHALL MARK EACH END OF CABLE AND EACH OF JUMPERS AS CLOSE TO EACH END AS POSSIBLE (NOT TO INTERFERE WITH WEATHER PROOFING).

B. ALL TAPE MARKINGS TO BE DONE IN SEQUENCE FROM TRUE NORTH, WORKING CLOCKWISE PER SECTOR RED, WHITE, BLUE MARKING (SEQUENCE DENOTED ON MATERIAL LIST).

MAIN CABINET



6' SUPER



DIN(F)
DIN(F)
DIN(F)
DIN(F)

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

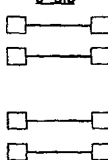
1 5/8" DIA COAXIAL CABLE AT 125'-0"±

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

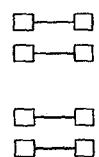
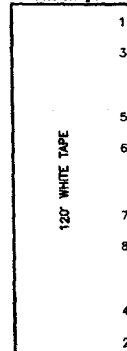
DIN(F)
DIN(F)
DIN(F)
DIN(F)

6' STD



ANDREW MODEL
ADFD1820-6565B-XD

EXPANSION #1



DIN(F)
DIN(F)
DIN(F)
DIN(F)

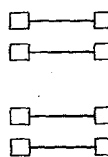
1 5/8" DIA COAXIAL CABLE AT 125'-0"±

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

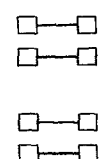
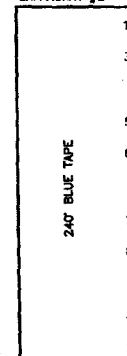
1 5/8" DIA COAXIAL CABLE AT 125'-0"±

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

DIN(F)
DIN(F)
DIN(F)
DIN(F)



ANDREW MODEL
ADFD1820-6565B-XD

FUTURE
EXPANSION #2

DIN(F)
DIN(F)
DIN(F)
DIN(F)

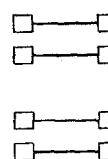
1 5/8" DIA COAXIAL CABLE AT 125'-0"±

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

1 5/8" DIA COAXIAL CABLE AT 125'-0"±

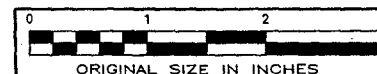
DIN(F)
DIN(F)
DIN(F)
DIN(F)



ANDREW MODEL
ADFD1820-6565B-XD

1 RF CABLE SCHEMATIC

B-2 SCALE: NTS



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

- PLANNING
- ENGINEERING
- SURVEYING
- CONSTRUCTION MANAGEMENT

Tectonic Engineering & Surveying Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 587-8658
Fax: (845) 587-8703

OMNIPONT COMMUNICATIONS INC

As Agent For
OMNIPONT FACILITIES NETWORK 2, LLC

OMNIPONT WIRELESS 700 PROGRESS BLVD., 2ND FLOOR STAMFORD, CT 06402 OFFICE: (203) 328-8800 FAX: (203) 328-8803

APPROVALS

OMNIPONT
UNAPPROVED
RF
CONSTRUCTION

PROJECT NUMBER 2306-09215A DESIGNED BY EP

REV	DATE	REVISION	DESIGNED BY
1	4/18/03	FOR COMMENT	JFL
2	6/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	OC
5	11/10/03	FOR CONSTRUCTION	WPK

ISSUED BY E. Picomaz DATE 11-10-03



SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

RF CABLE SCHEMATIC

SHEET NUMBER

B-2

GENERAL NOTES

1. PROPERTY LINES & EXIST TOWER FEATURES INDICATED BASED ON A SET OF DRAWINGS COMPLETED BY "TECTONIC ENGINEERING AND SURVEYING P.C." FOR "SPRINT SPECTRUM LP" ON JULY 15, 2002 AND A FIELD INSPECTION PERFORMED BY "TECTONIC ENGINEERING & SURVEYING CONSULTANTS P.C." ON NOVEMBER 13, 2002.
2. ADJACENT PROPERTY LINES AND TRUE NORTH SHOWN ARE BASED ON DRAWINGS REFERENCED IN NOTE #1.
3. VERTICAL DATUM BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929 (APPROXIMATE).
4. DESIGN AND CONSTRUCTION OF ANTENNA SUPPORTS SHALL CONFORM TO BOTH ANSI/TIA/EIA-222-F-1996 "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES". A BASIC WIND SPEED OF 80 MPH APPLIES TO WESTCHESTER COUNTY, NY, WHERE THE FACILITY IS LOCATED. THE BUILDING CODE OF NEW YORK STATE REQUIRES A 3-SECOND GUST WIND SPEED OF 110 MPH FOR THIS VICINITY, WHICH IS APPROXIMATELY EQUIVALENT TO A FASTEST-MILE WIND SPEED OF 90 MPH. ICE LOADS HAVE BEEN ESTABLISHED BASED ON A 0.5" RADIAL ICE THICKNESS IN ACCORDANCE WITH INDUSTRY STANDARD PRACTICE. A REDUCED WIND SPEED OF 79 MPH IS USED IN CONJUNCTION WITH THIS ICE LOAD.
5. THE PROPOSED FACILITY IS MINIMAL, AND WILL CREATE NEGLIGIBLE ADDITIONAL STORM WATER RUNOFF AND WILL THEREFORE NOT IMPACT THE EXISTING STORM WATER DRAINAGE SYSTEM.
6. THE PROPOSED FACILITY DOES NOT INCLUDE OUTDOOR STORAGE OR ANY SOLID WASTE RECEPTACLES.
7. THE PROPOSED FACILITY IS UNMANNED, AND THEREFORE DOES NOT REQUIRE A MEANS OF WATER SUPPLY OR SEWAGE DISPOSAL.
8. NO LANDSCAPING IS PROPOSED WITH THIS APPLICATION.
9. NO LIGHTING IS PROPOSED AS PART OF THIS APPLICATION.
10. NO COMMERCIAL OR RETAIL SIGN(S) SHALL BE ERECTED AS PART OF THIS APPLICATION.
11. THE PROPOSED FACILITY IS NOT WITHIN 2,500 FEET OF ANY HISTORIC DISTRICT OR ANY SITE WITH OFFICIAL DESIGNATION AS A LOCAL LANDMARK, OR WHICH IS LISTED OR DESIGNATED AS ELIGIBLE FOR LISTING ON THE STATE OR THE NATIONAL REGISTERS OF HISTORIC PLACES.
12. THE WETLANDS DELINEATION SHOWN ON SHEET C-2 WAS PERFORMED ON 4/24/01 BY DAVID GRIGGS FROM ERS CONSULTANTS, INC, WARWICK, N.Y.
13. NO TOWN-REGULATED TREES ARE PROPOSED TO BE REMOVED.
14. NO TOWN-REGULATED STEEP SLOPES ARE PROPOSED TO BE DISTURBED.
15. THE PROPOSED FACILITY SHALL BE PAINTED BLACK, OR ANOTHER NEUTRAL OR COMPATIBLE COLOR TO BE DETERMINED BY THE PLANNING BOARD AND THE MOUNTINGS SHALL BE NONREFLECTIVE AND OF THE APPROPRIATE COLOR TO BLEND WITH THEIR BACKGROUND.
16. PROPOSED ANTENNAS WILL BE PAINTED TO MATCH THE COLOR OF THE EXISTING TOWER.

SITE NOTES

1. ALL SITE WORK SHALL BE AS INDICATED ON THE DRAWINGS C-2 & C-3.
2. RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
3. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE BTS EQUIPMENT AND TOWER AREAS.
4. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.
5. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
6. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY ENGINEERS. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR PIER DRILLING AROUND OR NEAR UTILITIES.
7. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF ENGINEERING.
8. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK SHALL BE GRADED TO A UNIFORM SLOPE, FERTILIZED, SEEDED, AND COVERED WITH MULCH.
9. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE NEW YORK STATE STANDARDS AND SPECIFICATIONS FOR SEDIMENT AND EROSION CONTROL.
10. ALL RESTORATION ISSUES SHALL BE COMPLETED WITHIN 72 HOURS OF THE COMPLETION OF THE WORK ACTIVITY OR WITHIN A REASONABLE AMOUNT OF TIME AS DIRECTED BY THE CONSTRUCTION MANAGER/ENGINEER.
11. CARE SHALL BE TAKEN TO RETAIN NATURAL GROWTH AND PREVENT DAMAGE TO TREES, WITHIN AND OUTSIDE THE LIMITS OF CONSTRUCTION AND SPECIFIED WORK AREAS, CAUSED BY EQUIPMENT AND MATERIALS. ANY DAMAGE TO THIS NATURAL GROWTH SHALL BE RESTORED AT THE EXPENSE OF THE CONTRACTOR.
12. ALL AREAS DISTURBED BY THE CONTRACTOR WITHOUT AUTHORIZATION SHALL BE RESTORED BY THE CONTRACTOR.
13. IN THE EVENT THE CONTRACTOR DAMAGES AN EXISTING UTILITY SERVICE CAUSING AN INTERRUPTION IN SAID SERVICE, HE SHALL IMMEDIATELY COMMENCE WORK TO RESTORE SERVICE AND MAY NOT CONTINUE HIS WORK OPERATION UNTIL SERVICE IS RESTORED.



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT
TECTONIC Engineering & Surveying
Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-8656
Fax: (845) 567-8703

OMNIPONT COMMUNICATIONS INC.

An Agent For
OMNIPONT FACILITIES NETWORK 2, LLC
OMNIPONT WIRELESS
71 PROGRESS DRIVE, 2ND FLOOR
STAMFORD, CT 06902
OFFICE: (203) 328-0800
FAX: (203) 328-0803

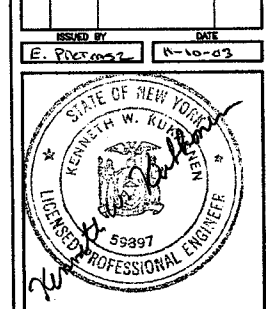
APPROVALS

CLIENT: _____
LANDLORD: _____
RF: _____
CONSTRUCTION: _____

PROJECT NUMBER: 2368/0215A
DESIGNED BY: EP

REV	DATE	REVISION	DRAWN BY
1	4/10/03	FOR COMMENT	JFL
2	6/15/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	OC
5	11/10/03	FOR CONSTRUCTION	BNK

ISSUED BY: E. P. 02/03/03
DATE: 11-10-03



SITE INFORMATION

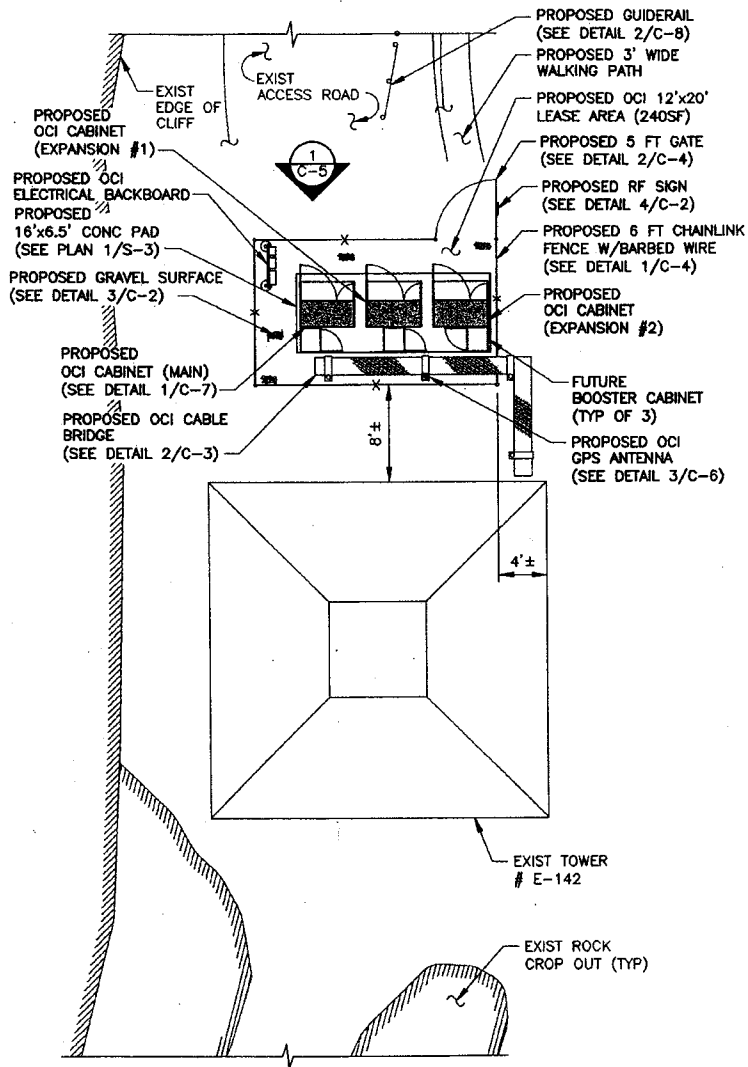
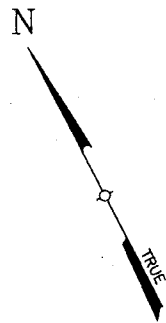
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

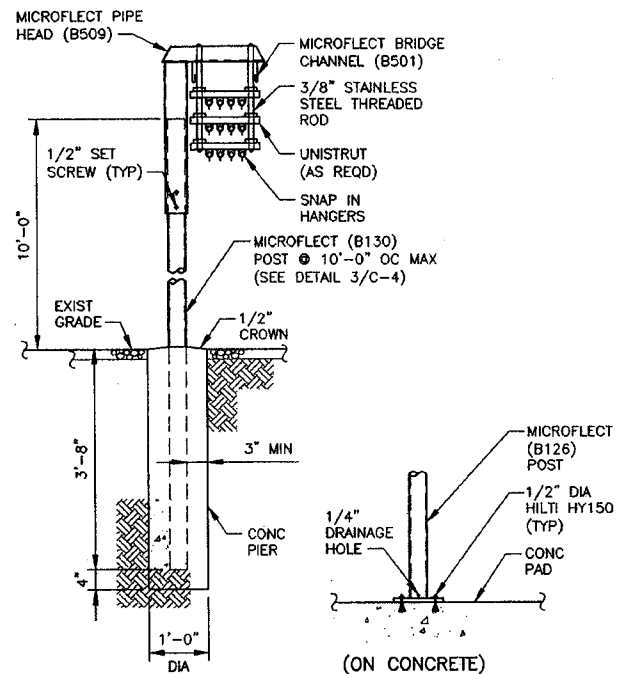
NOTES

SHEET NUMBER

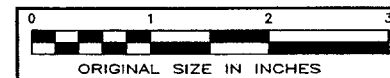
C-1



1
C-3
SITE DETAIL PLAN
SCALE: 1" = 10'



2
C-3
CABLE BRIDGE (TYP)
SCALE: 1/2" = 1'-0"



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

PLANNING
ENGINEERING
SURVEYING
CONSTRUCTION
MANAGEMENT
TECTONIC Engineering & Surveying
Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 587-5656
Fax: (845) 587-8703

OMNIPONT COMMUNICATIONS INC.

An Agent For
OMNIPONT FACILITIES NETWORK 2, LLC
OMNIPONT WIRELESS
75 PROGRESS DRIVE, 3RD FLOOR
SHELTON, CT 06484
OFFICE: (203) 338-8900
FAX: (203) 338-8953

APPROVALS
LANDLORD
RF
CONSTRUCTION

PROJECT NUMBER
2356.06215A
DESIGNED BY
EP

REV	DATE	REVISION	DRAWN BY
1	4/16/03	FOR COMMENT	JFL
2	8/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/10/03	FOR CONSTRUCTION	BFK

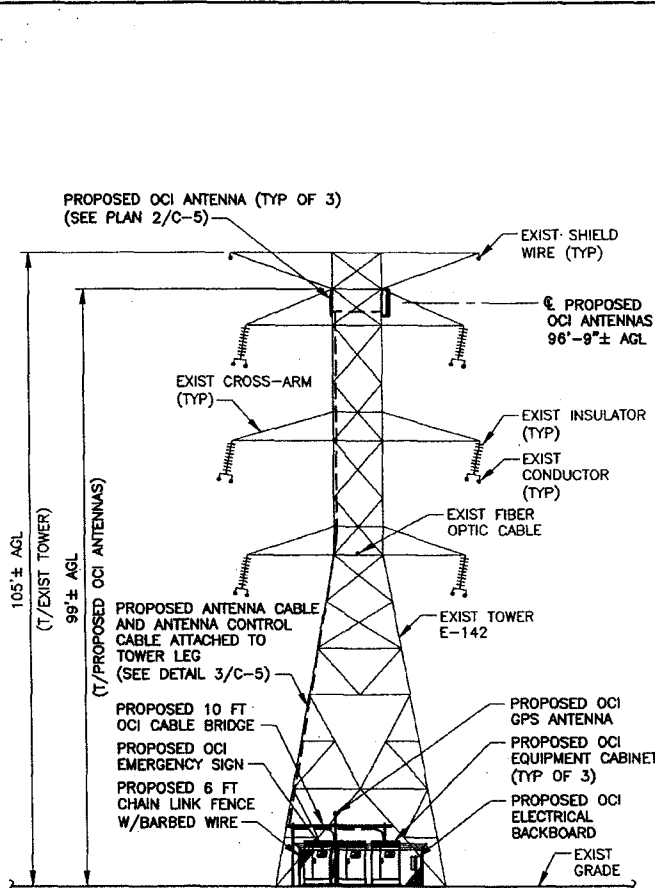
SIGNED BY
E. P. PETERSON
DATE
11-10-03



SITE INFORMATION
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE
SITE DETAIL PLAN

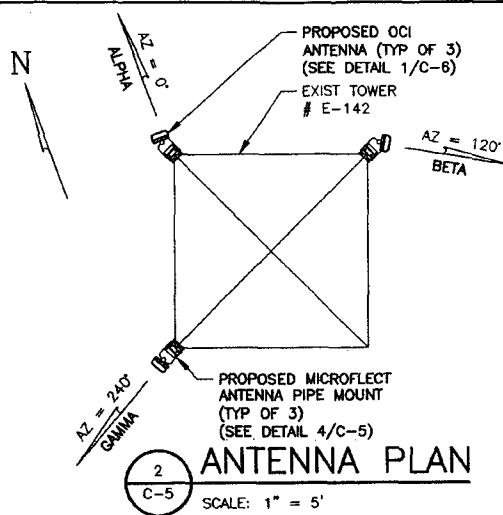
SHEET NUMBER
C-3



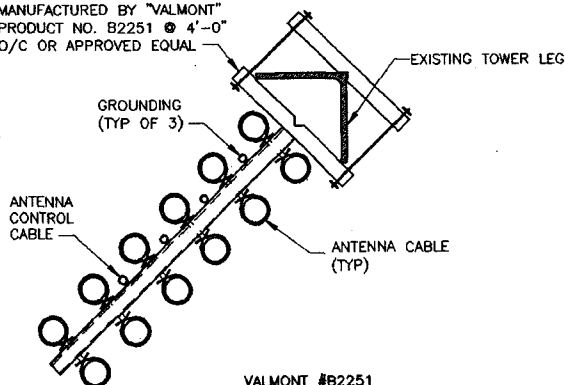
1
C-5
ELEVATION
SCALE: 1" = 20'

NOTES:

- ONE ATCA-AD1-060 ANTENNA CONTROL CABLE ASSEMBLY TO CONTROL THE REMOTE VARIABLE ELECTRICAL DOWNTILT FOR UP TO THREE ANTENNAS. CONTRACTOR TO VERIFY LENGTH OF CONTROL CABLE PRIOR TO ORDERING
- CONTROL CABLE CARRIES 24V DC WHEN DOWNTILT IS BEING ADJUSTED.
- CONTROL CABLE WILL BE ROUTED IN PVC CONDUIT TO ANTENNAS FROM EQUIPMENT CABINETS.



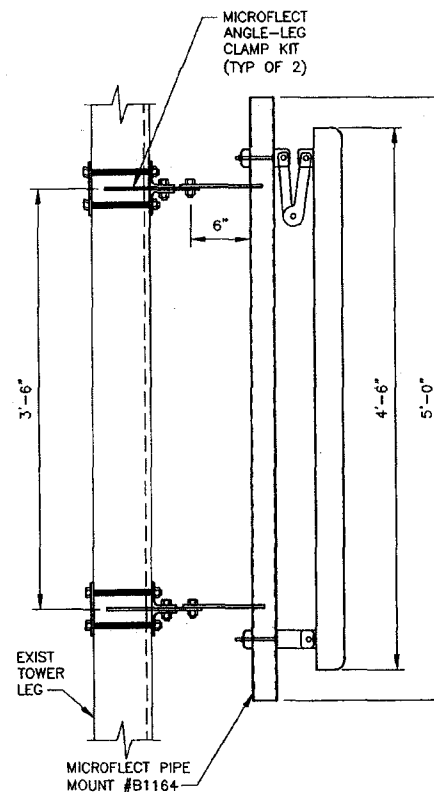
UNIVERSAL SNAP IN BRACKET
MANUFACTURED BY "VALMONT"
PRODUCT NO. B2251 @ 4'-0"
O/C OR APPROVED EQUAL



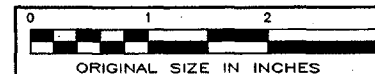
NOTES:

- INSTALL UNISTRUTS TO EXISTING TOWER LEG AT LOCATIONS THAT DO NOT INTERFERE WITH EXISTING BRACING MEMBERS.
- ANTENNA CABLE & HANGER TO BE PAINTED TO MATCH COLOR OF EXIST TOWER.

3
C-5
COAX SUPPORT DETAIL
SCALE: 1 1/2" = 1'-0"



4
C-5
ANTENNA ELEVATION
SCALE: 1" = 1'-0"



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-8454
Fax: (845) 567-8703

OMNIPONT COMMUNICATIONS INC

An Agent For
OMNIPONT FACILITIES NETWORK 1, LLC
OMNIPONT WIRELESS
75 PROGRESS DRIVE AND FLOOR FIVE
STAMFORD, CT 06903
OFFICE (203) 328-8800
FAX (203) 328-8853

APPROVALS

OWNER/POINT
LANDLORD
RF
CONSTRUCTION

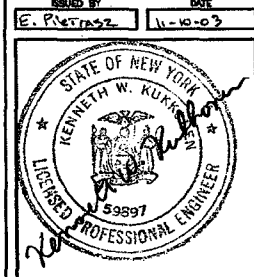
PROJECT NUMBER
2358.092104

DESIGNED BY
EP

REV	DATE	REVISION	DRAWN BY
1	4/16/03	FOR COMMENT	JFL
2	8/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	DC
5	11/10/03	FOR CONSTRUCTION	BFK

ISSUED BY
E. P. P. P. P.

DATE
11-10-03



SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

ELEVATION
& ANTENNA PLAN

SHEET NUMBER

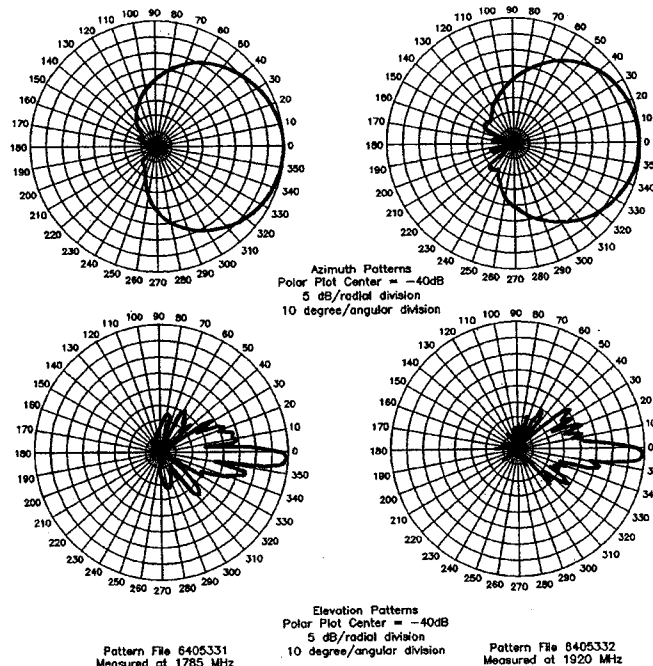
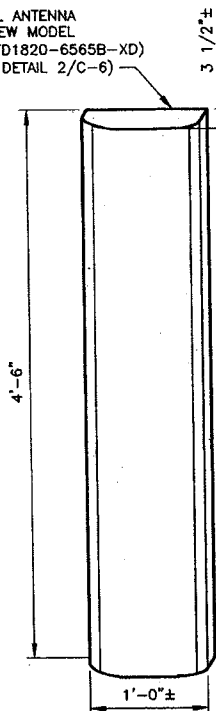
C-5

ANTENNA DATA

SECTOR	MODEL NO.	NUMBER	AZIMUTH
ALPHA	ADFD1820-6565B-XD	1	0°
BETA	ADFD1820-6565B-XD	1	120°
GAMMA	ADFD1820-6565B-XD	1	240°

ADFD1820-6565B-XD			
HEIGHT	54.4"	AZIMUTH BEAMWIDTH	65°
WIDTH	11.8"	ELEVATION BEAMWIDTH	6.5°
DEPTH	3.5"	POLARIZATION	SLANTED ±45°
WEIGHT	15.7 LBS	PORT-TO-PORT ISOLATION	30dB
FRONT WIND LOAD @ 100 MPH	163 LBS	FRONT-TO-BACK RATIO	50 dB
SIDE WIND LOAD @ 100 MPH	57 LBS	VSWR	1.5:1 MAX
RATED WIND VELOCITY	125 MPH	POWER HANDLING	300 WATTS CW
		PASSIVE INTERMODULATION	<-150 dBc
		LIGHTNING PROTECTION	DC GROUND

PANEL ANTENNA
ANDREW MODEL
(#ADFD1820-6565B-XD)
(SEE DETAIL 2/C-6)



ANTENNA DETAIL

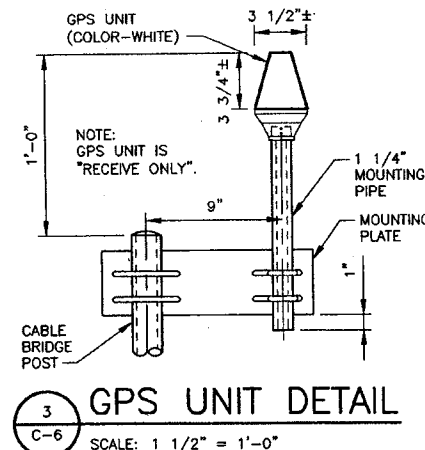
SCALE: 1" = 1'-0"

ANTENNA PATTERNS

SCALE: NTS

ANTENNA MOUNTING NOTES

- DESIGN AND CONSTRUCTION OF ANTENNA SUPPORTS SHALL CONFORM TO BOTH ANSI/TIA/EIA-222-F-1996 "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES". A BASIC WIND SPEED OF 80 MPH APPLIES TO WESTCHESTER COUNTY, NY, WHERE THE FACILITY IS LOCATED. THE BUILDING CODE OF NEW YORK STATE REQUIRES A 3-SECOND GUST WIND SPEED OF 110 MPH FOR THIS VICINITY, WHICH IS APPROXIMATELY EQUIVALENT TO A FASTEST-MILE WIND SPEED OF 90 MPH. ICE LOADS HAVE BEEN ESTABLISHED BASED ON A 0.5" RADIAL ICE THICKNESS IN ACCORDANCE WITH INDUSTRY STANDARD PRACTICE. A REDUCED WIND SPEED OF 79 MPH IS USED IN CONJUNCTION WITH THIS ICE LOAD.
- ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC-COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.
- ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC-COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.
- DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED BY COLD GALVANIZING IN ACCORDANCE WITH ASTM A780.
- ALL ANTENNA MOUNTS SHALL BE INSTALLED WITH DOUBLE NUTS AND SHALL BE INSTALLED SNUG TIGHT.
- DESIGN RESPONSIBILITY OF ANTENNA MOUNTING BRACKETS AND POLES AND ALL COMPONENTS THEREOF AND ATTACHMENT THERE TO SHALL BE THE RESPONSIBILITY OF THE MANUFACTURER. MFR SHALL PROVIDE THE OWNER DRAWINGS DETAILING ALL COMPONENTS OF THE ASSEMBLY, INCLUDING CONNECTIONS, DESIGN LOADS, AND ALL OTHER PERTINENT DATA. ALL SUBMISSIONS SHALL BEAR THE STAMP AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF NEW YORK.
- ALL WORK SHALL BE PERFORMED IN CALM WEATHER, WITH WIND GUSTS LESS THAN 20 MPH.



GPS UNIT DETAIL

SCALE: 1 1/2" = 1'-0"



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

PLANNING ENGINEERING SURVEYING CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

855 Little Britain Road
New Windsor, NY 12553
Phone: (845) 947-8555
Fax: (845) 567-8703

OMNIPONT COMMUNICATIONS INC.

OMNIPONT FACILITIES NETWORK 2, LLC

OMNIPONT WIRELESS
78 PROGRESS DRIVE, 2ND FLOOR
STAMFORD, CT 06902
OFFICE: (203) 325-8880
FAX: (203) 325-8853

APPROVALS

OMNIPONT
LANDLORD
BY
CONSTRUCTION

PROJECT NUMBER
2394.09215A

DESIGNED BY
EP

REV DATE REVISION DRAWN BY

4/18/03 FOR COMMENT JFL

6/5/03 FOR PERMIT JFL

7/24/03 REVISED ANTENNAS CA

8/28/03 PER COMMENT CC

11/10/03 FOR CONSTRUCTION BFK

ISSUED BY
E. PICTORSZ

DATE
11-10-03

STATE OF NEW YORK

KENNETH W. KUPCEN

59897

LICENSED PROFESSIONAL ENGINEER

SITE INFORMATION

NY-09-215-A

CON-ED TOWER E-142

STATION PLACE

TOWN OF

NEW CASTLE, NY

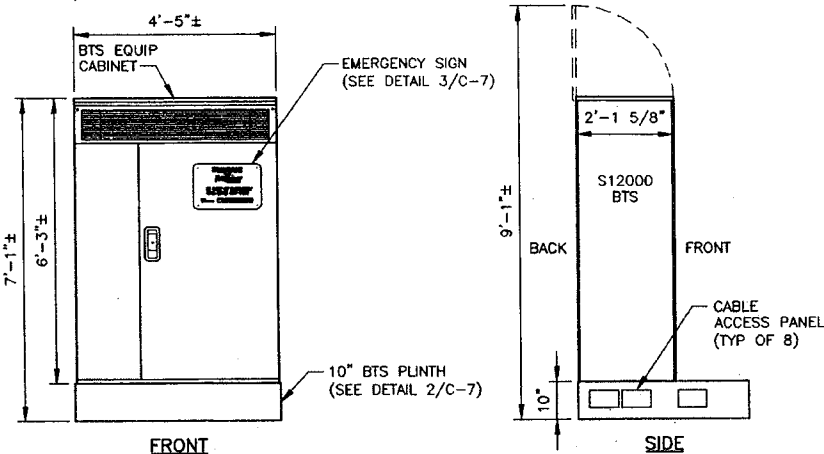
SHEET TITLE

ANTENNA DETAIL

& NOTES

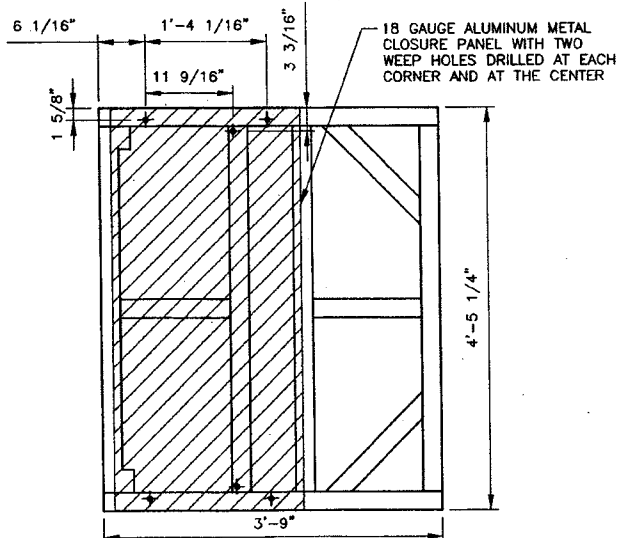
SHEET NUMBER

C-6



1 EQUIPMENT DETAIL

SCALE: 3/8" = 1'-0"



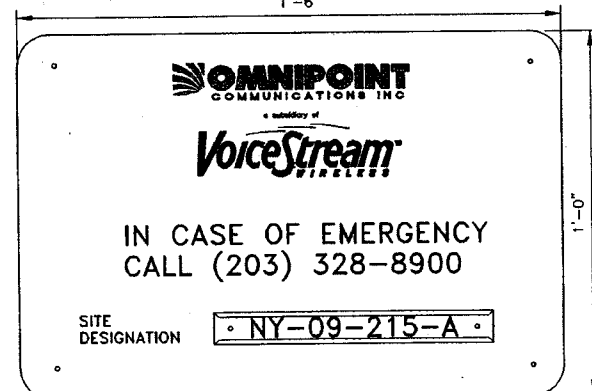
- NOTE:
1. CONTRACTOR TO VERIFY ALL BOLT LOCATIONS.
 2. ALL HOLES 11/16" DIA. TYPICAL.
 3. WEIGHT OF PLINTH IS 110 LBS.

2 PLAN OF PLINTH

SCALE: 3/4" = 1'-0"

EQUIPMENT SPECIFICATION TABLE

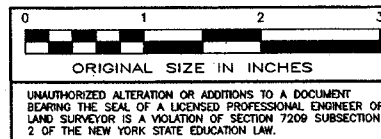
MANUFACTURE MODEL #	NORTEL S12000
FREQUENCY RANGE	900 MHZ GSM/900MHZ EXTENDED GSM 1800MHZ GSM AND DUAL BAND GSM 900/1800 850 MHZ GSM 1900MHZ GSM AND DUAL BAND 850/1900
RECEIVE SENSITIVITY	W/O DIVERSITY -110 DBM GUARANTEED (W/O TMA) WITH DIVERSITY -115 DBM GUARANTEED (W/O TMA)
HEIGHT	75.2 IN.
WIDTH	53.2 IN.
DEPTH	25.2 IN.
WEIGHT	EMPTY CABINET 441 LB FULLY EQUIPPED 1257 LB
CAPACITY	STANDARD 12 TRX PER RADIO CABINET UP TO 3 RADIO CABINETS OPTIONAL UP TO 4 RADIO CABINETS
CONFIGURATION	MONOBAND TRISECTORIAL UP TO S16-16-16 (4 RADIO CABINETS) DUAL BAND TRISECTORIAL S222_222 (1 RADIO CABINET) CELL SPLITTING MONO-BCCCH DUAL BAND CELLS CELL SPLITTING ACROSS RADIO CABINETS
AMPLIFIER OUTPUT POWER	STANDARD 30W (+/- 0.5 DB) OPTIONAL 60W (+/- 0.5 DB)
TRANSMISSION COUPLING	ALL COUPLING CONFIGURATIONS FROM DUPLEXERS TO 4 WAYS HYBRID COUPLING (H4D)
POWER CONTROL	STATIC 6 STEPS OF 2DB DYNAMIC 15 STEPS OF 2DB
FREQUENCY HOPPING	RF SYNTHESIZED
SUPPORTED VOCODERS	FULL RATE (FR) ENHANCED FULL RATE (EFR) ADAPTIVE MULTI-RATE - FULL RATE (AMR FR) ADAPTIVE MULTI-RATE - HALF RATE (AMR FR)
ENCRYPTION ALGORITHMS	A5/1 & A5/2
POWER SUPPLY	NOMINAL DC -48V
OPERATIONAL TEMPERATURE RANGE	-5°C TO +45°C
MAX ACOUSTIC NOISE	65 DB(A)
BACKHAUL	STANDARD 6 E1/T1 LINKS OPTIONAL 8 E1/T1 LINKS



(RED METAL SIGN W/WHITE LETTERING)

3 EMERGENCY SIGN

SCALE: 3" = 1'-0"



TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION MANAGEMENT
TECTONIC Engineering & Surveying Consultants P.C.
955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 587-5555
Fax: (845) 587-8703

OMNIPONT COMMUNICATIONS INC.
An Agent For
OMNIPONT FACILITIES NETWORKS 2, LLC
OMNIPONT WIRELESS 28 PROCESSION DRIVE, 2ND FLOOR OFFICE (203) 328-8900
STAMFORD, CT 06902 FAX: (203) 328-8903

APPROVALS			
OMNIPONT			
LANDLORD			
RF			
CONSTRUCTION			
PROJECT NUMBER	2306.09215A	DESIGNED BY	EP
REV	DATE	REASON	ORIGIN BY
1	4/15/03	FOR COMMENT	JFL
2	8/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/10/03	FOR CONSTRUCTION	EPK

ISSUED BY: E. PICTONZL DATE: 11-10-03



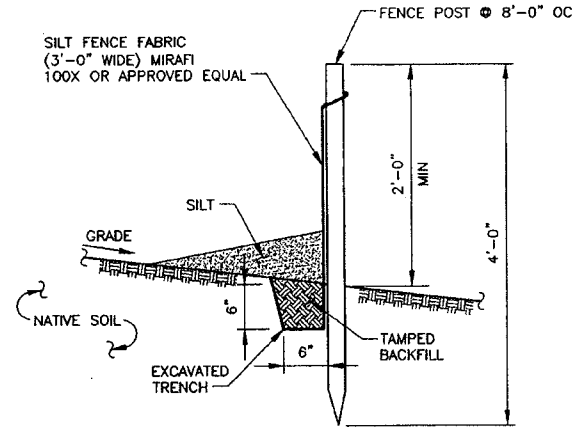
SITE INFORMATION
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE
CABINET DETAILS

SHEET NUMBER
C-7

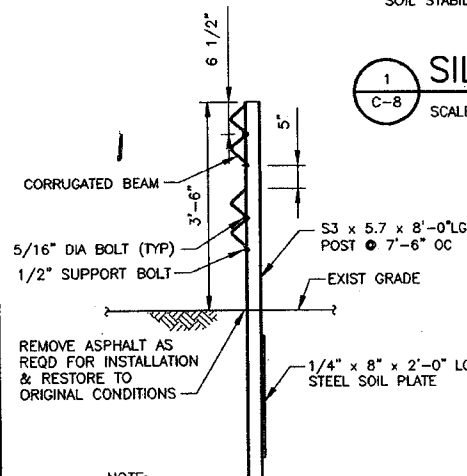
SOIL AND EROSION CONTROL NOTES

1. TEMPORARY SEDIMENTATION ENTRAPMENT AREAS SHALL BE PROVIDED AT KEY LOCATIONS TO INTERCEPT AND CLARIFY SILT LADEN RUNOFF FROM THE SITE.
2. SILT THAT LEAVES THE SITE IN SPITE OF THE REQUIRED PRECAUTIONS SHALL BE COLLECTED AND REMOVED AS DIRECTED BY APPROPRIATE MUNICIPAL AUTHORITIES.
3. AT THE COMPLETION OF THE PROJECT, ALL TEMPORARY SILTATION DEVICES SHALL BE REMOVED AND THE AFFECTED AREAS REGRADED, OR TREATED IN ACCORDANCE WITH THE APPROVED SITE PLANS.
4. ALL SEDIMENTATION ENTRAPMENT STRUCTURES WILL BE INSPECTED AND MAINTAINED ON A REGULAR BASIS.
5. CONTRACTOR TO INSTALL EROSION CONTROL MEASURES (SILT FENCE AND/OR HAY BALES) AROUND AREAS BEING DISTURBED DURING CONSTRUCTION AND AS NECESSARY.
6. CONTRACTOR TO INSTALL SILT FENCE ON DOWN SLOPE OF ALL UTILITY TRENCHES.
7. a. DISTANCES SHOWN FROM THE WETLANDS ON THE CONSTRUCTION PLANS AND SOIL EROSION AND SEDIMENTATION CONTROL PLANS ESTABLISH THE MINIMUM SEPARATION PERMITTED BETWEEN THE PROPOSED CONSTRUCTION ACTIVITIES AND BOUNDARY OF THE WETLANDS.
- b. AREA OF DISTURBANCE LINES SHALL BE CLEARLY DELINEATED IN THE FIELD BY INSTALLING SNOW FENCE AROUND THE ENTIRE PROPOSED CONSTRUCTION AREA. EXCEPT AS NECESSARY TO PROVIDE MITIGATION PLANTINGS, NO ENCROACHMENT BEYOND THESE LIMITS BY WORKERS OR MACHINERY SHALL BE PERMITTED.
- c. GRADING AND CLEARING AND OTHER CONSTRUCTION-RELATED ACTIVITIES SHALL TAKE PLACE ONLY WITHIN THE DELINEATED AREA OF DISTURBANCE LINES. THESE AREA OF DISTURBANCE LINES REPRESENT THE MAXIMUM LIMITS OF CONSTRUCTION ACTIVITIES. EVERY ATTEMPT SHALL BE MADE TO FURTHER REDUCE GRADING AND CLEARING ACTIVITIES WITHIN THE AREA OF DISTURBANCE LINES BY MAINTAINING NATURAL VEGETATION AND TOPOGRAPHY WHEREVER PRACTICABLE.
- d. ALL CONSTRUCTION AND CONSTRUCTION-RELATED ACTIVITIES OCCURRING ON THIS SITE SHALL COMPLY WITH STANDARDS AND RECOMMENDATIONS OF THE LATEST "BEST MANAGEMENT PRACTICES MANUAL FOR CONSTRUCTION RELATED ACTIVITIES" PREPARED BY THE WESTCHESTER COUNTY SOIL AND WATER CONSERVATION DISTRICT.
- e. PRIOR TO THE COMMENCEMENT OF ANY SITE WORK, THE APPLICANT SHALL STAKE THE LOCATION OF THE CONSTRUCTION ACTIVITY FOR INSPECTION AND APPROVAL BY THE TOWN ENGINEER.
- f. ALL SOIL EROSION AND SEDIMENTATION CONTROL MEASURES SHOWN ON THIS PLAN SHALL BE IN PLACE PRIOR TO THE START OF ANY SITE WORK. THE TOWN ENGINEER SHALL HAVE INSPECTED THE INSTALLATION OF ALL REQUIRED SOIL EROSION AND SEDIMENTATION CONTROL MEASURES PRIOR TO THE AUTHORIZATION TO PROCEED WITH ANY PHASE OF THE SITE WORK.
- g. THROUGHOUT THE CONSTRUCTION PERIOD, A QUALIFIED PROFESSIONAL RETAINED BY THE APPLICANT SHALL, ON AT LEAST A WEEKLY BASIS, PRIOR TO ANY PREDICTED RAIN EVENT AND AFTER ANY RUNOFF-PRODUCING RAIN EVENT, INSPECT THE SOIL EROSION AND SEDIMENTATION CONTROL MEASURES TO ENSURE THEIR PROPER FUNCTIONING.
- h. ALL DRAINAGE STRUCTURES AND ANY OTHER REQUIRED UTILITY APPURTENANCES SHALL BE INSTALLED AS REQUIRED BY TOWN SPECIFICATIONS AND AS SHOWN ON THESE PLANS.
- i. IF THE APPLICANT, DURING THE COURSE OF CONSTRUCTION, ENCOUNTERS SUCH CONDITIONS AS FLOOD AREAS, UNDERGROUND WATER, SOFT OR SILTY AREAS, IMPROPER DRAINAGE, OR ANY OTHER UNUSUAL CIRCUMSTANCES OR CONDITIONS THAT WERE NOT FORESEEN IN THE ORIGINAL PLANNING, HE SHALL REPORT SUCH CONDITIONS IMMEDIATELY TO THE TOWN ENGINEER. THE APPLICANT MAY SUBMIT, IF HE SO DESIRES, HIS RECOMMENDATIONS AS THE SPECIAL TREATMENT TO BE GIVEN SUCH AREAS TO SECURE ADEQUATE, PERMANENT AND SATISFACTORY CONSTRUCTION. THE TOWN ENGINEER, WITHOUT UNNECESSARY DELAY, SHALL INVESTIGATE THE CONDITION OR CONDITIONS, AND SHALL EITHER APPROVE THE APPLICANT'S RECOMMENDATION TO CORRECT THE CONDITIONS, ORDER A MODIFICATION THEREOF, OR ISSUE HIS OWN SPECIFICATION FOR THE CORRECTION OF THE CONDITIONS. IN THE EVENT OF THE APPLICANT'S DISAGREEMENT WITH THE DECISION OF THE TOWN ENGINEER, OR IN THE EVENT OF A SIGNIFICANT CHANGE RESULTING TO THE SITE PLAN OR ANY CHANGE THAT INVOLVES THE WETLANDS REGULATED AREAS, THE MATTER SHALL BE DECIDED BY THE PLANNING BOARD. ANY SUCH CONDITIONS OBSERVED BY THE PLANNING BOARD OR ITS AGENTS SHALL BE SIMILARLY TREATED.



NOTE:
SILT FENCE TO BE MAINTAINED IN PLACE DURING CONSTRUCTION AND SOIL STABILIZATION PERIOD.

1 SILT FENCE
C-8 SCALE: 3/4" = 1'-0"



NOTE:
CONTRACTOR TO VERIFY THE U/G UTILITIES BEFORE DIGGING OR DRILLING.

2 GUIDERAIL DETAIL
C-8 SCALE: 1/2" = 1'-0"



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT

TECTONIC Engineering & Surveying
Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 947-8858
Fax: (845) 947-8703

OMNIPONT COMMUNICATIONS INC

An Agent For
OMNIPONT FACILITIES NETWORK 2, LLC

OMNIPONT WIRELESS 78 PROGRESS DRIVE, 2ND FLOOR
STAMFORD, CT 06903 OFFICE: (203) 328-8880
FAX: (203) 328-8853

APPROVALS

OMNIPONT
LANDLORD
IF
CONSTRUCTION

PROJECT NUMBER
2359.08215A

DESIGNED BY
EP

REV	DATE	REVISION	DRAWN BY
1	4/18/03	FOR COMMENT	JFL
2	6/3/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/16/03	FOR CONSTRUCTION	BRK

ISSUED BY
E. PIETAS

DATE
8-10-03



SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

NOTES & DETAILS

SHEET NUMBER

C-8

STEEL NOTES

DESIGN BASIS (BUILDING CODE GOVERNING CODE OF NEW YORK)

DESIGN LOADS:
SNOW LOAD 45 PSF
WIND LOAD (ANTENNA) 30 PSF
TOTAL EQUIPMENT DEAD LOAD ON FRAME 4200 LBS.

1. DETAIL, FABRICATE AND ERECT STRUCTURAL STEEL IN ACCORDANCE WITH THE LATEST AISC MANUAL OF STEEL CONSTRUCTION (ASD), AWS D1.1, AND OTHER CODES, STANDARDS, AND SPECIFICATIONS EXCEPT AS EXPLICITLY MODIFIED HEREIN.
 - A) ASTM A36, GRADE 36; ROLLED STEEL, RODS, PLATES, AND U BOLTS
 - B) ASTM A325 BEARING TYPE BOLTS FOR ALL BEAMS AND COLUMN CONNECTIONS
2. CONNECTIONS: WELD OR BOLT CONNECTIONS, AS INDICATED:
 - A) CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL CONFORM TO THE REQUIREMENTS OF THE CITED AISC SPECIFICATION.
 - B) STRUCTURAL BOLTS SHALL CONFORM TO THE LATEST ASTM A325 "HIGH STRENGTH BOLTS FOR STRUCTURAL JOINTS, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS".
 - C) WHERE THE REACTION VALUES OF BEAMS, BRACING, STRUTS, ETC., ARE NOT SHOWN ON THE DRAWINGS THE CONNECTIONS SHALL BE DESIGNED TO SUPPORT THE END REACTION DERIVED FROM THE TABLES AND FORMULA OF UNIFORM LOAD CONSTANTS IN PART 2, NINTH EDITION, OF THE AISC MANUAL OF STEEL CONSTRUCTION FOR THE GIVEN MEMBER SIZE, SPAN, AND YIELD STRENGTH.
 - D) MINIMUM 3/16" FILLET E70-XX WELD SHALL APPLY UNLESS NOTED.
 - E) MINIMUM 1/2" DIA. A325 BOLTS SHALL APPLY UNLESS NOTED.
 - F) MINIMUM SIZE OF CLIP ANGLES SHALL BE 1.3x3x3/8" UNLESS NOTED.
 - G) ALL GUSSET PLATES SHALL BE 3/8" THICK UNLESS NOTED.
 - H) ALL HOLES FOR BOLTS SHALL BE 1/16 INCH LARGER THAN THE BOLT DIAMETER WITH AN EDGE DISTANCE OF AT LEAST 1 1/2 TIMES THE BOLT DIAMETER AND A SPACING OF AT LEAST 3 TIMES THE BOLT DIAMETER. ALL BOLTS SHALL BE PROVIDED WITH PALNUTS OR LOCK NUTS.
3. SUBMIT DRAWINGS OF ALL STRUCTURAL AND MISCELLANEOUS STEEL TO THE ENGINEER FOR APPROVAL, AND INCORPORATE ALL COMMENTS PRIOR TO FABRICATION.
4. CONTRACTOR SHALL COMPLY WITH THE LATEST AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS, AND FOR METHODS USED IN CORRECTING WELDING. ALL WELDERS AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES".
5. EXPANSION BOLTS SHALL CONFORM TO FEDERAL SPECIFICATION FF-S-325, GROUP II, TYPE 4, CLASS 1, HILTI KWIK BOLT II OR APPROVED EQUAL. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE FOUR (4) INCHES.
6. ALL EXTERIOR BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC-COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.
7. ROUND AND SQUARE HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500 "COLD-FORMED WELDED & SEAMLESS CARBON STEEL STRUCTURAL TUBING", GRADE B.
8. ALL STEEL MATERIALS SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A153 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS", UNLESS OTHERWISE NOTED.
9. DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED BY COLD GALVANIZING IN ACCORDANCE WITH ASTM A780.
10. INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE ENGINEER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER APPROVAL.
11. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE ENGINEER.
12. GRATING SHALL BE GALVANIZED SERRATED WELDED STEEL BAR GRATING WITH 1"x3/16" BEARING BARS AT 1-3/16" O.C. FASTEN TO SUPPORTING MEMBERS WITH SADDLE-TYPE CLIPS AT 2'-0" O.C. AND BAND ALL EXPOSED EDGES.
13. HAMMER DRILLS ARE NOT TO BE USED WHEN DRILLING HOLES FOR SLEEVE OR EXPANSION BOLTS INSTALLED IN MASONRY BLOCKS/BRICKS
14. EPOXY ANCHOR ASSEMBLY SHALL CONSIST OF A 5/8" DIAMETER STAINLESS STEEL THREADED ANCHOR ROD WITH NUTS & WASHERS, A SCREEN TUBE WHERE REQUIRED AND AN EPOXY ADHESIVE. THE ANCHORING SYSTEM SHALL BE THE HILTI HIT HY-20 OR HY-150 SYSTEM OR APPROVED EQUAL. MINIMUM EMBEDMENT SHALL BE SIX (6) INCHES UNLESS OTHERWISE NOTED.
15. ALL HOLES TO BE ADDED IN THE FIELD SHALL BE PUNCHED OR DRILLED. NO HOLE BURNING SHALL BE ALLOWED.



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 587-8856
Fax: (845) 587-8705

OMNIPONT COMMUNICATIONS INC.
An Agent For
OMNIPONT FACILITIES NETWORK 2, LLC

OMNIPONT WIRELESS
75 PROGRESS DRIVE, 2ND FLOOR
STAMFORD, CT 06903
OFFICE (203) 328-8900
FAX (203) 328-8953

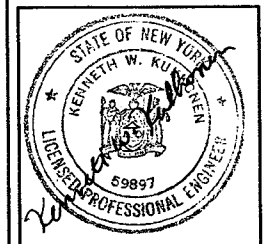
APPROVALS

OWNER: _____
LANDLORD: _____
IF: _____
CONSTRUCTION: _____

PROJECT NUMBER: 2356.0215A DESIGNED BY: EP

REV	DATE	REVISION	DRAWN BY
Δ	4/18/03	FOR COMMENT	JFL
Δ	6/5/03	FOR PERMIT	JFL
Δ	7/24/03	REVISED ANTENNAS	CA
Δ	8/28/03	PER COMMENT	CC
Δ	11/10/03	FOR CONSTRUCTION	EPK

ISSUED BY: E. PETERSZ DATE: 11-10-03



SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

STEEL NOTES

SHEET NUMBER

S-1

CONCRETE NOTES

1. DESIGN AND CONSTRUCTION OF ALL CONCRETE SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318.
2. ULTIMATE COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE 3000 PSI.
3. CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C150 - TYPE I OR II.
4. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60, "DEFORMED AND PLAIN BILLET STEEL BARS FOR CONCRETE REINFORCEMENT".
5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, "WELDED STEEL WIRE FABRIC FOR CONCRETE REINFORCEMENT".
6. CONCRETE WORK AND MATERIALS SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS", ACI 301.
7. SUBMIT CONCRETE MIX DESIGN TO THE DESIGN ENGINEER FOR APPROVAL NOT LESS THAN 3 DAYS PRIOR TO CONSTRUCTION. MIX DESIGN SHALL BE APPROVED BY THE ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
8. READY MIX CONCRETE SHALL COMPLY WITH ACI 304 AND ASTM C94 WITH A MAXIMUM WATER-CEMENT RATIO OF 0.50. TIME BETWEEN INTRODUCTION OF WATER AND THE PLACEMENT OF CONCRETE SHALL NOT EXCEED 1-1/2 HOURS.
9. CONCRETE AGGREGATES SHALL BE NORMAL WEIGHT, CONFORMING TO ASTM C33. MAXIMUM SIZE OF COARSE AGGREGATE SHALL BE 1".
10. CHLORIDE-CONTAINING ADMIXTURES SHALL NOT BE USED.
11. CONCRETE SLUMP SHALL NOT EXCEED 5 INCHES UNLESS SPECIFICALLY AUTHORIZED BY THE ENGINEER. SLUMP SHALL BE DETERMINED IN ACCORDANCE WITH ASTM C143.
12. PROVIDE AIR ENTRAINMENT IN EXTERIOR EXPOSED CONCRETE TO OBTAIN TOTAL AIR CONTENT OF 5% ± 1% IN ACCORDANCE WITH ACI 301.
13. FOR CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH, CONCRETE COVER FOR REINFORCING SHALL BE: 3" FOR ALL BARS
2" FOR #6 AND LARGER BARS
1-1/2" FOR #5 AND SMALLER BARS OR WIRE FABRIC
14. LAP SPLICES FOR REINFORCING SHALL BE 40 BAR DIAMETERS, UNLESS OTHERWISE NOTED. STANDARD HOOKS SHALL BE 16 BAR DIAMETERS. ALL OTHER REINFORCING DETAILS SHALL BE IN ACCORDANCE WITH ACI 315.
15. WELDING OF REINFORCING STEEL OR THE APPLICATION OF HEAT TO FACILITATE BENDING IS SPECIFICALLY PROHIBITED.
16. ALL REINFORCING, ANCHOR BOLTS, DOWELS, EMBEDDED STEEL, INSERTS AND ALL OTHER EMBEDDED ITEMS AND FORMED DETAILS SHALL BE IN PLACE BEFORE START OF CONCRETE PLACEMENT.
17. PROVIDE A 3/4" CHAMFER AT ALL EXPOSED EDGES OF CONCRETE, UNLESS OTHERWISE NOTED.
18. PROVIDE NOT LESS THAN 48 HOURS NOTICE TO THE FIELD REPRESENTATIVE PRIOR TO PLACEMENT OF CONCRETE.
19. WHEN AMBIENT TEMPERATURE IS BELOW 50 DEGREES F, CONCRETE MATERIALS AND PLACEMENT SHALL CONFORM TO THE RECOMMENDATIONS OF ACI 306R "COLD WEATHER CONCRETING".
20. WHEN AMBIENT TEMPERATURE IS ABOVE 90 DEGREES F, CONCRETE MATERIALS AND PLACEMENT SHALL CONFORM TO THE RECOMMENDATIONS OF ACI 305R "HOT WEATHER CONCRETING".
21. REMOVE ALL LOOSE MATERIAL AND DEBRIS FROM COMPACTED SUBGRADE SURFACE PRIOR TO PLACING CONCRETE. CONCRETE SHALL NOT BE PLACED ON FROZEN SUBGRADE.
22. CONCRETE SHALL BE SUFFICIENTLY CONSOLIDATED BY VIBRATION TO REMOVE AIR VOIDS. VIBRATION SHALL BE IN ACCORDANCE WITH ACI 309 "STANDARD PRACTICE FOR CONSOLIDATION OF CONCRETE".
23. THE TOP OF ALL CONCRETE SURFACES SHALL BE TRUE AND LEVEL WITH A SMOOTH FLOAT FINISH, UNLESS OTHERWISE NOTED.
24. TESTING OF CONCRETE SHALL BE PERFORMED UNDER THE DIRECTION OF THE CONSTRUCTION MANAGER.
25. THROUGHOUT CONSTRUCTION, THE CONCRETE WORK SHALL BE ADEQUATELY PROTECTED AGAINST DAMAGE DUE TO EXCESSIVE LOADING, CONSTRUCTION EQUIPMENT, MATERIALS OR METHODS, ICE, RAIN, OR SNOW. PROTECT CONCRETE FROM EXCESSIVE HEAT AND FREEZING FOR NOT LESS THAN 14 DAYS.
26. DRYING OUT OF CONCRETE, ESPECIALLY DURING THE FIRST 24 HOURS, SHALL BE CAREFULLY GUARDED AGAINST. ALL SURFACES SHALL BE MOIST CURED OR PROTECTED USING A MEMBRANE CURING AGENT CONFORMING TO ASTM C309 APPLIED AS SOON AS FORMS ARE REMOVED. IF MEMBRANE CURING AGENT IS USED, EXERCISE CARE NOT TO DAMAGE SURFACE.
27. CONTRACTOR SHALL BRING TO THE IMMEDIATE ATTENTION OF THE CONSTRUCTION MANAGER ANY DEFECTS OR ERRORS IN THE WORK; PRIOR TO MAKING REPAIRS. CONTRACTOR SHALL OBTAIN PERMISSION FROM THE OMNIPOINT CONSTRUCTION MANAGER TO PATCH OR OTHERWISE REPAIR DEFECTS OTHER THAN MINOR HONEYCOMBING.
28. GROUT SHALL BE NON-METALLIC, NON-SHRINK, PREPACKAGED GROUT WITH A MINIMUM STRENGTH OF 5,000 PSI AT 28 DAYS, REQUIRING ONLY THE ADDITION OF WATER. GROUT SHALL BE FIVE STAR GROUT AS MANUFACTURED BY FIVE STAR PRODUCTS INC., FAIRFIELD, CT OR APPROVED EQUAL GROUT SHALL BE MIXED AND PLACED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS.



ORIGINAL SIZE IN INCHES

UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT

TECTONIC Engineering & Surveying
Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 347-6555
Fax: (845) 347-6703

OMNIPOINT COMMUNICATIONS INC.

An Agent For
OMNIPOINT FACILITIES NETWORK S. LLC
OMNIPOINT WIRELESS
75 HUGHES DRIVE, 2ND FLOOR FAX: (203) 338-8800
STAMFORD, CT 06902 (203) 338-8833

APPROVALS

OMNIPOINT
LANDLORD
RF
CONSTRUCTION

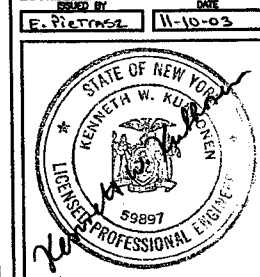
PROJECT NUMBER
2358/09215A

DESIGNED BY
EP

REV	DATE	REVISION	DESIGN BY
1	4/18/03	FOR COMMENT	JFL
2	6/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/10/03	FOR CONSTRUCTION	BFK

ISSUED BY
E. Pietroski

DATE
11-10-03



SITE INFORMATION

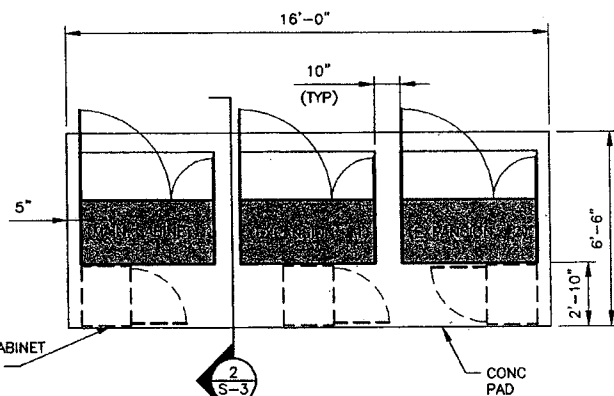
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

CONCRETE NOTES

SHEET NUMBER

S-2

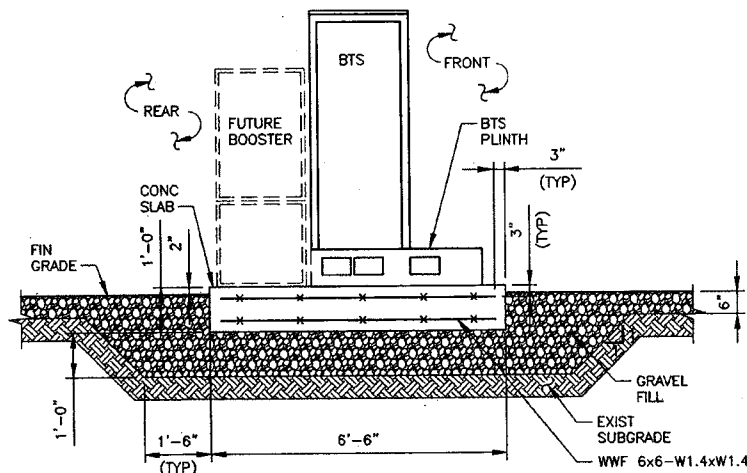


- NOTE:
1. COORDINATE ALL EQUIPMENT CABINET DIMENSIONS WITH MANUFACTURER.
 2. REFER TO SITE PLAN FOR EQUIPMENT ORIENTATION.

1
S-3

EQUIPMENT PLAN

SCALE: 1/4" = 1'-0"



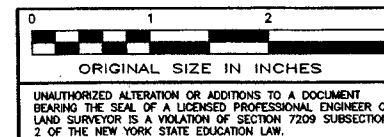
NOTE:

SLAB ON GRADE SHALL BE PLACED OVER A MIN OF 12" GRAVEL TO BE AASHTO NO. 57 OR MID CR-6 ON UNDISTURBED OR WELL COMPACTED SOIL WITH BEARING PRESSURE OF 2000 PSF MIN.

2
S-3

BTS FOUNDATION

SCALE: 3/8" = 1'-0"



TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

055 Little Britain Road
New Windsor, NY 12553
Phone: (845) 587-8858
Fax: (845) 587-8703

OMNIPONT COMMUNICATIONS INC

An Agent For
OMNIPONT FACILITIES NETWORK 2, LLC

OMNIPONT WIRELESS
75 PROGRESS BLVD, 2ND FLOOR
STAMFORD, CT 06903

OFFICE: (203) 358-8800
FAX: (203) 358-8803

APPROVALS			
OMNIPONT			
LANDLORD			
RF			
CONSTRUCTION			
PROJECT NUMBER	2308.00210A	DESIGNED BY	EP
REV	DATE	REVISION	DRAWN BY
Δ	4/18/03	FOR COMMENT	JFL
Δ	6/5/03	FOR PERMIT	JFL
Δ	7/24/03	REVISED ANTENNAS	CA
Δ	8/28/03	PER COMMENT	CC
Δ	11/10/03	FOR CONSTRUCTION	BPK

ISSUED BY
E. Pietras

DATE
11-10-03



SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

EQUIPMENT PLAN & DETAILS

SHEET NUMBER

S-3

ELECTRICAL NOTES

1. CONTRACTOR SHALL PERFORM ALL VERIFICATION OBSERVATION TESTS, AND EXAMINATION WORK PRIOR TO THE ORDERING OF THE ELECTRICAL EQUIPMENT AND THE ACTUAL CONSTRUCTION. CONTRACTOR SHALL ISSUE A WRITTEN NOTICE OF ALL FINDINGS TO THE ENGINEER LISTING ALL MALFUNCTIONS, FAULTY EQUIPMENT AND DISCREPANCIES.
2. CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIALS, INSURANCE, EQUIPMENT, INSTALLATION, CONSTRUCTION TOOLS, TRANSPORTATION, ETC., FOR A COMPLETE AND PROPERLY OPERATIVE SYSTEM ENERGIZED THROUGHOUT AND AS INDICATED ON DRAWINGS, AS SPECIFIED HEREIN AND/OR OTHERWISE REQUIRED.
3. ALL MATERIALS AND EQUIPMENT SHALL BE NEW AND IN PERFECT CONDITION WHEN INSTALLED AND SHALL BE OF THE BEST GRADE AND OF THE SAME MANUFACTURER THROUGHOUT FOR EACH CLASS OR GROUP OF EQUIPMENT. MATERIALS SHALL BE LISTED AND APPROVED BY UNDERWRITER'S LABORATORIES (U.L.). MATERIALS SHALL MEET WITH APPROVAL OF ALL GOVERNING BODIES HAVING JURISDICTION AND SHALL BE MANUFACTURED IN ACCORDANCE WITH APPLICABLE STANDARDS ESTABLISHED BY ANSI, NEMA AND NBFU.
4. CONTRACTOR TO COORDINATE WITH BUILDING OWNER FOR CONNECTION OF TEMPORARY AND PERMANENT POWER TO THE SITE. THE TEMPORARY POWER AND ALL HOOKUP COSTS TO BE PAID BY CONTRACTOR.
5. ALL ELECTRICAL EQUIPMENT SHALL BE LABELED WITH PERMANENT ENGRAVED PLASTIC LABELS.
6. ALL CIRCUIT BREAKERS, FUSES AND ELECTRICAL EQUIPMENT SHALL HAVE AN INTERRUPTING RATING NOT LESS THAN THE MAXIMUM SHORT CIRCUIT CURRENT TO WHICH THEY MAY BE SUBJECTED, AS INDICATED BY UTILITY COMPANY AND A MINIMUM OF 22,000 A.I.C.
7. METER SOCKET AMPERES, VOLTAGE, AND NUMBER OF PHASES SHALL BE AS NOTED AND SHALL BE MANUFACTURED BY SQUARE "D" COMPANY, SANGAMO, OR APPROVED EQUAL METER SOCKET SHALL BE APPROVED BY UTILITY COMPANY PRIOR TO INSTALLATION.
8. WIRE AND CABLE CONDUCTORS SHALL BE COPPER #12 AWG MINIMUM WITH TYPE THWN INSULATION UNLESS SPECIFICALLY NOTED OTHERWISE.
9. EACH CONDUCTOR OF EVERY SYSTEM SHALL BE PERMANENTLY TAGGED IN EACH PANEL BOARD, PULL BOX, J-BOX, SWITCH BOX, ECT., IN COMPLIANCE WITH THE OCCUPATIONAL SAFETY AND HEALTH ACT (O.S.H.A.)
10. CONDUIT:
 - A. RIGID CONDUIT SHALL BE U.L. LABEL GALVANIZED ZINC COATED WITH ZINC INTERIOR AND SHALL BE USED WHEN INSTALLED IN OR UNDER CONCRETE SLABS, IN CONTACT WITH THE EARTH, UNDER PUBLIC ROADWAYS, IN MASONRY WALLS OR EXPOSED ON BUILDING EXTERIOR.
 - B. ELECTRICAL METALLIC TUBING (EMT) SHALL HAVE U.L. LABEL. FITTINGS SHALL BE GLAND RING COMPRESSION TYPE. EMT SHALL BE USED ONLY FOR INTERIOR RUNS. NO SET SCREW OR CRIMP TYPE FITTINGS SHALL BE USED.
 - C. FLEXIBLE METALLIC CONDUIT SHALL HAVE U.L. LABEL AND MAY BE USED WHERE PERMITTED BY CODE. FITTINGS SHALL BE "JAKE" OR "SQUEEZE" TYPE, SEAL TIGHT FLEXIBLE CONDUIT. ALL CONDUIT IN EXCESS OF SIX FEET IN LENGTH SHALL HAVE FULL SIZE GROUND WIRE.
 - D. PVC CONDUIT AND FITTINGS SHALL HAVE U.L. LABEL AND SHALL BE PVC SCHEDULE 40.
 - E. CONDUIT SHALL BE SIZED PER NEC OR ANY GOVERNING LOCAL CODE, AND AS SHOWN.
 - F. CONDUIT RUNS MAY BE SURFACE MOUNTED IN CEILINGS OR WALLS UNLESS INDICATED OTHERWISE. CONDUIT INDICATED SHALL RUN PARALLEL OR AT RIGHT ANGLES TO CEILING, FLOOR OR BEAMS. VERIFY EXACT ROUTING OF ALL EXPOSED CONDUIT WITH OWNER PRIOR TO INSTALLING.
 - G. ALL CONDUIT ONLY (C.O.) RUNS SHALL HAVE A PULL WIRE OR ROPE.
11. REFER TO MANUFACTURERS MANUAL FOR RECOMMENDED FUSE AND WIRE SIZES.
12. ALL FINAL CONNECTIONS TO THE EQUIPMENT ARE TO BE OF FLEXIBLE WEATHERPROOF CONDUIT TO MEET APPLICABLE CODES. CONDUIT TO MEET APPLICABLE CODES.
13. PROVIDE ONE SET OF COMPLETE ELECTRICAL "AS INSTALLED" DRAWINGS AT THE COMPLETION OF JOB, SHOWING ACTUAL DIMENSIONS, ROUTINGS, AND CIRCUITS.
14. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH, GAINING APPROVALS AND PAYING ALL FEES ASSESSED BY UTILITY COMPANY FOR ELECTRICAL SERVICE.
15. SEAL PENETRATIONS THROUGH FIRE RATED AREAS WITH UL LISTED AND CODE APPROVED MATERIALS.
16. TELCO CONDUIT RUN IN/ON BUILDINGS SHALL BE 2" DIA. AND HAVE MINIMUM 12" RADIUS BENDS & ELBOWS. CONDUIT RUNS SHALL HAVE ONE 18"x18"x8" PULL BOX AT DEMARC LOCATION. TWO (2) PULL ROPES SHALL BE INSTALLED IN TELCO CONDUIT.
17. CONTRACTOR TO INSTALL BRASS TAGS ON THE COAX CABLE AT THE ANTENNA END & AT THE EQUIPMENT END. THE TAGS MUST INCLUDE SECTOR NO.# AND CABLE LENGTH.



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT

TECTONIC Engineering & Surveying
Consultants P.C.

935 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-8656
Fax: (845) 567-8703

OMNIPORT COMMUNICATIONS INC

An Agent For
OMNIPORT FACILITIES NETWORK 2, LLC

OMNIPORT WIRELESS 78 PROGRESS DRIVE, 2ND FLOOR, OFFICE: (505) 338-8800
SEAFORD, CT 06482 FAX: (505) 338-8803

APPROVALS

OMNIPORT
LANDLORD
RF
CONSTRUCTION

PROJECT NUMBER 2396.06215A DESIGNED BY EP

REV	DATE	REVISION	CHURN BY
Δ	4/16/03	FOR COMMENT	JFL
Δ	5/5/03	FOR PERMIT	JFL
Δ	7/24/03	REVISED ANTENNAS	CA
Δ	8/28/03	PER COMMENT	CC
Δ	11/10/03	FOR CONSTRUCTION	BFK

ISSUED BY E. Pietrasz DATE 11-10-03



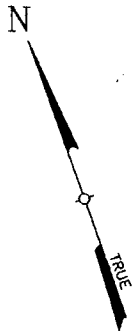
SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE ELECTRICAL NOTES

SHEET NUMBER

E-1



SAFE DIG

Before You Dig, Drill Or Blast!
- FOR PRIVATE PROPERTY -



1-800-825-9283

101 SHORE DRIVE,
NEW WINDSOR, NY 12553

MEMBERS OF NATIONAL UTILITY
LOCATING CONTRACTORS ASSOCIATION

DIG SAFELY - NEW YORK

Before You Dig, Drill Or Blast!

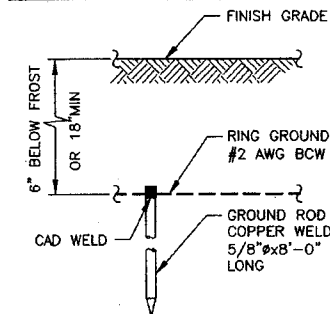


UNDERGROUND FACILITIES
PROTECTIVE ORGANIZATION
CALL US TOLL FREE 1-800-982-7962

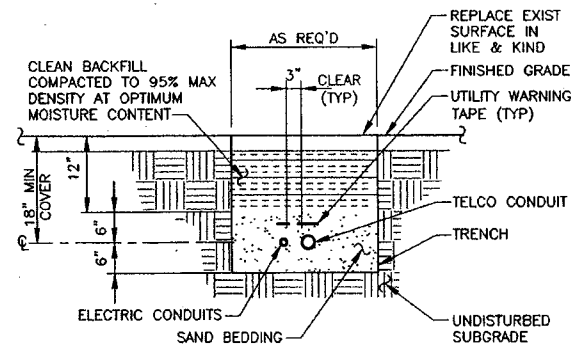
NY Industrial code rule 753 requires no
less than two working days notice, but
not more than ten days notice.

GROUNDING LEGEND

- 5/8"x8'-0" COPPER OR STAINLESS
STEEL COPPER CLAD GROUND ROD.
- ▲ CADWELD OR OTHER APPROVED
EXOTHERMIC WELDING SYSTEM.
- GROUND LUG
- GROUND CONDUCTOR

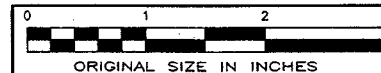


2
E-2
GROUND ROD DETAIL
SCALE: NTS



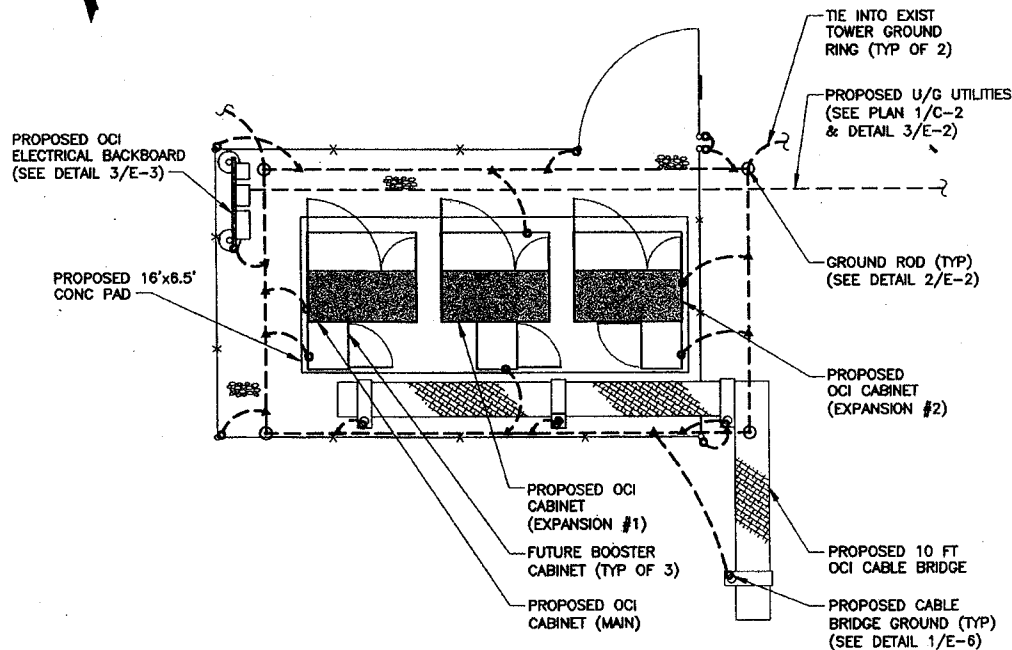
NOTE:
DETAIL IS APPLICABLE FOR ANY
NUMBER OF ELECTRIC CONDUITS.

3
E-2
TRENCH DETAIL
SCALE: 1/2" = 1'-0"



ORIGINAL SIZE IN INCHES

UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT
BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR
LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION
2 OF THE NEW YORK STATE EDUCATION LAW.



1
E-2
GROUNDING & ELECTRICAL PLAN
SCALE: 1" = 5'

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT
TECTONIC Engineering & Surveying
Consultants P.C.

855 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-8686
Fax: (845) 567-8703



As Agent For
OMNIPORT FACILITIES NETWORK S. LLC
OMNIPORT WIRELESS
78 PROGRESS DRIVE, 2ND FLOOR
BOSTON, MA 02108
OFFICE (203) 328-8800
FAX (203) 328-8853

APPROVALS
LANDLORD _____
PF _____
CONSTRUCTION _____

PROJECT NUMBER 235802215A
DESIGNED BY EP

REV	DATE	REVISION	DRAWN BY
1	4/16/03	FOR COMMENT	JFL
2	6/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/10/03	FOR CONSTRUCTION	BPK

ISSUED BY E. RETRASEZ
DATE 11-10-03



SITE INFORMATION
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE
GROUNDING &
ELECTRICAL PLAN

SHEET NUMBER

E-2

EXIST UTILITY COMPANY TRANSFORMER (IDENTIFIED AS V18514 F40U3A) LOCATED ABOUT 75' FROM THE PROPOSED OCI TOWER BASE SITE. THIS TRANSFORMER HAS BEEN INSTALLED FOR SPRINT. THE ADEQUACY TO BE CHECKED BY UTILITY COMPANY AND TO BE UPGRADED, IF NECESSARY.

200A, 208/120V, 1PH, 3 WIRE METER AS PER UTILITY CO. SPECS.

200A, 208/120V, 1PH, 3 WIRE (2 BLADES WITH FUSE HOLDERS, 1- S/N) HEAVY DUTY NEMA 3R ENCL FUSED DISCONNECT SW. SQUARE. D # H224N FUSE WITH BUSSMAN FUSES, # LPN-RK-200SP

(3) #3/0 THWN & (1) #6 THWN GND. IN 2" CONDUIT

(3) #3/0 THWN & (1) #6 THWN GND IN 2" CONDUIT

NOTE:

1. GROUNDING PER NEC AND LOCAL CODES.
2. UPGRADE OF EXIST TRANSFORMER WILL REQUIRE EXISTING ELECTRIC SERVICE TO SITE TO BE TEMPORARILY SHUTDOWN. OCI AND CONTRACTOR TO ARRANGE FOR TEMP POWER WITH EXISTING CARRIERS.
3. ELECTRICAL DOWNTILT CONTROL BOX WILL UTILIZE THE GFI OUTLET WHEN IN USE.

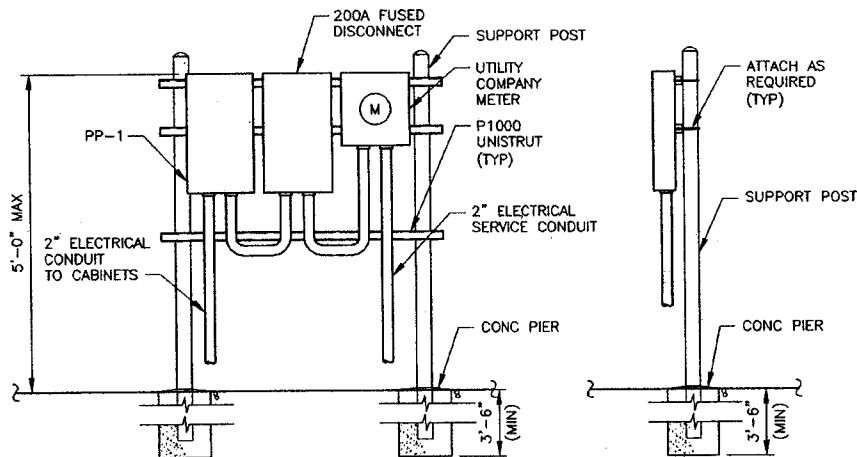
1 ONE-LINE DIAGRAM

SCALE: NTS

NEW PANEL PP-1 MAINS: 200A MAIN BREAKER 120/240V, 1-PHASE, 3-WIRE NEMA 3R SURFACE MOUNT, 16 POLES TOTAL						
LOAD	CONDUCTORS	C.B.	200A MAIN	C.B.	CONDUCTORS	LOAD
MAIN BTS	(3) #6 THWN & (1) #10 GND COND IN 1" RIGID STEEL CONDUIT	50	1 2 3 4	50	(3) #6 THWN & (1) #10 GND COND IN 1" RIGID STEEL CONDUIT	BTS EXPAN #2
BTS EXPAN #1	(3) #6 THWN & (1) #10 GND COND IN 1" RIGID STEEL CONDUIT	50	5 6 7	20	(2) #12 THWN & (1) #12 GND COND IN 3/4" RIGID STEEL	GFI BLANK

2 PANEL SCHEDULE

SCALE: NTS



NOTES:

1. ALL EQUIPMENT SHALL BE GROUNDED.
2. ALL ELECTRICAL ENCLOSURES TO BE NEMA 3R RATED.
3. SEE SITE PLAN FOR GENERAL ROUTING OF ALL CONDUITS. DETERMINE EXACT ROUTINGS IN FIELD.
4. ELECTRICAL EQUIPMENT SHALL BE MIN. 3'-0" FROM ANY STRUCTURE.
5. ALL COVERPLATES TO BE WEATHERPROOF.

3 ELECT/TELCO BACKBOARD

SCALE: NTS



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 587-8858
Fax: (845) 587-8703

OMNIPONT COMMUNICATIONS INC

An Agent For
OMNIPONT FACILITIES NETWORK 2, LLC

OMNIPONT WIRELESS 7th FLOOR OFFICE: (315) 328-8800
10 PROGRESS DRIVE FAX: (315) 328-8803
TEMPLETON, NY 13155

APPROVALS

OMNIPONT
LANDLORD
RF
CONSTRUCTION

PROJECT NUMBER 2306.09218A DESIGNED BY EP

REV	DATE	REVISION	DESIGNED BY
1	4/18/03	FOR COMMENT	JFL
2	6/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	CC
5	11/10/03	FOR CONSTRUCTION	EPK

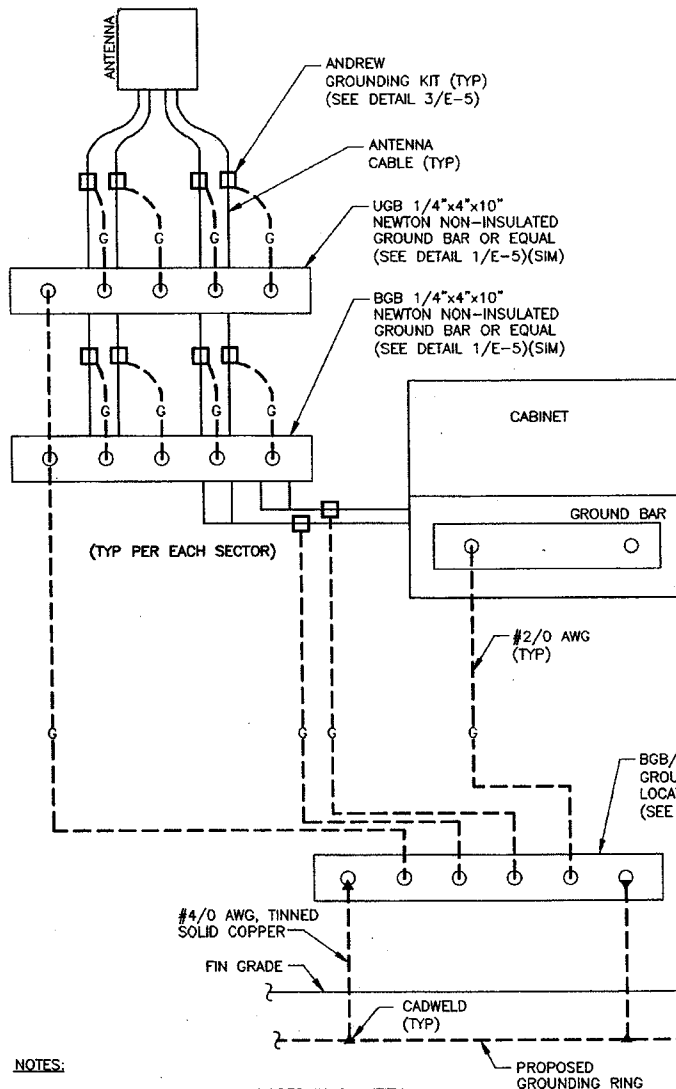
ISSUED BY E. Pfeiffer DATE 11-10-03



SITE INFORMATION
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE
ONE-LINE DIAGRAM

SHEET NUMBER
E-3



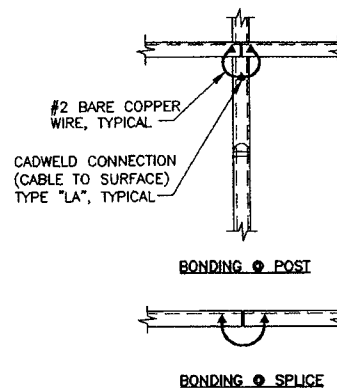
NOTES:

1. PROVIDE GROUNDING ELECTRODES IN QUANTITY, TYPE AND SIZE AS INDICATED ON THE GROUNDING PLAN.
2. ALL GROUNDING CONDUCTOR ABOVE GRADE SHALL BE #2 AWG, GREEN INSULATED UNLESS OTHERWISE NOTED.
3. ALL GROUNDING CONDUCTOR BELOW GRADE SHALL BE #2 AWG, TINNED, SOLID COPPER UNLESS OTHERWISE NOTED.

1
E-4

GROUNDING RISER DIAGRAM

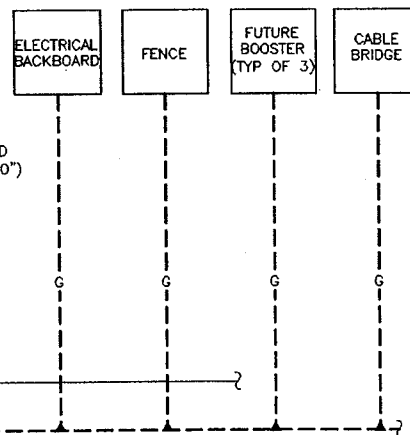
SCALE: NTS



2
E-4

COAX BRIDGE BONDING DETAIL

SCALE: NTS



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
• MANAGEMENT
TECTONIC Engineering & Surveying
Consultants P.C.

955 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-8656
Fax: (845) 567-8703

OMNIPORT COMMUNICATIONS INC

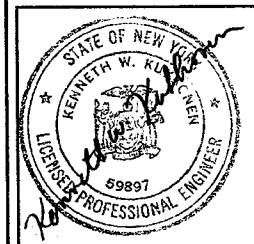
An Agent For
OMNIPORT FACILITIES NETWORK 2, LLC
OMNIPORT WIRELESS
75 PROGRESS DRIVE, 2ND FLOOR
STAMFORD, CT 06902
OFFICE: (203) 338-8900
FAX: (203) 338-8853

APPROVALS
OWNER: _____
LANDLORD: _____
RF: _____
CONSTRUCTION: _____

PROJECT NUMBER: 2398.09215A DESIGNED BY: EP

REV	DATE	REVISION	DRAWN BY
Δ	4/18/03	FOR COMMENT	JFL
Δ	6/5/03	FOR PERMIT	JFL
Δ	7/24/03	REVISED ANTENNAS	CA
Δ	8/28/03	PER COMMENT	CC
Δ	11/10/03	FOR CONSTRUCTION	BFK

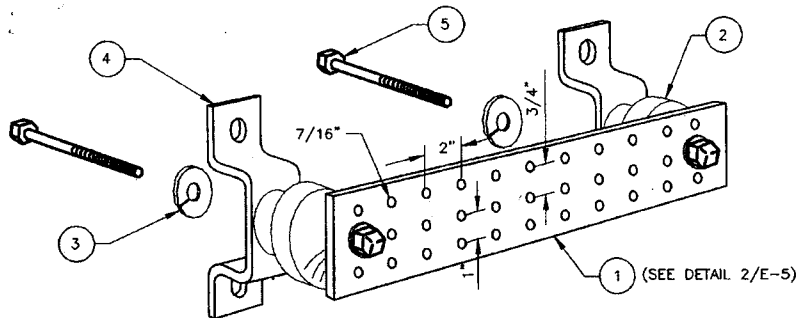
ISSUED BY: E. P. F. DATE: 11-10-03



SITE INFORMATION
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE
GROUNDING DIAGRAM

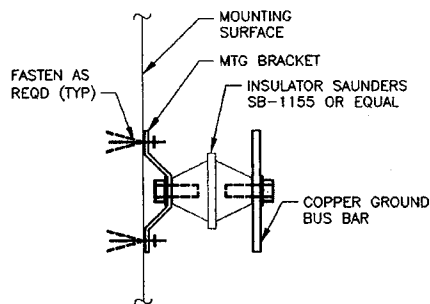
SHEET NUMBER
E-4



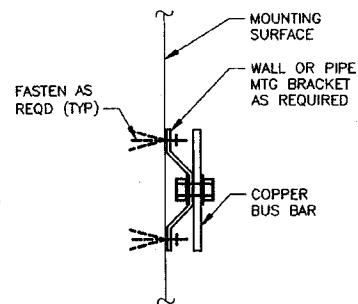
LEGEND

- 1- COPPER GROUND BAR, 1/4"x 4"x20 HOLE CENTERS TO MATCH NEMA DOUBLE LUG CONFIGURATION
- 2- INSULATORS
- 3- 5/8" LOCK WASHERS
- 4- WALL MOUNTING BRACKET
- 5- 5/8-11 x 1" H.H.C.S.BOLTS

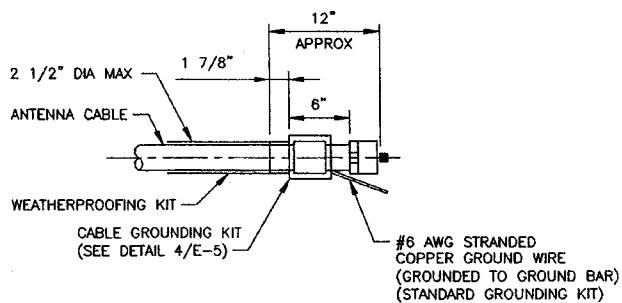
1 GROUNDING BAR DETAIL
SCALE: NTS



INSULATED GROUND BAR

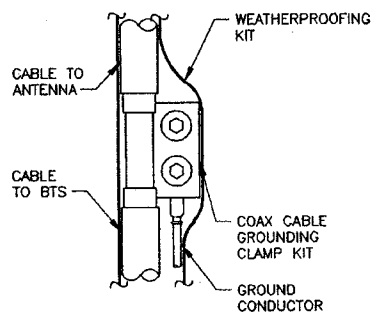


NON-INSULATED GROUND BAR

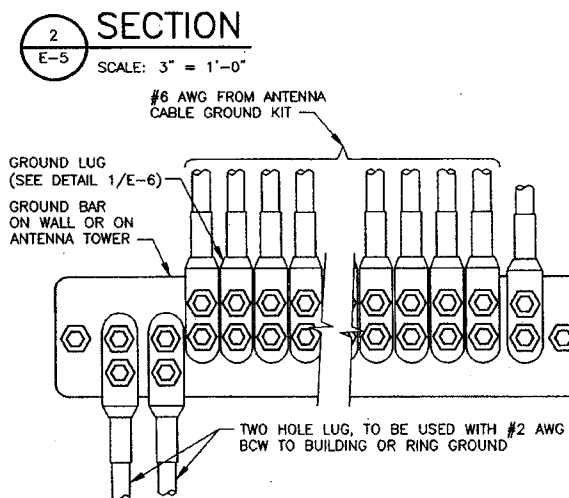


NOTE:
DO NOT INSTALL CABLE GROUND KIT AT A BEND AND
ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.

3 CABLE GROUND KIT DETAIL
SCALE: 1" = 1'-0"



4 CABLE GROUNDING DETAIL
SCALE: NTS



5 GROUNDING DETAIL
SCALE: NTS



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT
BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR
LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION
2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

- PLANNING
- ENGINEERING
- SURVEYING
- CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

935 Little Britain Road
New Windsor, NY 12553
Phone: (845) 567-8656
Fax: (845) 567-8703

OMNIPORT COMMUNICATIONS INC.

An Agent For
OMNIPORT FACILITIES NETWORK & LLC

OMNIPORT WIRELESS 75 PROGRESS DRIVE, 2ND FLOOR
SIMPSON, CT 06060

OFFICE: (203) 338-8800
FAX: (203) 338-8853

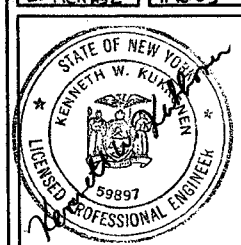
APPROVALS

OMNIPORT: _____
LANDLORD: _____
IF: _____
CONSTRUCTION: _____

PROJECT NUMBER: 2356.08215A
DESIGNED BY: EP

REV	DATE	REVISION	OWNED BY
1	4/18/03	FOR COMMENT	JFL
2	8/5/03	FOR PERMIT	JFL
3	7/24/03	REVISED ANTENNAS	CA
4	8/28/03	PER COMMENT	DC
5	11/10/03	FOR CONSTRUCTION	BYK

ISSUED BY: E. Pictus
DATE: 11-10-03



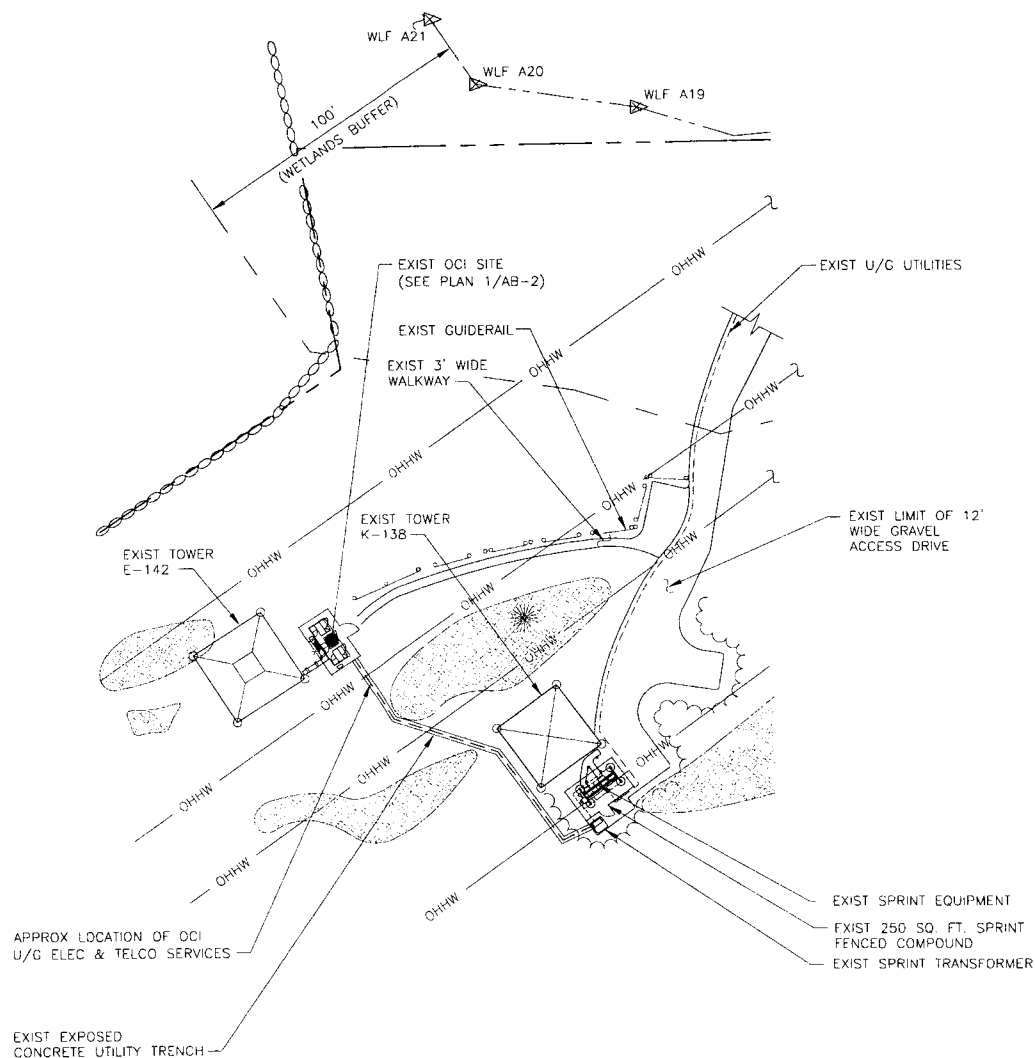
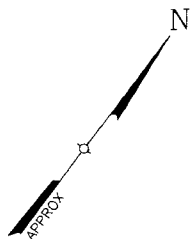
SITE INFORMATION

NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

GROUNDING DETAILS

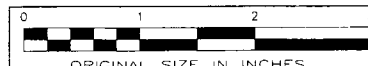
SHEET NUMBER

E-5



1
AB-1
PARTIAL SITE PLAN
SCALE: 1" = 40'

AS-BUILT



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

- PLANNING
- SURVEYING
- ENGINEERING
- CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

1279 ROUTE 300
NEWBURGH, NY 12550
Phone: (845) 567-6656
Fax: (845) 567-8703

OMNIPONT COMMUNICATIONS INC

OMNIPONT COMMUNICATIONS INC. PHONE (873) 282-4000
4 SYLVAN DRIVE
PARIS, NY 12454

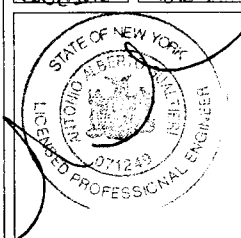
APPROVALS

OMNIPONT _____
LANDLORD _____
RF _____
CONSTRUCTION _____

PROJECT NUMBER 2356.09215A DESIGNED BY TN

REV	DATE	REVISION	DRAWN BY
1	01/06/09	AS BUILT	LH

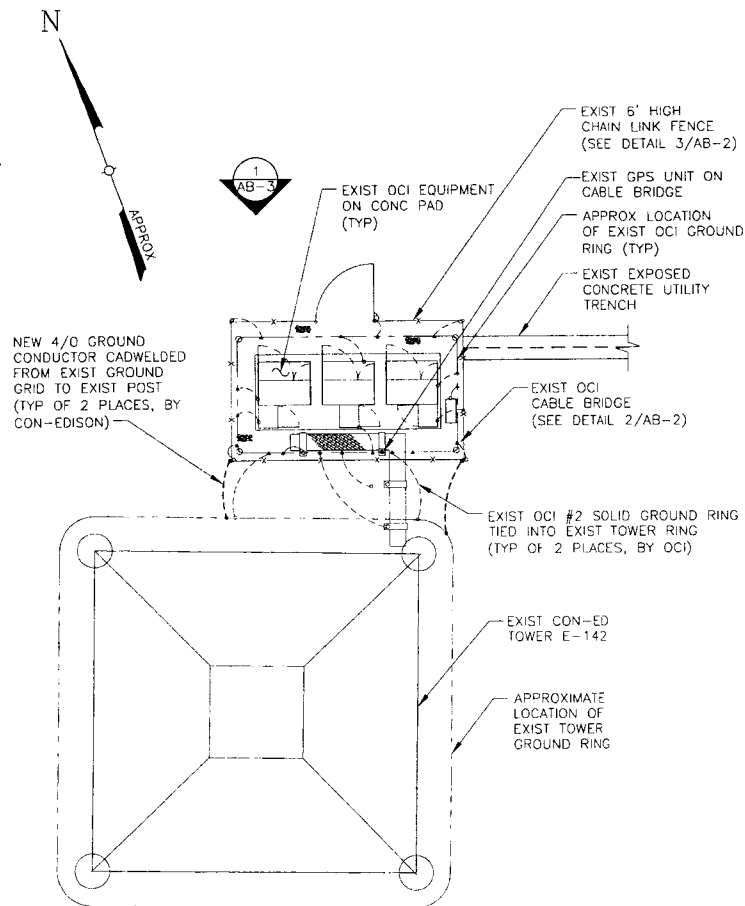
ISSUED BY: [Signature] DATE: 1/6/09



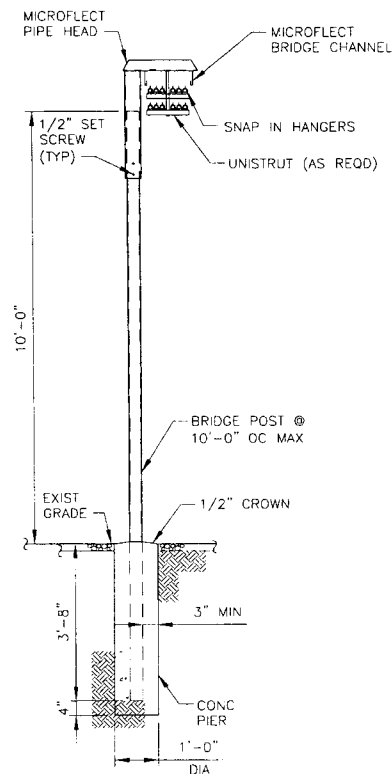
SITE INFORMATION
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE
PARTIAL SITE PLAN

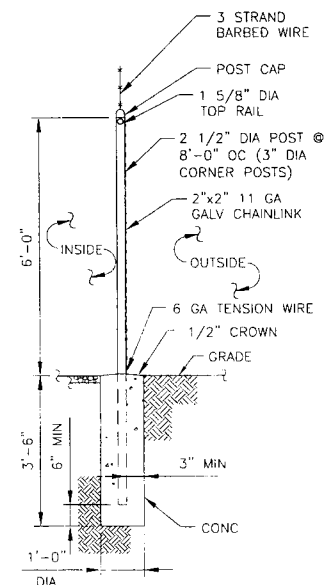
SHEET NUMBER
AB-1



1
AB-2
SITE DETAIL PLAN
SCALE: 1" = 10'

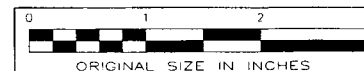


2
AB-2
CABLE BRIDGE
SCALE: 3/8" = 1'-0"



3
AB-2
FENCE DETAIL
SCALE: 3/8" = 1'-0"

AS-BUILT



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7209 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC
• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION
MANAGEMENT

TECTONIC Engineering & Surveying
Consultants P.C.

1279 ROUTE 300
NEWBURGH, NY 12550
Phone: (845) 567-6556
Fax: (845) 567-8703

OMNIPONT
COMMUNICATIONS INC

OMNIPONT COMMUNICATIONS INC PHONE: (873) 292-4000
4 SYLVAN DRIVE
PASICENT, NJ 07504

APPROVALS

OWNPOINT
LANDLORD
RF
CONSTRUCTION

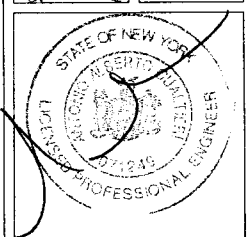
PROJECT NUMBER 2356.09215A DESIGNED BY TN

REV DATE REVISION DRAWN BY

01/08/09 AS BUILT LH

ISSUED BY DATE

1/15/09



SITE INFORMATION

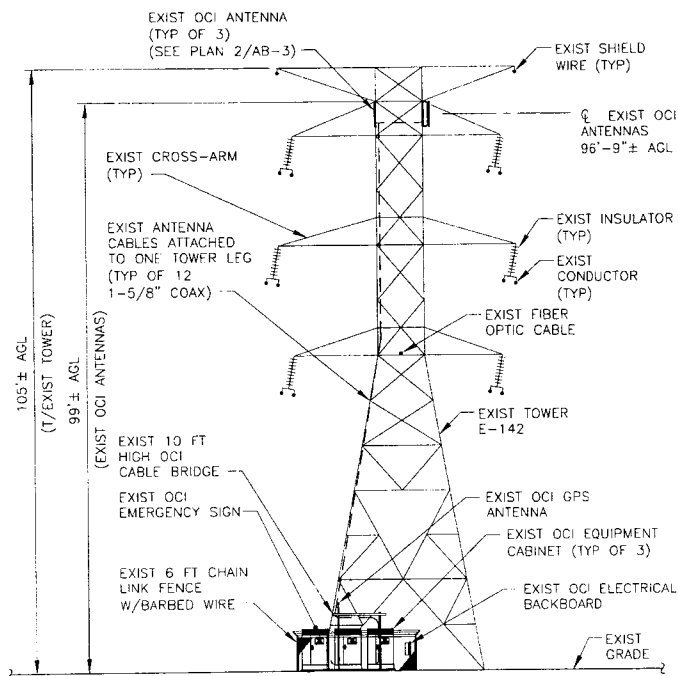
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE

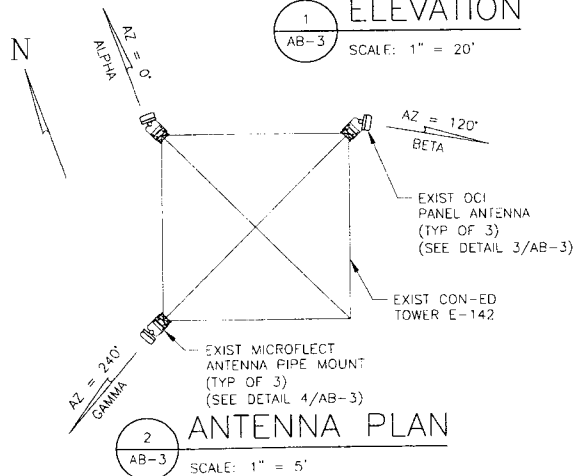
SITE DETAIL PLAN
AND GROUNDING PLAN

SHEET NUMBER

AB-2



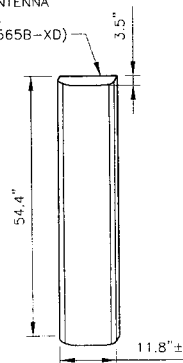
1 ELEVATION
AB-3 SCALE: 1" = 20'



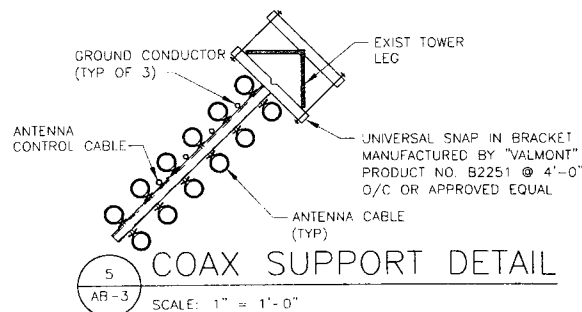
2 ANTENNA PLAN
AB-3 SCALE: 1" = 5'

THE ANTENNA INSTALLATION AS SHOWN REQUIRED NO ADDITIONAL REINFORCEMENT AS PER THE STRUCTURAL ANALYSIS PERFORMED BY TECTONIC ENGINEERING, DATED 12/19/2008, REVISION 1.

EXIST PANEL ANTENNA
ANDREW MODEL
(#ADFD1820-6565B-XD)



3 ANTENNA DETAIL
AB-3 SCALE: 1/2" = 1'-0"



5 COAX SUPPORT DETAIL
AB-3 SCALE: 1" = 1'-0"

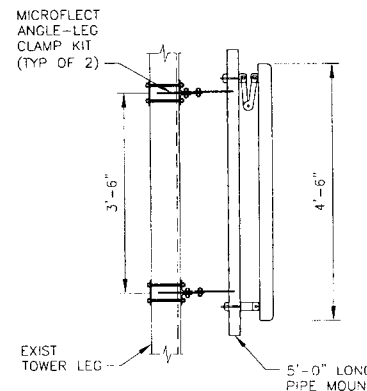
ANTENNA DATA

ANTENNA SPECIFICATIONS

SECTOR	MODEL	ANT. QTY	CABLE SIZE	CABLE QTY	AZIMUTH
ALPHA	ADFD1820-6565B-XD	1	1 5/8"	4	0°
BETA	ADFD1820-6565B-XD	1	1 5/8"	4	120°
GAMMA	ADFD1820-6565B-XD	1	1 5/8"	4	240°

ANDREW ANTENNA ADFD1820-6565B-XD

HEIGHT	54.4"
WIDTH	11.8"
DEPTH	3.5"
WEIGHT	34.5 LBS
WIND AREA	2.91 SF



4 ANTENNA ELEVATION
AB-3 SCALE: 1/2" = 1'-0"

AS-BUILT



UNAUTHORIZED ALTERATION OR ADDITIONS TO A DOCUMENT BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LAND SURVEYOR IS A VIOLATION OF SECTION 7709 SUBSECTION 2 OF THE NEW YORK STATE EDUCATION LAW.

TECTONIC

• PLANNING • SURVEYING
• ENGINEERING • CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

1279 ROUTE 300
NEWBURGH, NY 12550
Phone: (845) 567-6655
Fax: (845) 567-8703

OMNIPONT
COMMUNICATIONS INC

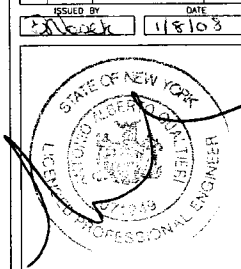
OMNIPONT COMMUNICATIONS INC PHONE: (873) 292-4000
4 STUYVENANT DRIVE
PASCATUNNY, NJ 07964

APPROVALS
OMNIPONT
LANDLORD
RF
CONSTRUCTION

PROJECT NUMBER 2356.09215A DESIGNED BY TN

REV DATE REVISION
01/08/08 AS BUILT LH

ISSUED BY DATE
11/8/08



SITE INFORMATION
NY-09-215-A
CON-ED TOWER E-142
STATION PLACE
TOWN OF
NEW CASTLE, NY

SHEET TITLE
ELEVATION & ANTENNA PLAN

SHEET NUMBER
AB-3

TECTONIC Engineering & Surveying Consultants P.C.
955 Little Britain Road
New Windsor, NY 12553

(845) 567-6656 FAX: (845) 567-8703
www.lectonicengineering.com

Joe Deker
Omnipoint Communications, Inc.
76 Progress Drive
Stamford, CT 06902

March 11, 2003

**RE: W.O. 2356.09215A
SITE: NY-09-215-A
CON-ED TOWER #E-142
STATION PLACE
NEW CASTLE, NY
STRUCTURAL CAPACITY VERIFICATION**

Mr. Deker:

Omnipoint Communications, Inc. is proposing a telecommunication site at the above referenced tower. The site includes installation of three (3) panel antennas attached to the existing electric transmission line tower. In addition, radio equipment cabinets will be located on a concrete pad at grade.

For the purpose of structural design of the equipment and antenna supports, the most stringent criteria of The Building Code of New York State and the ANSI/TIA/EIA-222-F-1996 "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures" have been applied.

The existing tower was checked against additional loading induced due to the proposed antenna mounts. The proposed installation will be designed so as not to diminish the structural integrity of the existing tower.

Should you have any questions, please do not hesitate to contact me.

Sincerely,
TECTONIC ENGINEERING AND SURVEYING CONSULTANTS P.C.

Kenneth W. Kukkonen

Kenneth W. Kukkonen, P.E.
Assistant Director of Telecommunications



G:\2356-VoiceStream (T-Mobile)\2356-09-215A Con-Ed #E-142\2356-09-215A-ContractDocuments\NW-2356-09-215A str ltr 3-11-03.doc

**T-MOBILE: CON EDISON TOWER E-142
W.O. 2356.09215A
SITE NO. NY-09-215-A
EXISTING TRANSMISSION TOWER
MILLWOOD, NY
STRUCTURAL ANALYSIS REPORT
FEBRUARY 27, 2003**

1.0 INTRODUCTION

The Consolidated Edison (Con Edison) electric transmission tower no. E-142 is located adjacent to Somerstown Turnpike (Route 100) at Station Place in Millwood, NY. The tower was designed as a part of the 345kV double-circuit transmission line system between the Sprain Brook and Millwood Substations. T-Mobile (aka Omnipoint) anticipates installing its antennas on this tower in the near future.

Tectonic Engineering & Surveying Consultants, P.C. has performed a structural analysis of the tower to verify its adequacy for supporting the proposed antennas in accordance with Con Edison requirements.

1.1 Information Provided

For the purpose of the analysis, we were provided with the following Con Edison drawings:

1. "Design Drawing, 25° Strain Tower Type "D1", Overhead Transmission Line, Dunwoodie to Millwood, N.Y.", drawing no. EO-7436-C, rev. 1, dated 3/7/67.
2. "Overhead Transmission Line, 1956 Line, 345 kV. Modification, Sprain Brook to Millwood Substations, Design of 2 Circuit, 25° Strain Tower, Type D-1", drawing no. EO-13575-B, rev. 1, dated 4/30/69.
3. "Typical Core Footing in Rock for Transmission Towers", drawing no. EO-7300-D, rev. 6, dated 4/17/56.
4. "Design & Construction Summary, Overhead Transmission Line, Sprain Brook to Millwood West Sub-Sta., 345 kV.-Rebuilt 1956 Line, Sheet 2 of 2", drawing no. EO-13610-A, rev. 5, dated 1/4/82.
5. "Jumper & Suspension Attachments, Assembly with Hardware, Bundled Conductors – 345 K.V.", drawing no. EO-13149-C, rev. 2, dated 2/19/69.
6. "Strain Attachment for Hot Line Operation, Assembly with Hardware, Bundled Conductors – 345 K.V.", drawing no. EO-13148-C, rev. 7, dated 3/30/71.

In addition to the above, we also retrieved the following relevant Con Edison documents from our files:

7. "Purchase and Test Manual No. 6 – Section 21, Lattice Steel Transmission Towers, EO-1018-1", dated 6/21/76.
8. "Con Edison Transmission System – P.C.S. Antenna/Cellular Installation Procedure" May 2000.

2.0 ORIGINAL TOWER DESIGN

2.1 Tower Structure

The existing tower E-142 was designed and fabricated by American Bridge based on Con Edison's standard specifications, and was designated as a Type D-1 25° strain tower. It was originally designed and built circa 1956, and was initially designated as tower #167. The tower was upgraded for double conductors circa 1969.

The tower is a four-legged, self-supporting square tower. It consists of a 89'-0" tall body and 16'-0" long extensions on all four (4) legs. It has a total height of 105'-0" from the base of the leg extensions to the top of the tower. Unless otherwise noted, levels throughout this report are relative to the base of the leg extensions.

The tower is approximately 27'-9" wide at the base. The tower tapers uniformly to a width of 8'-0" at the lower cross arm level, which is 55'-0" above the base. It has a uniform width of 8'-0" from the 55'-0" level to the top of the tower in the longitudinal and transverse faces. The lower, middle, and upper cross arms extend to total widths of 35'-8", 40'-8", and 35'-8", respectively. The shield wire peak has a total width of 40'-8".

The tower utilizes single angles for all members, except for the crossarm girts, which are flat bars. The arm hangers and girts for the conductor arms have been replaced with larger members. All connections are bolted.

A diagram of the structure is presented in Figure 1, attached.

2.2 Loading Criteria

The tower was originally designed based on Con Edison design specifications which provide tower strengths that equal or surpass the requirements of the National Electrical Safety Code for Grade "B" construction. The tower was designed to support two (2) 1/2" diameter EHS Copperweld shield wires and six (6) 1,033,500 CM 54/7 ACSR conductors, with a maximum line angle of 25° and an average span of 900'. The tower was designed to withstand loading

conditions of the original Con Edison specifications, including broken conductors and/or shield wire, with their appropriate overload factors.

We believe that at the time of the conductor upgrade, the tower was designed to support two (2) 7 No. 5 Alumoweld shield wires and six (6) twin-bundled 2493 MCM 54/37 ACAR conductors.

2.3 Tower Foundation

The Con Edison drawings indicate that the tower is supported on a rock foundation. The stub angles are encased in a reinforced concrete shaft, which bears at a maximum depth of 9'-0" below the rock surface.

3.0 EXISTING CONFIGURATION

According to the Con Edison drawings provided, the existing tower was installed at a line angle of 5°22', a wind span of 865', and a weight span of 1155', and is currently supporting the following:

Shield Wire:

9/16", 7 No. 5 AWG Alumoweld (2 positions)
Diameter = 0.546 in, Weight = 0.525 lb/ft

Conductor:

Twin Bundled 2493 KCM 54/37 ACAR (6 positions)
Diameter = 1.821 in, Weight = 2.341 lb/ft

It is believed that the tower centerline bisects the conductor line angle complement.

The tower is also supporting one (1) fiber optic cable, connected to the tower at the 55' level. This is believed to have been installed in 1999, and has the following properties:

Fiber Optic Cable:

288 Fiber ADSS (1 position)
Diameter = 1.042 in, Weight = 0.386 lb/ft

4.0 PROPOSED INSTALLATION

It is our understanding that T-Mobile is proposing to install the following items on the tower:

- 3 EMS DR65-18-02DP panel antennas at the 94' level (centerline), pipe mounted one (1) each on three (3) tower legs
- 12 1-5/8" diameter coaxial cables to the 94' level in Valmont-Microflex Universal Snap-In Brackets on the inside of one (1) tower leg

5.0 STRUCTURAL ANALYSIS

5.1 Current Loading Criteria

The design of electrical transmission line structures is governed by IEEE Standard C2-2002 "National Electrical Safety Code" (NESC) and ASCE 10-97 "Design of Latticed Steel Transmission Structures". Con Edison has imposed additional design constraints to accommodate local conditions and to account for special design considerations.

The loading conditions considered in our analysis of the tower are for the wind acting perpendicular (90°) to the longitudinal tower face acting in conjunction with the load due to the line angle. The loading conditions considered, including all applicable code requirements and the requirements stipulated by the utility, are summarized as follows:

Loading Condition (LC)	Pressure (psf)		Radial Ice (in)		Overload Factors		
	Wire	Tower	Wire	Tower	Wind	Weight	Wire Tension
1. Con Ed Standard	4	13	0.5	0	2.54	1.27	1.65
2. Con Ed Emergency Ice	8	20	1.0	0	1.00	1.00	1.00
3. Con Ed Hurricane	30	50	0	0	1.00	1.00	1.00
Note: Wind force on the tower is based on 1.5 times the net projected area of one face.							

The Con Edison Stringing & Sagging condition and Climbing condition were not evaluated in the analysis, because they are not affected by the proposed antenna installation.

The design of antenna supports is governed by ANSI/TIA/EIA-222-F-1996 "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures". From this document, a basic wind speed of 80 mph applies to Westchester County, NY where the tower is located. Ice loads have been established based on a 0.5" radial ice thickness in accordance with industry standard practice. A reduced wind speed of 69 mph is used in conjunction with this ice load.

The provisions of ANSI/TIA/EIA-222-F-1996 should be used for the design of the proposed antenna installation. However, the wind and ice loads stipulated by ANSI/TIA/EIA-222-F-1996 are overly conservative for the evaluation of the existing transmission line tower. Therefore, these loads were not considered in our analysis of the tower.

5.2 Procedure

The tower has been analyzed using PLS-Tower, a specialized, three-dimensional structural analysis program. The tower analysis included the existing conductors, shield wires, and fiber optic wire described in Section 3.0 and the proposed T-Mobile antennas and cables described in Section 4.0, using Con Edison loading criteria summarized above.

A breakdown of the total factored loads acting on the tower is provided in the following table:

LC	Conductor Bundle (lbs)			Shield Wire (lbs)		
	Weight	Transverse		Weight	Transverse	
		Wind	Wire Tension		Wind	Wire Tension
1	75690	25428	31800	3788	2606	2428
2	87066	26422	28086	5654	2936	2304
3	38448	47256	21348	1212	2362	1254

LC	Fiber Optic Cable (lbs)			Tower (lbs)		Proposed Installation (lbs)	
	Weight	Transverse		Weight	Wind	Weight	Wind
		Wind	Wire Tension				
1	2140	1680	1200	32640	13160	1330	3180
2	3380	1756	1148	25680	7980	1047	1956
3	444	2252	628	25680	19940	1047	4890

5.3 Assumptions

Several assumptions were made in order to perform the analysis. Each of these is considered by Tectonic to be both reasonable and consistent with current standards of practice.

1. Redundant members, which are not intended to carry loads, are not included in the model.
2. Tower member sizes and material properties are as indicated on the Con Edison drawings.
3. The wind loads applied to the tower due to the antenna installation are conservatively based on the full projected area of all antennas in all loading cases.

4. The tower and its foundation was constructed according to the approved plans, and the connection of the tower to its foundation is considered as pinned.
5. The conductor and shield wire tensions are assumed to be the same as those used in the design of similar towers upgraded circa 1969. The fiber optic cable tension is equal to the shield wire tension.

5.4 Results

Our analysis indicates that the Con Edison emergency ice loading condition (LC2) is the most severe, therefore resulting in maximum member forces in the tower. The member capacities are as indicated in the Con Edison drawings. Details of the critical member forces in terms of their stress ratios are summarized in the following table:

Member Location	Con Edison Member Number	See Note	Stress Ratio - Proposed Condition	
			Compression	Tension
			%	%
16'-0" Leg Ext.	7		57	32
	21		75	34
39'-0" Body	4		59	52
	5		57	33
	14		71	75
	15		44	48
	16		28	18
	17		25	26
	18		32	3
	19		46	31
19'-0" Mid Body	3		49	37
	13		51	50
	33		10	36
	45	1	—	67
19'-0" Upper Body	2		47	22
	12		53	51
	34		0	16
	44	1	—	75
12'-0" Body	1		53	3
	11		43	38
	32		0	13
	34		0	16
	43	1	0	29
Lower arm	27		46	0
	31	1	—	75
	48	1	—	72
Middle arm	26		44	0
	30	1	—	81
	47	1	—	78
Upper arm	25		39	0
	29	1	—	74
	46	1	—	72
SW peak	24		2	12
	28		73	0
Note: 1. Original member previously replaced with larger size in conjunction with conductor upgrade				

Details of all member forces are included in the computer output attached to this report.

The maximum foundation reactions occur for the Con Edison hurricane loading condition (LC3), and are summarized as follows:

<u>Direction</u>	<u>Calculated Reaction</u>
Uplift	110.4 kips
Download	144.0 kips
Shear (longitudinal)	25.8 kips
Shear (transverse)	25.9 kips

Although the original foundation design reactions have not been provided, we have estimated them from the upgraded design loads. The calculated reactions are smaller than the estimated upgraded design reactions.

6.0 CONCLUSIONS AND RECOMMENDATIONS

As a result of our analysis, we conclude that the tower has sufficient capacity to support the proposed installation. The tower member stresses are generally low. This is due mainly to the difference between the original design line angle of 25° and the existing line angle of 5°22', and the broken wire conditions that were considered in the original design, but were not included in this analysis. No structural problems for the tower are anticipated, and no modifications are necessary.

We further conclude that the existing foundation has sufficient capacity to support the proposed installation, based on comparison to the foundation reactions resulting from the design loads from the provided Con Edison drawings. No structural problems for the foundation are anticipated, and no modifications are necessary.

The clearance requirements for the proposed installation have not been evaluated in detail. However, the proposed antennas will be mounted directly on the tower legs and the cables will be installed inside the tower legs. Therefore, we expect that the proposed installation will have only a very minor effect on the existing conductor clearances.

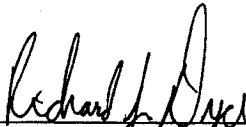
W.O. 2356.09215A / Millwood
Con Edison Tower E-142
Structural Analysis Report

Page 8

February 27, 2003

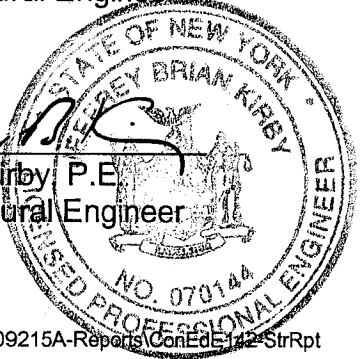
Any further changes to the antenna configuration or other appurtenances should be reviewed with respect to their effect on structural loads prior to implementation.

Prepared by:


Richard J. Dyer/E.I.T.
Staff Structural Engineer

Reviewed by:


Jeffrey B. Kirby P.E.
Chief Structural Engineer



Date: 2/27/03

T-MOBILE: CON EDISON TOWER E-142
W.O. 2356.09215A
SITE NO. NY-09-215-A
EXISTING TRANSMISSION TOWER
MILLWOOD, NY
STRUCTURAL ANALYSIS REPORT
FEBRUARY 27, 2003



~~T. MOBILE~~
~~E. 142~~

T-MOBILE: CON EDISON TOWER E-142
W.O. 2356.09215A
SITE NO. NY-09-215-A
EXISTING TRANSMISSION TOWER
MILLWOOD, NY
STRUCTURAL ANALYSIS REPORT
FEBRUARY 27, 2003

1.0 INTRODUCTION

The Consolidated Edison (Con Edison) electric transmission tower no. E-142 is located adjacent to Somerstown Turnpike (Route 100) at Station Place in Millwood, NY. The tower was designed as a part of the 345kV double-circuit transmission line system between the Sprain Brook and Millwood Substations. T-Mobile (aka Omnipoint) anticipates installing its antennas on this tower in the near future.

Tectonic Engineering & Surveying Consultants, P.C. has performed a structural analysis of the tower to verify its adequacy for supporting the proposed antennas in accordance with Con Edison requirements.

1.1 Information Provided

For the purpose of the analysis, we were provided with the following Con Edison drawings:

1. "Design Drawing, 25° Strain Tower Type "D1", Overhead Transmission Line, Dunwoodie to Millwood, N.Y.", drawing no. EO-7436-C, rev. 1, dated 3/7/67.
2. "Overhead Transmission Line, 1956 Line, 345 kV. Modification, Sprain Brook to Millwood Substations, Design of 2 Circuit, 25° Strain Tower, Type D-1", drawing no. EO-13575-B, rev. 1, dated 4/30/69.
3. "Typical Core Footing in Rock for Transmission Towers", drawing no. EO-7300-D, rev. 6, dated 4/17/56.
4. "Design & Construction Summary, Overhead Transmission Line, Sprain Brook to Millwood West Sub-Sta., 345 kV.-Rebuilt 1956 Line, Sheet 2 of 2", drawing no. EO-13610-A, rev. 5, dated 1/4/82.
5. "Jumper & Suspension Attachments, Assembly with Hardware, Bundled Conductors - 345 K.V.", drawing no. EO-13149-C, rev. 2, dated 2/19/69.
6. "Strain Attachment for Hot Line Operation, Assembly with Hardware, Bundled Conductors - 345 K.V.", drawing no. EO-13148-C, rev. 7, dated 3/30/71.

In addition to the above, we also retrieved the following relevant Con Edison documents from our files:

7. "Purchase and Test Manual No. 6 – Section 21, Lattice Steel Transmission Towers, EO-1018-1", dated 6/21/76.
8. "Con Edison Transmission System – P.C.S. Antenna/Cellular Installation Procedure" May 2000.

2.0 ORIGINAL TOWER DESIGN

2.1 Tower Structure

The existing tower E-142 was designed and fabricated by American Bridge based on Con Edison's standard specifications, and was designated as a Type D-1 25° strain tower. It was originally designed and built circa 1956, and was initially designated as tower #167. The tower was upgraded for double conductors circa 1969.

The tower is a four-legged, self-supporting square tower. It consists of a 89'-0" tall body and 16'-0" long extensions on all four (4) legs. It has a total height of 105'-0" from the base of the leg extensions to the top of the tower. Unless otherwise noted, levels throughout this report are relative to the base of the leg extensions.

The tower is approximately 27'-9" wide at the base. The tower tapers uniformly to a width of 8'-0" at the lower cross arm level, which is 55'-0" above the base. It has a uniform width of 8'-0" from the 55'-0" level to the top of the tower in the longitudinal and transverse faces. The lower, middle, and upper cross arms extend to total widths of 35'-8", 40'-8", and 35'-8", respectively. The shield wire peak has a total width of 40'-8".

The tower utilizes single angles for all members, except for the crossarm girts, which are flat bars. The arm hangers and girts for the conductor arms have been replaced with larger members. All connections are bolted.

A diagram of the structure is presented in Figure 1, attached.

2.2 Loading Criteria

The tower was originally designed based on Con Edison design specifications which provide tower strengths that equal or surpass the requirements of the National Electrical Safety Code for Grade "B" construction. The tower was designed to support two (2) 1/2" diameter EHS Copperweld shield wires and six (6) 1,033,500 CM 54/7 ACSR conductors, with a maximum line angle of 25° and an average span of 900'. The tower was designed to withstand loading

conditions of the original Con Edison specifications, including broken conductors and/or shield wire, with their appropriate overload factors.

We believe that at the time of the conductor upgrade, the tower was designed to support two (2) 7 No. 5 Alumoweld shield wires and six (6) twin-bundled 2493 MCM 54/37 ACAR conductors.

2.3 Tower Foundation

The Con Edison drawings indicate that the tower is supported on a rock foundation. The stub angles are encased in a reinforced concrete shaft, which bears at a maximum depth of 9'-0" below the rock surface.

3.0 EXISTING CONFIGURATION

According to the Con Edison drawings provided, the existing tower was installed at a line angle of 5°22', a wind span of 865', and a weight span of 1155', and is currently supporting the following:

Shield Wire:

9/16", 7 No. 5 AWG Alumoweld (2 positions)

Diameter = 0.546 in, Weight = 0.525 lb/ft

Conductor:

Twin Bundled 2493 KCM 54/37 ACAR (6 positions)

Diameter = 1.821 in, Weight = 2.341 lb/ft

It is believed that the tower centerline bisects the conductor line angle complement.

The tower is also supporting one (1) fiber optic cable, connected to the tower at the 55' level. This is believed to have been installed in 1999, and has the following properties:

Fiber Optic Cable:

288 Fiber ADSS (1 position)

Diameter = 1.042 in, Weight = 0.386 lb/ft

4.0 PROPOSED INSTALLATION

It is our understanding that T-Mobile is proposing to install the following items on the tower:

- 3 EMS DR65-18-02DP panel antennas at the 94' level (centerline), pipe mounted one (1) each on three (3) tower legs
- 12 1-5/8" diameter coaxial cables to the 94' level in Valmont-Microflex Universal Snap-In Brackets on the inside of one (1) tower leg

5.0 STRUCTURAL ANALYSIS

5.1 Current Loading Criteria

The design of electrical transmission line structures is governed by IEEE Standard C2-2002 "National Electrical Safety Code" (NESC) and ASCE 10-97 "Design of Latticed Steel Transmission Structures". Con Edison has imposed additional design constraints to accommodate local conditions and to account for special design considerations.

The loading conditions considered in our analysis of the tower are for the wind acting perpendicular (90°) to the longitudinal tower face acting in conjunction with the load due to the line angle. The loading conditions considered, including all applicable code requirements and the requirements stipulated by the utility, are summarized as follows:

Loading Condition (LC)	Pressure (psf)		Radial Ice (in)		Overload Factors		
	Wire	Tower	Wire	Tower	Wind	Weight	Wire Tension
1. Con Ed Standard	4	13	0.5	0	2.54	1.27	1.65
2. Con Ed Emergency Ice	8	20	1.0	0	1.00	1.00	1.00
3. Con Ed Hurricane	30	50	0	0	1.00	1.00	1.00
Note: Wind force on the tower is based on 1.5 times the net projected area of one face.							

The Con Edison Stringing & Sagging condition and Climbing condition were not evaluated in the analysis, because they are not affected by the proposed antenna installation.

The design of antenna supports is governed by ANSI/TIA/EIA-222-F-1996 "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures". From this document, a basic wind speed of 80 mph applies to Westchester County, NY where the tower is located. Ice loads have been established based on a 0.5" radial ice thickness in accordance with industry standard practice. A reduced wind speed of 69 mph is used in conjunction with this ice load.

The provisions of ANSI/TIA/EIA-222-F-1996 should be used for the design of the proposed antenna installation. However, the wind and ice loads stipulated by ANSI/TIA/EIA-222-F-1996 are overly conservative for the evaluation of the existing transmission line tower. Therefore, these loads were not considered in our analysis of the tower.

5.2 Procedure

The tower has been analyzed using PLS-Tower, a specialized, three-dimensional structural analysis program. The tower analysis included the existing conductors, shield wires, and fiber optic wire described in Section 3.0 and the proposed T-Mobile antennas and cables described in Section 4.0, using Con Edison loading criteria summarized above.

A breakdown of the total factored loads acting on the tower is provided in the following table:

LC	Conductor Bundle (lbs)			Shield Wire (lbs)		
	Weight	Transverse		Weight	Transverse	
		Wind	Wire Tension		Wind	Wire Tension
1	75690	25428	31800	3788	2606	2428
2	87066	26422	28086	5654	2936	2304
3	38448	47256	21348	1212	2362	1254

LC	Fiber Optic Cable (lbs)			Tower (lbs)		Proposed Installation (lbs)	
	Weight	Transverse		Weight	Wind	Weight	Wind
		Wind	Wire Tension				
1	2140	1680	1200	32640	13160	1330	3180
2	3380	1756	1148	25680	7980	1047	1956
3	444	2252	628	25680	19940	1047	4890

5.3 Assumptions

Several assumptions were made in order to perform the analysis. Each of these is considered by Tectonic to be both reasonable and consistent with current standards of practice.

1. Redundant members, which are not intended to carry loads, are not included in the model.
2. Tower member sizes and material properties are as indicated on the Con Edison drawings.
3. The wind loads applied to the tower due to the antenna installation are conservatively based on the full projected area of all antennas in all loading cases.

4. The tower and its foundation was constructed according to the approved plans, and the connection of the tower to its foundation is considered as pinned.
5. The conductor and shield wire tensions are assumed to be the same as those used in the design of similar towers upgraded circa 1969. The fiber optic cable tension is equal to the shield wire tension.

5.4 Results

Our analysis indicates that the Con Edison emergency ice loading condition (LC2) is the most severe, therefore resulting in maximum member forces in the tower. The member capacities are as indicated in the Con Edison drawings. Details of the critical member forces in terms of their stress ratios are summarized in the following table:

Member Location	Con Edison Member Number	See Note	Stress Ratio - Proposed Condition	
			Compression	Tension
			%	%
16'-0" Leg Ext.	7		57	32
	21		75	34
39'-0" Body	4		59	52
	5		57	33
	14		71	75
	15		44	48
	16		28	18
	17		25	26
	18		32	3
	19		46	31
	3		49	37
19'-0" Mid Body	13		51	50
	33		10	36
	45	1	—	67
19'-0" Upper Body	2		47	22
	12		53	51
	34		0	16
12'-0" Body	44	1	—	75
	1		53	3
	11		43	38
	32		0	13
	34		0	16
Lower arm	43	1	0	29
	27		46	0
	31	1	—	75
Middle arm	48	1	—	72
	26		44	0
	30	1	—	81
Upper arm	47	1	—	78
	25		39	0
	29	1	—	74
SW peak	46	1	—	72
	24		2	12
	28		73	0
Note: 1. Original member previously replaced with larger size in conjunction with conductor upgrade				

Details of all member forces are included in the computer output attached to this report.

The maximum foundation reactions occur for the Con Edison hurricane loading condition (LC3), and are summarized as follows:

<u>Direction</u>	<u>Calculated Reaction</u>
Uplift	110.4 kips
Download	144.0 kips
Shear (longitudinal)	25.8 kips
Shear (transverse)	25.9 kips

Although the original foundation design reactions have not been provided, we have estimated them from the upgraded design loads. The calculated reactions are smaller than the estimated upgraded design reactions.

6.0 CONCLUSIONS AND RECOMMENDATIONS

As a result of our analysis, we conclude that the tower has sufficient capacity to support the proposed installation. The tower member stresses are generally low. This is due mainly to the difference between the original design line angle of 25° and the existing line angle of $5^\circ 22'$, and the broken wire conditions that were considered in the original design, but were not included in this analysis. No structural problems for the tower are anticipated, and no modifications are necessary.

We further conclude that the existing foundation has sufficient capacity to support the proposed installation, based on comparison to the foundation reactions resulting from the design loads from the provided Con Edison drawings. No structural problems for the foundation are anticipated, and no modifications are necessary.

The clearance requirements for the proposed installation have not been evaluated in detail. However, the proposed antennas will be mounted directly on the tower legs and the cables will be installed inside the tower legs. Therefore, we expect that the proposed installation will have only a very minor effect on the existing conductor clearances.

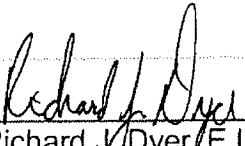
W.O. 2356.09215A / Millwood
Con Edison Tower E-142
Structural Analysis Report

Page 8

February 27, 2003

Any further changes to the antenna configuration or other appurtenances should be reviewed with respect to their effect on structural loads prior to implementation.

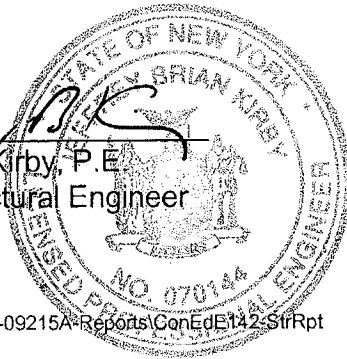
Prepared by:


Richard J. Dyer E.I.T.
Staff Structural Engineer

Reviewed by:


Jeffrey B. Kirby, P.E.
Chief Structural Engineer

Date: 2/27/03



G:\2356-T-Mobile\2356-09215A\2356-09215A-Reports\ConEdE142-StrRpt

W.O. 2356.09215A
KOTONA, NY

SUMMARY OF RESULTS

Project Name : Con Edison Tower E-142 (2356.09215A)
 Project Notes: Transmission Tower Analysis
 Project File : g:\2356-t-mobile\2356-09215a-analysis\conede142-twranalysis.tow
 Date run : 11:10:28 AM Friday, February 14, 2003
 by : Tower Version 6.02
 Licensed to : Tectonic Engineering

Successfully performed nonlinear analysis

Clamp insulator "100P" has a structure attach joint "100P" that is fixed with respect to translation ??
 The model has 1 warning. ??

Loads from file: g:\2356-t-mobile\2356-09215a\2356-09215a-analysis\twrloads(proposed).lca

*** Analysis Results:

Maximum element utilization is 80.91% for Angle "Arm2dY" in load case "ConEdEmerIce(int)"
 Maximum insulator utilization is 17.46% for Clamp "Arm2bP" in load case "ConEdEmerIce(int)"

Summary of Joint Support Reactions For All Loadcases:

Load Case	Joint Label	Long. Force (kips)	Tran. Force (kips)	Vert. Force (kips)	Shear Force (kips)	Tran. Moment (ft-k)	Long. Moment (ft-k)	Vert. Moment (ft-k)	Bending Moment (ft-k)
ConEdStd(int)	100P	-24.25	-24.27	135.34	34.31	0.00	0.00	0.00	0.00
ConEdStd(int)	100X	17.17	-18.03	-77.26	24.90	0.00	0.00	0.00	0.00
ConEdStd(int)	100XY	-16.26	-15.92	-73.32	22.76	0.00	0.00	0.00	0.00
ConEdStd(int)	100Y	23.34	-23.28	130.81	32.96	0.00	0.00	0.00	0.00
ConEdEmerIce(int)	100P	-22.64	-22.68	126.69	32.05	0.00	0.00	0.00	0.00
ConEdEmerIce(int)	100X	14.29	-14.47	-65.03	20.34	0.00	0.00	0.00	0.00
ConEdEmerIce(int)	100XY	-13.84	-11.27	-63.06	17.85	0.00	0.00	0.00	0.00
ConEdEmerIce(int)	100Y	22.19	-24.19	124.24	32.82	0.00	0.00	0.00	0.00
ConEdHurr(int)	100P	-25.78	-25.94	144.02	36.57	0.00	0.00	0.00	0.00
ConEdHurr(int)	100X	24.55	-25.92	-110.37	35.70	0.00	0.00	0.00	0.00
ConEdHurr(int)	100XY	-23.64	-23.29	-106.09	33.18	0.00	0.00	0.00	0.00
ConEdHurr(int)	100Y	24.87	-24.79	139.28	35.11	0.00	0.00	0.00	0.00

Summary of Joint Support Reactions For All Loadcases in Direction of Leg:

Load Case	Support Joint	Origin Joint	Leg Member	Force In Leg Dir. (kips)	Force To Leg Perpendicular (kips)
ConEdStd(int)	100P	101P	Legs1P	139.618	0.022
ConEdStd(int)	100X	101X	Legs1X	-81.004	5.164
ConEdStd(int)	100XY	101XY	Legs1XY	-76.661	4.039
ConEdStd(int)	100Y	101Y	Legs1Y	134.899	0.218
ConEdEmerIce(int)	100P	101P	Legs1P	130.677	0.090
ConEdEmerIce(int)	100X	101X	Legs1X	-68.036	3.723
ConEdEmerIce(int)	100XY	101XY	Legs1XY	-65.489	2.492
ConEdEmerIce(int)	100Y	101Y	Legs1Y	128.493	1.876
ConEdHurr(int)	100P	101P	Legs1P	148.596	0.117
ConEdHurr(int)	100X	101X	Legs1X	-115.759	7.518
ConEdHurr(int)	100XY	101XY	Legs1XY	-110.990	6.085
ConEdHurr(int)	100Y	101Y	Legs1Y	143.637	0.217

Overturning Moment Summary For All Load Cases:

Load Case	Transverse Moment (ft-k)	Longitudinal Moment (ft-k)	Resultant Moment (ft-k)
ConEdStd(int)	5779.872	8.128	5779.877
ConEdEmerIce(int)	5257.018	6.504	5257.022
ConEdHurr(int)	6931.712	6.396	6931.715

*** Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress

Group Summary (Compression Portion):

Group Label	Group Desc.	Angle Type	Angle Size	Steel Strength (ksi)	Max Usage %	Max Use In Comp. %	Comp. Control Member	Comp. Force (kips)	Comp. Control Load Case	L/R Capacity (kips)	Conn. Shear Capacity (kips)	Conn. Bearing Capacity (kips)	RLX	RLY	RLZ	L/R Length (ft)	Curve Comp.	No. Bolts	No. of Comp.
1	1	SAE	3.5X3.5X0.25	33.0	52.97	52.97	Legs6aY	-10.700	ConEdEmerI	20.200	0.000	0.000	1.000	1.000	1.000	103.75	6.000	1	0
2	2	SAE	6X6X0.375	33.0	46.95	46.95	Legs5aY	-40.474	ConEdStd(i	86.200	0.000	0.000	1.000	2.000	1.000	60.64	4.750	1	0
3	3	SAE	8X8X0.5	33.0	49.11	49.11	Legs4aY	-100.082	ConEdHurr(	203.800	0.000	0.000	1.000	2.000	1.000	45.60	4.750	1	0
4	4	SAE	8X8X0.5	33.0	58.83	58.83	Legs3aY	-137.418	ConEdHurr(	233.600	0.000	0.000	1.000	1.000	1.000	60.73	8.047	1	0

Group Summary (Tension Portion):																			
Group Label	Group Desc.	Angle Type	Angle Size	Steel Strength (ksi)	Max Usage (ksi)	Max Tens. (ksi)	Tension Control Member	Tension Force (kips)	Section Capacity (kips)	Net Tens. Capacity (kips)	Shear Capacity (kips)	Bearing Capacity (kips)	Rupture Capacity (kips)	Conn. Member Tens. (ft.)	Length No. Of Holes				
5	SAE	8X8X0.75		33.0	56.72	56.72	Legs2AP	-147.017	ConEdHurr	359.200	0.000	0.000	0.000	1.000	1.000	122.24	16.095	4	0
7	SAE	8X8X0.75		33.0	56.85	56.85	Legs1P	-148.026	ConEdHurr	260.400	0.000	0.000	0.000	1.000	1.000	125.37	16.507	4	0
11	SAE	2X2X0.1875		33.0	43.36	43.36	Dial1B	-3.989	ConEdHurr	9.200	0.000	0.000	0.000	0.750	0.500	152.28	10.000	5	0
12	SAE	3X3X0.25		33.0	53.34	53.34	Dial2P	-12.625	ConEdHurr	23.700	0.000	0.000	0.000	0.750	0.500	125.88	12.420	5	0
13	SAU	4X3.5X0.3125		33.0	51.28	51.28	Dial2P	-12.625	ConEdHurr	44.300	0.000	0.000	0.000	0.750	0.500	102.08	12.420	2	0
14	SAE	3X3X0.25		33.0	74.61	70.77	Dial2B	-14.721	ConEdHurr	20.800	0.000	0.000	0.000	0.750	0.500	102.61	12.295	5	0
15	SAE	3X3X0.25		33.0	48.01	44.37	Dial2B	-6.655	ConEdHurr	15.000	0.000	0.000	0.000	0.750	0.500	124.61	14.548	5	0
16	SAE	3.5X3.5X0.25		33.0	28.03	28.03	Hor1BP	-3.560	ConEdHurr	12.700	0.000	0.000	0.000	1.000	1.000	235.16	13.600	6	0
17	SAE	3.5X3.5X0.25		33.0	26.24	25.15	Dial2A	-3.522	ConEdHurr	14.000	0.000	0.000	0.000	1.000	1.000	180.56	10.442	6	0
18	DAS	2.5X2X0.1875		33.0	32.34	32.34	Hor1BP	-5.822	ConEdHurr	18.000	0.000	0.000	0.000	1.000	2.000	164.00	8.200	6	0
19	SAU	3X2X0.25		33.0	45.91	45.91	Dial2B	-3.347	ConEdHurr	7.290	0.000	0.000	0.000	1.000	0.500	246.14	17.845	5	0
21	SAU	3X2X0.25		33.0	75.01	75.01	Dial2A	-3.985	ConEdHurr	5.313	0.000	0.000	0.000	1.000	0.500	294.74	21.369	5	0
24	SAU	6X3.5X0.3125		33.0	12.17	1.49	Hor1BP	-0.687	ConEdHurr	46.000	0.000	0.000	0.000	1.000	1.000	124.35	8.000	6	0
25	SAU	6X4X0.3125		33.0	38.48	38.48	Arm3BP	-22.625	ConEdHurr	58.800	0.000	0.000	0.000	1.000	0.500	100.16	14.724	3	0
26	SAU	6X4X0.3125		33.0	45.05	45.05	Arm2BP	-31.100	ConEdHurr	63.400	0.000	0.000	0.000	1.000	0.500	117.29	17.143	3	0
27	SAU	6X4X0.3125		33.0	45.63	45.63	Arm1BP	-27.380	ConEdHurr	60.000	0.000	0.000	0.000	1.000	0.500	200.33	18.163	3	0
28	SAU	3.5X2.5X0.25		33.0	73.23	73.23	Arm1BP	-4.687	ConEdHurr	6.400	0.000	0.000	0.000	1.000	1.000	487.96	15.899	5	0
29	SAE	2X2X0.25		33.0	74.36	0.00	Arm2BP	0.000	0.000	1.678	0.000	0.000	0.000	1.000	1.000	494.14	17.789	5	0
30	SAU	3X2X0.3125		33.0	80.91	0.00	Arm2BP	0.000	0.000	12.200	0.000	0.000	0.000	1.000	1.000	476.03	15.471	5	0
31	SAE	2.5X2.5X0.1875		33.0	75.12	0.00	IntDial1X	0.000	0.000	14.573	0.000	0.000	0.000	0.750	0.500	137.14	11.314	5	0
32	SAE	2.5X2.5X0.1875		33.0	12.68	0.00	IntDial1X	-2.116	ConEdHurr	22.157	0.000	0.000	0.000	0.750	0.500	128.56	11.314	2	0
33	SAU	3X3X0.1875		33.0	35.85	9.55	IntDial1P	0.000	0.000	23.269	0.000	0.000	0.000	0.750	0.500	100.16	14.724	3	0
34	SAU	3X2.5X0.25		33.0	16.21	0.00	IntDial8X	0.000	0.000	17.400	0.000	0.000	0.000	1.000	1.000	182.86	8.000	6	0
43	SAU	3X2.5X0.3125		33.0	28.62	0.00	Hor1BP	0.000	0.000	0.682	0.000	0.000	0.000	1.000	1.000	1064.30	8.000	6	0
44	BAR	5/16X3.75		33.0	74.93	0.00	Hor1BP	0.000	0.000	0.635	0.000	0.000	0.000	1.000	1.000	469.73	15.305	5	0
45	BAR	5/16X3.75		33.0	66.61	0.00	Hor1BP	0.000	0.000	1.801	0.000	0.000	0.000	1.000	1.000	476.75	17.163	5	0
46	SAE	2X2X0.25		33.0	71.67	0.00	Arm2BP	0.000	0.000	2.721	0.000	0.000	0.000	1.000	1.000	457.22	14.860	5	0
47	SAU	3X2X0.3125		33.0	71.69	0.00	Arm2BP	0.000	0.000	2.316	0.000	0.000	0.000	1.000	1.000	237.84	11.000	6	0
48	SAE	2X2X0.3125		33.0	71.69	0.00	Arm2BP	0.000	0.000	12.053	0.000	0.000	0.000	2.000	1.000	237.84	11.000	6	0
Hor1	Hor1	SAU	3.5X3X0.25		33.0	13.45	Hor1BP	-1.621	ConEdHurr	41.227	0.000	0.000	0.000	0.000	1.000	0.000	11.000	0	1.000

*** Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case	Maximum Usage %	Element Label	Element Type
ConEdStd(int)	71.02	Arm2dP	Angle
ConEdEmerIce(int)	80.91	Arm2dY	Angle
ConEdHurr(int)	74.61	Dia5bX	Angle

Summary of Insulator Usages:

Insulator Label	Insulator Type	Maximum Usage %	Load case Weight (lbs)
Arm4aP	Clamp	4.06	ConEdEmerIce(int)
Arm4bP	Clamp	4.15	ConEdEmerIce(int)
Arm3aP	Clamp	17.30	ConEdEmerIce(int)
Arm3bP	Clamp	17.37	ConEdEmerIce(int)
Arm2aP	Clamp	17.39	ConEdEmerIce(int)
Arm2bP	Clamp	17.46	ConEdEmerIce(int)
Arm1aP	Clamp	17.31	ConEdEmerIce(int)
Arm1bP	Clamp	17.37	ConEdEmerIce(int)
100P	Clamp	0.70	ConEdStd(int)
101P	Clamp	1.32	ConEdStd(int)
102P	Clamp	0.82	ConEdStd(int)
103P	Clamp	1.07	ConEdHurr(int)
104P	Clamp	0.48	ConEdStd(int)
105P	Clamp	1.83	ConEdStd(int)
105X	Clamp	1.69	ConEdEmerIce(int)
105XY	Clamp	1.69	ConEdEmerIce(int)
105Y	Clamp	1.53	ConEdEmerIce(int)
106P	Clamp	0.24	ConEdStd(int)
107P	Clamp	0.53	ConEdStd(int)
108P	Clamp	1.07	ConEdHurr(int)
109P	Clamp	0.18	ConEdStd(int)
110P	Clamp	0.32	ConEdStd(int)
111P	Clamp	0.69	ConEdHurr(int)
112P	Clamp	0.32	ConEdHurr(int)
112X	Clamp	0.69	ConEdHurr(int)
112XY	Clamp	0.69	ConEdHurr(int)
112Y	Clamp	0.32	ConEdHurr(int)

*** Weight of structure (lbs):
Weight of Angles*Section DLF: 25691.2
Total: 25691.2

*** End of Report

PROPOSED ANTENNAS AND CABLES



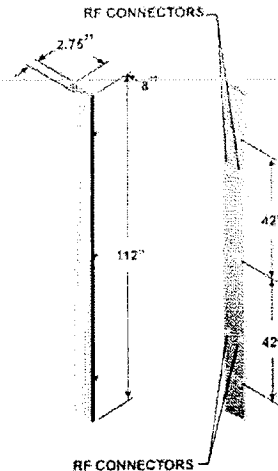
DR65-18-XXDP

Dual DualPol® Polarization
1850 MHz - 1990 MHz

OptiRange™

Electrical Specifications

Azimuth Beamwidth	65°
Elevation Beamwidth	6°
Gain	17.5 dBi (15.4 dBd)
Polarization	Quad Linear, Slant ($\pm 45^\circ$)
Port-to-Port Isolation (1 to 2 and 3 to 4)	≥ 30 dB
Port-to-Port Isolation (1 to 3 and 2 to 4)	≥ 30 dB
Front-to-Back Ratio	≥ 25 dB (≥ 30 dB Typ.)
Electrical Downtilt Options	0°, 2°
VSWR	1.35:1 Max
Connectors	4; 7-16 DIN (female)
Power Handling	250 Watts CW
Passive Intermodulation	≤ -150 dBc [2 x 20W (+ 43 dBm)]
Lightning Protection	Chassis Ground



Mechanical Specifications

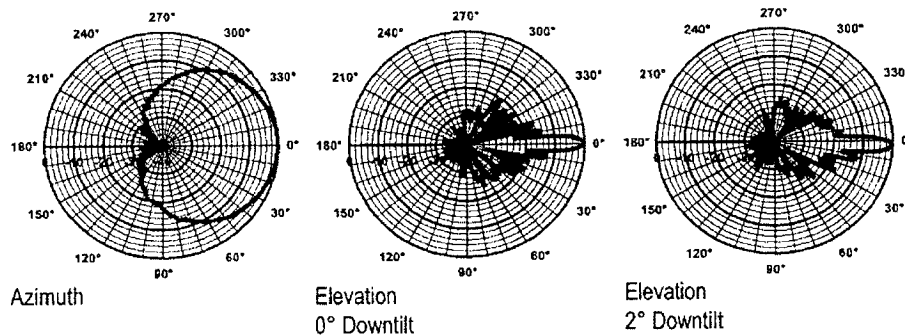
Dimensions (L x W x D)	112 in x 8 in x 2.75 in (284.5 cm x 20.3 cm x 7 cm)
Rated Wind Velocity	150 mph (241 km/hr)
Equivalent Flat Plate Area	6.2 ft ² (.58 m ²)
Front Wind Load @ 100 mph (161 kph)	179 lbs (796 N)
Side Wind Load @ 100 mph (161 kph)	61 lbs (274 N)
Weight	34 lbs (154 kg)

Mounting Options

MTG-P00-30, MTG-S02-30, MTG-DXX-30*, MTG-CXX-30*, MTG-C02-30, MTG-TXX-30*

Note: *Model number shown represents a series of products. See Mounting Options section for specific model number.

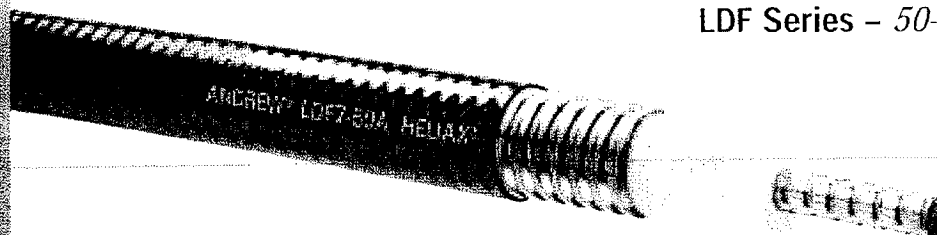
Patterns



Revised 04/05/02



LDF7-50A Coaxial Cables



1-5/8" Foam Dielectric, LDF Series – 50-ohm

LDF7-50A

Description	Type No.
Cable Ordering Information	
Standard Cable	
1-5/8" Standard Cable, Standard Jacket	LDF7-50A
Fire Retardant Cable	
1-5/8" Fire Retardant Jacket (CATVR)	LDF7RN-50A
Low VSWR and Specialized Cables	
1-5/8" Low VSWR, specify operating band	LDF7P-50A-(**)
** Insert suffix number from "Low VSWR Specifications" table, page 522.	
Characteristics	
Electrical	
Impedance, ohms	50 ± 1
Maximum Frequency, GHz	2.5
Velocity, percent	88
Peak Power Rating, kW	315
dc Resistance, ohms/1000 ft (1000 m)	
Inner	0.25 (0.83)
Outer	0.16 (0.52)
dc Breakdown, volts	11000
Jacket Spark, volts RMS	10000
Capacitance, pF/ft (m)	23.1 (75.8)
Inductance, µH/ft (m)	0.058 (0.190)
Mechanical	
Outer Conductor	Copper
Inner Conductor	Copper Tube
Diameter over Jacket, in (mm)	1.98 (50)
Diameter over Copper Outer Conductor, in (mm)	1.825 (46.3)
Diameter Inner Conductor, in (mm)	0.681 (17.3)
Nominal Inside Transverse Dimensions, cm	4.05
Minimum Bending Radius, in (mm)	20 (510)
Number of Bends, minimum (typical)	15 (50)
Bending Moment, lb-ft (N·m)	40 (54.2)
Cable Weight, lb/ft (kg/m)	0.82 (1.2)
Tensile Strength, lb (kg)	800 (363)
Flat Plate Crush Strength, lb/in (kg/mm)	120 (2.1)

Attenuation and Average Power

Frequency MHz	Attenuation dB/100 ft	Attenuation dB/100 m	Average Power, kW
0.5	0.014	0.044	247.0
1	0.019	0.063	175.0
1.5	0.024	0.077	142.0
2	0.027	0.089	123.0
10	0.062	0.202	54.3
20	0.088	0.289	38.1
30	0.109	0.356	30.9
50	0.142	0.465	23.6
88	0.191	0.627	17.5
100	0.205	0.671	16.4
108	0.213	0.699	15.7
150	0.254	0.834	13.2
174	0.276	0.904	12.2
200	0.297	0.976	11.3
300	0.372	1.22	9.01
400	0.437	1.43	7.67
450	0.467	1.53	7.18
500	0.496	1.63	6.76
512	0.503	1.65	6.67
600	0.550	1.81	6.09
700	0.602	1.97	5.57
800	0.650	2.13	5.15
824	0.662	2.17	5.06
894	0.694	2.28	4.83
950	0.724	2.38	4.63
1000	0.742	2.43	4.52
1250	0.848	2.78	3.95
1500	0.947	3.11	3.54
1700	1.02	3.35	3.28
2000	1.13	3.71	2.96
2300	1.23	4.05	2.72
2500	1.30	4.27	2.58

Standard Conditions:

For Attenuation, VSWR 1.0, ambient temperature 20°C (68°F), atmospheric pressure, dry air.

For Average Power, VSWR 1.0, inner temperature 100°C (212°F), ambient temperature 40°C (104°F), atmospheric pressure, dry air, no solar loading.

LOAD CALCULATIONS

CONDUCTOR & SHIELD WIRE LOAD CALCULATIONS
(Con Edison Tower E-142)

Loading Condition 1: Con Edison Standard - Intact - 5° 22'

Wind Pressure = 4.0 psf Radial Ice = 0.5 inch
Ice Density = 57 pcf

Overload Factors:

Vertical OLF = 1.27 Transverse OLF = 2.54
Wire Tension OLF = 1.65

Conductor Data:

Conductor Type: 2493 KCM 54/37 ACAR
Conductor Diameter: 1.821 in Weight: 2.341 lb/ft
Conductor Tension: 16900 lb (Back) Conductors/Bundle: 2 (North)
16900 lb (Ahead) 2 (South)
Wt. of Insulators, etc. 1000 lb

Shield Wire Data:

Shield Wire Type: 9/16", 7/5 A.W.G. Alumoweld
Shield Wire Dia.: 0.546 in Weight: 0.525 lb/ft
Shield Wire Tension: 7500 lb (Back)
7500 lb (Ahead)

Fiber Optic Cable Data:

Cable Type: 288 Fiber ADSS
Cable Dia.: 1.042 in Weight: 0.386 lb/ft
Cable Tension: 7500 lb (Back)
7500 lb (Ahead)

Design Spans and Angle:

Wind Span: 865 ft Weight Span: 1155 ft
Line Angle: 2.68 deg (Back)
2.68 deg (Ahead)

Force on Conductor:

Transverse Force = 9354 lb OR 10.814 lb/ft
Vertical Force = 12372 lb OR 10.711 lb/ft
Resultant Force = 15.221 lb/ft
NESC Constant (K Factor): 0.30 lb/ft
Final Transverse Force (Incl. NESC Constant) =
Final Vertical Force (Incl. NESC Constant) =
Final Longitudinal Force =

9538	lb
12615	lb
0	lb

Force on Shield Wire:

Transverse Force = 2291 lb OR 2.648 lb/ft
Vertical Force = 1724 lb OR 1.493 lb/ft
Resultant Force = 3.040 lb/ft
NESC Constant (K Factor): 0.30 lb/ft
Final Transverse Force (Incl. NESC Constant) =
Final Vertical Force (Incl. NESC Constant) =
Final Longitudinal Force =

2517	lb
1894	lb
0	lb

Force on Fiber Optic Cable:

Transverse Force = 2654 lb OR 3.068 lb/ft
Vertical Force = 1973 lb OR 1.708 lb/ft
Resultant Force = 3.512 lb/ft
NESC Constant (K Factor): 0.30 lb/ft
Final Transverse Force (Incl. NESC Constant) =
Final Vertical Force (Incl. NESC Constant) =
Final Longitudinal Force =

2881	lb
2141	lb
0	lb

CONDUCTOR & SHIELD WIRE LOAD CALCULATIONS

(Con Edison Tower E-142)

Loading Condition 2: Con Edison Emergency Ice - Intact - 5° 22'

Wind Pressure = 8.0 psf Radial Ice = 1.0 inch
Ice Density = 57 pcf

Overload Factors:

Vertical OLF = 1.00 Transverse OLF = 1.00
Wire Tension OLF = 1.00

Conductor Data:

Conductor Type: 2493 KCM 54/37 ACAR
Conductor Diameter: 1.821 in Weight: 2.341 lb/ft
Conductor Tension: 25000 lb (Back) Conductors/Bundle: 2 (North)
25000 lb (Ahead) 2 (South)
Wt. of Insulators, etc. 1000 lb

Shield Wire Data:

Shield Wire Type: 9/16", 7/5 A.W.G. Alumoweld
Shield Wire Dia.: 0.546 in Weight: 0.525 lb/ft
Shield Wire Tension: 12300 lb (Back)
12300 lb (Ahead)

Fiber Optic Cable Data:

Cable Type: 288 Fiber ADSS
Cable Dia.: 1.042 in Weight: 0.386 lb/ft
Cable Tension: 12300 lb (Back)
12300 lb (Ahead)

Design Spans and Angle:

Wind Span: 865 ft Weight Span: 1155 ft
Line Angle: 2.68 deg (Back)
2.68 deg (Ahead)

Force on Conductor:

Transverse Force = 9088 lb OR 10.507 lb/ft
Vertical Force = 14511 lb OR 12.564 lb/ft
Resultant Force = 16.378 lb/ft
NESC Constant (K Factor): 0.00 lb/ft

Final Transverse Force (Incl. NESC Constant) =

Final Vertical Force (Incl. NESC Constant) =

Final Longitudinal Force =

9088	lb
14511	lb
0	lb

Force on Shield Wire:

Transverse Force = 2620 lb OR 3.029 lb/ft
Vertical Force = 2827 lb OR 2.448 lb/ft
Resultant Force = 3.894 lb/ft
NESC Constant (K Factor): 0.00 lb/ft

Final Transverse Force (Incl. NESC Constant) =

Final Vertical Force (Incl. NESC Constant) =

Final Longitudinal Force =

2620	lb
2827	lb
0	lb

Force on Fiber Optic Cable:

Transverse Force = 2906 lb OR 3.359 lb/ft
Vertical Force = 3379 lb OR 2.925 lb/ft
Resultant Force = 4.455 lb/ft
NESC Constant (K Factor): 0.00 lb/ft

Final Transverse Force (Incl. NESC Constant) =

Final Vertical Force (Incl. NESC Constant) =

Final Longitudinal Force =

2906	lb
3379	lb
0	lb

CONDUCTOR & SHIELD WIRE LOAD CALCULATIONS

(Con Edison Tower E-142)

Loading Condition 3: Con Edison Hurricane - Intact - 5° 22'

Wind Pressure =	30.0 psf	Radial Ice =	0.0 inch
		Ice Density =	57 pcf

Overload Factors:

Vertical OLF =	1.00	Transverse OLF =	1.00
Wire Tension OLF =	1.00		

Conductor Data:

Conductor Type:	2493 KCM 54/37 ACAR		
Conductor Diameter:	1.821 in	Weight:	2.341 lb/ft
Conductor Tension:	19000 lb (Back)	Conductors/Bundle:	2 (North)
	19000 lb (Ahead)		2 (South)
Wt. of Insulators, etc.	1000 lb		

Shield Wire Data:

Shield Wire Type:	9/16", 7/5 A.W.G. Alumoweld		
Shield Wire Dia.:	0.546 in	Weight:	0.525 lb/ft
Shield Wire Tension:	6700 lb (Back)		
	6700 lb (Ahead)		

Fiber Optic Cable Data:

Cable Type:	288 Fiber ADSS		
Cable Dia.:	1.042 in	Weight:	0.386 lb/ft
Cable Tension:	6700 lb (Back)		
	6700 lb (Ahead)		

Design Spans and Angle:

Wind Span:	865 ft	Weight Span:	1155 ft
Line Angle:	2.68 deg (Back)		
	2.68 deg (Ahead)		

Force on Conductor:

Transverse Force =	11434 lb	OR	13.218 lb/ft
Vertical Force =	6408 lb	OR	5.548 lb/ft
Resultant Force =	14.335 lb/ft		
NESC Constant (K Factor):		0.00 lb/ft	
Final Transverse Force (Incl. NESC Constant) =			11434 lb
Final Vertical Force (Incl. NESC Constant) =			6408 lb
Final Longitudinal Force =			0 lb

Force on Shield Wire:

Transverse Force =	1808 lb	OR	2.090 lb/ft
Vertical Force =	606 lb	OR	0.525 lb/ft
Resultant Force =	2.155 lb/ft		
NESC Constant (K Factor):		0.00 lb/ft	
Final Transverse Force (Incl. NESC Constant) =			1808 lb
Final Vertical Force (Incl. NESC Constant) =			606 lb
Final Longitudinal Force =			0 lb

Force on Fiber Optic Cable:

Transverse Force =	2881 lb	OR	3.330 lb/ft
Vertical Force =	446 lb	OR	0.386 lb/ft
Resultant Force =	3.353 lb/ft		
NESC Constant (K Factor):		0.00 lb/ft	
Final Transverse Force (Incl. NESC Constant) =			2881 lb
Final Vertical Force (Incl. NESC Constant) =			446 lb
Final Longitudinal Force =			0 lb

LOAD CASE 1 - CON ED STANDARD												
Tower Loads												
Wind Pressure:	8	psf										
Flat Surface Shape Factor:	1.6											
Round Surface Shape Factor:	1.0											
Wind Overload Factor:	2.54											
Weight Overload Factor:	1.27											
Radial Ice:	0.50	inch										
Antenna Loads												
Antennas	Quantity	Net Weight Each (lb)	Total Weight (w/ OLF) (lb)	qz (psf)	Projected Area (ft^2)	z (ft)	Fx (ea) (lb)	Fx (total) (lb)	Fx per leg (lb)	Fy per leg (lb)	Total Moment (ft-kip)	
EMS DR65-18-02DP (Proposed T-Mobile)	3	34	130	8.00	6.20	94	201.6	605	151.2	32.4	56.8	
			130					605			56.8	
Cable Loads												
Cable	Quantity	Section	Section Height (ft)	Tributary Cable Length (ft)	qz (psf)	Projected Area (ea) (ft^2)	z (ft)	Fx (ea, w/OLF) (lb)	Fx (total) (lb)	Cable Weight (lb/ft)	Total Cable Weight (w/ OLF) (lb)	Total Moment (ft-kip)
(12) 1-5/8" coax (Note: assumed (8) 1-5/8" cables exposed to wind on one leg)	12	1	16.00	8.00	8.00	1.32	8.00	26.8	215	0.82	100	1.7
	12	2	23.40	19.70	8.00	3.25	27.70	66.1	528	0.82	246	14.6
	12	3	15.60	19.50	8.00	3.22	47.20	65.4	523	0.82	244	24.7
	12	4	19.00	17.30	8.00	2.85	64.50	58.0	464	0.82	216	29.9
	12	5	19.00	19.00	8.00	3.14	83.50	63.7	510	0.82	237	42.6
	12	6	6.00	12.50	8.00	2.06	96.00	41.9	335	0.82	156	32.2
	12	Top		3.00								
			99.0	99.0				2575			1200	145.7
Mount Loads												
Mounts	Quantity	Net Weight Each (lb)	Total Weight (w/ OLF) (lb)	qz (psf)	Projected Area (ft^2)	z (ft)	Fx (ea) (lb)	Fx (total) (lb)	Fx per leg (lb)	Fy per leg (lb)	Total Moment (ft-kip)	
			0					0			0.0	

LOAD CASE 2 - CON ED EMERGENCY ICE

Tower Loads

Wind Pressure:	12.5	psf
Flat Surface Shape Factor:	1.6	
Round Surface Shape Factor:	1.0	
Wind Overload Factor:	1.00	
Weight Overload Factor:	1.00	
Radial Ice:	1.00	inch

Antenna Loads

Antennas	Quantity	Net Weight Each (lb)	Total Weight (w/ OLF) (lb)	qz (psf)	Projected Area (ft^2)	z (ft)	Fx (ea) (lb)	Fx (total) (lb)	Fx per leg (lb)	Fy per leg (lb)	Total Moment (ft-kip)
EMS DR65-18-02DP (Proposed T-Mobile)	3	34	102	12.50	6.20	94	124.0	372	93.0	25.5	35.0
			102					372			35.0

Cable Loads

Cable	Quantity	Section	Section Height (ft)	Tributary Cable Length (ft)	qz (psf)	Projected Area (ea) (ft^2)	z (ft)	Fx (ea, w/OLF) (lb)	Fx (total) (lb)	Cable Weight (lb/ft)	Total Cable Weight (w/ OLF) (lb)	Total Moment (ft-kip)
(12) 1-5/8" coax (Note: assumed (8) 1-5/8" cables exposed to wind on one leg)	12	1	16.00	8.00	12.50	1.32	8.00	16.5	132	0.82	79	1.1
	12	2	23.40	19.70	12.50	3.25	27.70	40.6	325	0.82	194	9.0
	12	3	15.60	19.50	12.50	3.22	47.20	40.2	322	0.82	192	15.2
	12	4	19.00	17.30	12.50	2.85	64.50	35.7	285	0.82	170	18.4
	12	5	19.00	19.00	12.50	3.14	83.50	39.2	314	0.82	187	26.2
	12	6	6.00	12.50	12.50	2.06	96.00	25.8	206	0.82	123	19.8
	12	Top		3.00								
			99.0	99.0				1584			945	89.6

Mount Loads

Mounts	Quantity	Net Weight Each (lb)	Total Weight (w/ OLF) (lb)	qz (psf)	Projected Area (ft^2)	z (ft)	Fx (ea.) (lb)	Fx (total) (lb)	Fx per leg (lb)	Fy per leg (lb)	Total Moment (ft-kip)
			0					0			0.0

LOAD CASE 3 - CON ED HURRICANE

Tower Loads

Wind Pressure:	31.25	psf
Flat Surface Shape Factor:	1.6	
Round Surface Shape Factor:	1.0	
Wind Overload Factor:	1.00	
Weight Overload Factor:	1.00	
Radial Ice:	0.00	inch

Antenna Loads

Antennas	Quantity	Net Weight Each (lb)	Total Weight (w/ OLF) (lb)	qz (psf)	Projected Area (ft^2)	z (ft)	Fx (ea.) (lb)	Fx (total) (lb)	Fx per leg (lb)	Fy per leg (lb)	Total Moment (ft-kip)
EMS DR65-18-02DP (Proposed T-Mobile)	3	34	102	31.25	6.20	94	310.0	930	232.5	25.5	87.4
			102					930			87.4

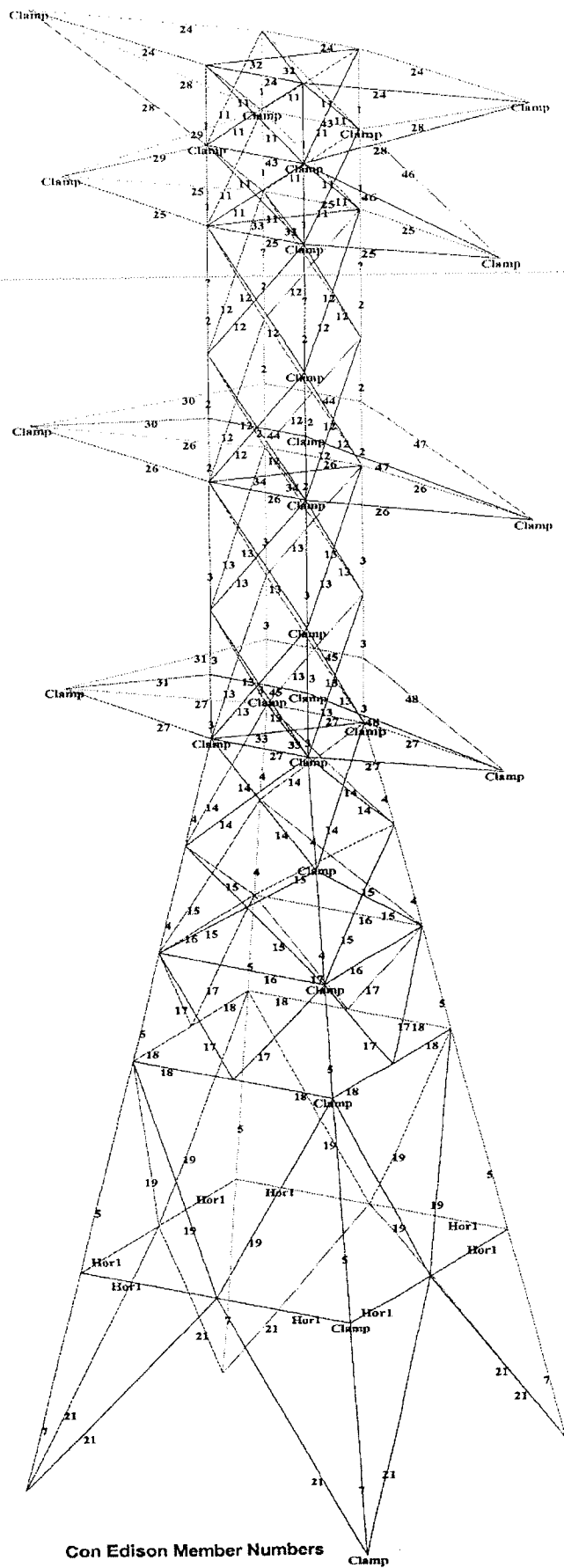
Cable Loads

Cable	Quantity	Section	Section Height (ft)	Tributary Cable Length (ft)	qz (psf)	Projected Area (ea) (ft^2)	z (ft)	Fx (ea, w/OLF) (lb)	Fx (total) (lb)	Cable Weight (lb/ft)	Total Cable Weight (w/ OLF) (lb)	Total Moment (ft-kip)
(12) 1-5/8" coax (Note: assumed (8) 1-5/8" cables exposed to wind on one leg)	12	1	16.00	8.00	31.25	1.32	8.00	41.3	330	0.82	79	2.6
	12	2	23.40	19.70	31.25	3.25	27.70	101.6	813	0.82	194	22.5
	12	3	15.60	19.50	31.25	3.22	47.20	100.5	804	0.82	192	38.0
	12	4	19.00	17.30	31.25	2.85	64.50	89.2	714	0.82	170	46.0
	12	5	19.00	19.00	31.25	3.14	83.50	98.0	784	0.82	187	65.4
	12	6	6.00	12.50	31.25	2.06	96.00	64.5	516	0.82	123	49.5
	12	Top		3.00								
				99.0		99.0			3960		945	224.1

Mount Loads

Mounts	Quantity	Net Weight Each (lb)	Total Weight (w/ OLF) (lb)	qz (psf)	Projected Area (ft^2)	z (ft)	Fx (ea.) (lb)	Fx (total) (lb)	Fx per leg (lb)	Fy per leg (lb)	Total Moment (ft-kip)
			0					0			0.0

TOWER ANALYSIS MODEL AND OUTPUT



Con Edison Member Numbers

```

*****
*   TOWER ANALYSIS AND DESIGN   - (C) 1986-2000 Power Line Systems, Inc.
*
*****

```

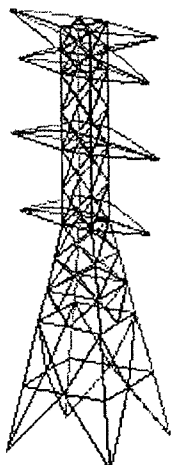
```

Project Name : Con Edison Tower E-142 (2356.09215A)
Project Notes: Transmission Tower Analysis
Project File : g:\2356-t-mobile\2356-09215a\2356-09215a-analysis\conede142-twranalysis.tow
Date run    : 11:10:27 AM Friday, February 14, 2003
by         : Tower Version 6.02
Licensed to : Tectonic Engineering

```

Successfully performed nonlinear analysis

Clamp insulator "100P" has a structure attach joint "100P" that is fixed with respect to translation ??
The model has 1 warning. ??



Nonlinear convergence parameters: Use Standard Parameters
Member check option: ASCE 10
Connection rupture check: Not Checked
Apply wind only to face: Yes

Joints Geometry:

Joint Label	Symmetry Code	X Coord (ft)	Y Coord (ft)	Z Coord (ft)	X Disp. Rest.	Y Disp. Rest.	Z Disp. Rest.	X Rot. Rest.	Y Rot. Rest.	Z Rot. Rest.
100P	XY-Symmetry	13.87	13.87	0	Fixed	Fixed	Fixed	Free	Free	Free
101P	XY-Symmetry	11	11	16	Free	Free	Free	Free	Free	Free
101aP	X-Symmetry	0	11	16	Free	Free	Free	Free	Free	Free
101bP	Y-Symmetry	11	0	16	Free	Free	Free	Free	Free	Free
102P	XY-Symmetry	8.2	8.2	31.6	Free	Free	Free	Free	Free	Free
102aP	X-Symmetry	0	8.2	31.6	Free	Free	Free	Free	Free	Free
102bP	Y-Symmetry	8.2	0	31.6	Free	Free	Free	Free	Free	Free
103P	XY-Symmetry	6.8	6.8	39.4	Free	Free	Free	Free	Free	Free
104P	XY-Symmetry	5.4	5.4	47.2	Free	Free	Free	Free	Free	Free
105P	XY-Symmetry	4	4	55	Free	Free	Free	Free	Free	Free
106P	XY-Symmetry	4	4	59.75	Free	Free	Free	Free	Free	Free
107P	XY-Symmetry	4	4	64.5	Free	Free	Free	Free	Free	Free
108P	XY-Symmetry	4	4	74	Free	Free	Free	Free	Free	Free
109P	XY-Symmetry	4	4	78.75	Free	Free	Free	Free	Free	Free
110P	XY-Symmetry	4	4	83.5	Free	Free	Free	Free	Free	Free
111P	XY-Symmetry	4	4	93	Free	Free	Free	Free	Free	Free
112P	XY-Symmetry	4	4	99	Free	Free	Free	Free	Free	Free
113P	XY-Symmetry	4	4	105	Free	Free	Free	Free	Free	Free
Arm1aP	None	0	17.5	55	Free	Free	Free	Free	Free	Free
Arm1bP	None	0	-18.17	55	Free	Free	Free	Free	Free	Free
Arm2aP	None	0	20	74	Free	Free	Free	Free	Free	Free
Arm2bP	None	0	-20.67	74	Free	Free	Free	Free	Free	Free
Arm3aP	None	0	17.5	93	Free	Free	Free	Free	Free	Free

Angle Type	Angle Size	Long Leg (in)	Short Leg (in)	Thick. (in)	Unit Weight (lbs/ft)	Gross Area (in ²)	w/t Ratio (Hyp. 1)	Radius of Gyration (in)	Radius of Gyration (in)	Radius of Gyration (in)	Number of Angles	Short Edge Dist. (in)	Long Section Edge Modulus (in ³)
SAE	8X8X0.75	8	8	0.75	39	11.4	8.8	2.47	2.47	1.58	1	8	0
SAE	8X8X0.5	8	8	0.5	26	7.75	14	2.5	2.5	1.59	1	8	0
SAE	6X6X0.375	6	6	0.375	15	4.36	14	1.88	1.88	1.19	1	6	0
SAE	3.5X3.5X0.25	3.5	3.5	0.25	5.8	1.69	12	1.09	1.09	0.694	1	3.5	0
SAE	3X3X0.25	3	3	0.25	4.9	1.44	9.8	0.93	0.93	0.592	1	3	0
SAE	3X3X0.1875	3	3	0.1875	3.7	1.09	13	0.939	0.939	0.596	1	3	0
SAE	2.5X2.5X0.1875	2.5	2.5	0.1875	3.1	0.902	11	0.778	0.778	0.495	1	2.5	0
SAE	2X2X0.3125	2	2	0.3125	3.9	1.15	3.8	0.601	0.601	0.391	1	2	0
SAE	2X2X0.25	2	2	0.25	3.2	0.94	5	0.609	0.609	0.391	1	2	0
SAE	2X2X0.1875	2	2	0.1875	2.4	0.71	8	0.617	0.617	0.394	1	2	0
SAU	6X4X0.375	6	4	0.375	12	3.61	14	1.93	1.17	0.877	1	6	0
SAU	6X4X0.3125	6	4	0.3125	10	3.03	17	1.94	1.17	0.882	1	6	0
SAU	6X3.5X0.3125	6	3.5	0.3125	9.8	2.87	17	1.95	0.996	0.772	1	6	0
SAU	4X3.5X0.3125	4	3.5	0.3125	7.7	2.25	10	1.26	1.07	0.73	1	4	0
SAU	3.5X3X0.25	3.5	3	0.25	5.4	1.56	11	1.11	0.914	0.631	1	3.5	0

SAU	3.5X2.5X0.25	3.5	2.5	0.25	4.9	1.44	11	1.12	0.735	0.544	1	3.5	0	0	0	0
SAU	3X2.5X0.3125	3	2.5	0.3125	5.6	1.62	7.1	0.937	0.744	0.535	1	3	0	0	0	0
SAU	3X2.5X0.375	3	2.5	0.375	4.5	1.31	9.5	0.945	0.753	0.538	1	3	0	0	0	0
SAU	3X2X0.3125	3	2	0.3125	4.3	1.16	7.6	0.948	0.567	0.432	1	3	0	0	0	0
SAU	3X2X0.375	3	2	0.375	4.1	1.13	9.8	0.957	0.574	0.435	1	3	0	0	0	0
DAS	2.5X2X0.1875	2.5	2	0.1875	5.5	1.62	11	0.6	1.24	0.6	2	2.5	0	0	0	0
BAR	5/16X3.75	3.8	0.31	0.3125	4	1.17	0	1.083	0.0502	0.0502	1	3.8	0	0	0	0
BAR	5/16X3.5	3.5	0.31	0.3125	3.7	1.09	0	1.101	0.0502	0.0502	1	3.5	0	0	0	0

Angle Groups:

Group Label Description Type Angle Material Element Group Optimize Allow. Add. Angle Width For Optimize (in)

1	1	SAE	3.5X3.5X0.25	A7	(33ksi)	Beam	Leg	None	0.000							
2	2	SAE	6X6X0.375	A7	(33ksi)	Beam	Leg	None	0.000							
3	3	SAE	8X8X0.5	A7	(33ksi)	Beam	Leg	None	0.000							
4	4	SAE	8X8X0.5	A7	(33ksi)	Beam	Leg	None	0.000							
5	5	SAE	8X8X0.75	A7	(33ksi)	Beam	Leg	None	0.000							
6	6	SAE	8X8X0.75	A7	(33ksi)	Beam	Leg	None	0.000							
7	7	SAE	2X2X0.1875	A7	(33ksi)	Truss	Other	None	0.000							
8	8	SAE	3X3X0.25	A7	(33ksi)	Truss	Other	None	0.000							
9	9	SAE	4X3.5X0.3125	A7	(33ksi)	Truss	Other	None	0.000							
10	10	SAE	3X3X0.25	A7	(33ksi)	Truss	Other	None	0.000							
11	11	SAE	3X3X0.25	A7	(33ksi)	Truss	Other	None	0.000							
12	12	SAE	3.5X3.5X0.25	A7	(33ksi)	Beam	Other	None	0.000							
13	13	SAE	2.5X2X0.1875	A7	(33ksi)	Beam	Other	None	0.000							
14	14	SAE	3X2X0.25	A7	(33ksi)	T-Only	Other	None	0.000							
15	15	SAE	3X2X0.25	A7	(33ksi)	T-Only	Other	None	0.000							
16	16	SAE	6X4X0.375	A7	(33ksi)	Beam	Other	None	0.000							
17	17	SAE	6X4X0.375	A7	(33ksi)	Beam	Other	None	0.000							
18	18	SAE	3.5X3.5X0.25	A7	(33ksi)	Beam	Other	None	0.000							
19	19	SAU	3X2X0.25	A7	(33ksi)	T-Only	Other	None	0.000							
20	20	SAU	3X2X0.25	A7	(33ksi)	T-Only	Other	None	0.000							
21	21	SAU	6X3.5X0.3125	A7	(33ksi)	Beam	Other	None	0.000							
22	22	SAU	6X4X0.375	A7	(33ksi)	Beam	Other	None	0.000							
23	23	SAU	6X4X0.375	A7	(33ksi)	Beam	Other	None	0.000							
24	24	SAU	6X4X0.375	A7	(33ksi)	Beam	Other	None	0.000							
25	25	SAU	3.5X3.5X0.25	A7	(33ksi)	Beam	Other	None	0.000							
26	26	SAU	3.5X3.5X0.25	A7	(33ksi)	Beam	Other	None	0.000							
27	27	SAU	3.5X3.5X0.25	A7	(33ksi)	Beam	Other	None	0.000							
28	28	SAU	3.5X3.5X0.25	A7	(33ksi)	Beam	Other	None	0.000							
29	29	SAE	2X2X0.25	A7	(33ksi)	T-Only	Other	None	0.000							
30	30	SAU	3X2X0.3125	A7	(33ksi)	T-Only	Other	None	0.000							
31	31	SAE	2X2X0.3125	A7	(33ksi)	T-Only	Other	None	0.000							
32	32	SAE	2.5X2.5X0.1875	A7	(33ksi)	Truss	Other	None	0.000							
33	33	SAE	3X3X0.1875	A7	(33ksi)	Truss	Other	None	0.000							
34	34	SAU	3X2.5X0.25	A7	(33ksi)	Truss	Other	None	0.000							
35	35	SAU	3X2.5X0.25	A7	(33ksi)	Truss	Other	None	0.000							
36	36	SAU	5/16X3.5	A7	(33ksi)	T-Only	Other	None	0.000							
37	37	SAU	5/16X3.5	A7	(33ksi)	T-Only	Other	None	0.000							
38	38	SAU	2X2X0.3125	A7	(33ksi)	T-Only	Other	None	0.000							
39	39	SAU	3X2X0.3125	A7	(33ksi)	T-Only	Other	None	0.000							
40	40	SAU	3.5X3X0.25	A7	(33ksi)	Beam	Other	None	0.000							

Aggregate Angle Information:

Angle Type	Angle Size	Material	Total Length (ft)	Total Surface Area (ft ²)	Total Weight (lbs)
SAE	8X8X0.75	A7 (33ksi)	162.60	433.59	6324.96
SAE	8X8X0.5	A7 (33ksi)	140.38	374.34	3706.00
SAE	6X6X0.375	A7 (33ksi)	76.00	152.00	1132.40
SAE	3.5X3.5X0.25	A7 (33ksi)	185.94	216.93	1078.44
SAU	3.5X3X0.25	A7 (33ksi)	88.00	95.33	475.20
DAS	2.5X2X0.1875	A7 (33ksi)	65.60	49.20	360.80
SAU	6X4X0.3125	A7 (33ksi)	147.22	245.36	1516.32
BAR	5/16X3.5	A7 (33ksi)	16.00	10.17	59.52
SAU	6X4X0.375	A7 (33ksi)	83.27	138.79	1024.24
BAR	5/16X3.75	A7 (33ksi)	16.00	10.83	63.84
SAU	3X2.5X0.3125	A7 (33ksi)	16.00	14.67	89.60
SAU	6X3.5X0.3125	A7 (33ksi)	83.27	131.85	816.06
SAU	3X2X0.25	A7 (33ksi)	313.71	261.42	1286.20
SAE	3X3X0.25	A7 (33ksi)	413.46	413.46	2025.94
SAU	4X3.5X0.3125	A7 (33ksi)	198.72	248.39	1530.11
SAE	2X2X0.1875	A7 (33ksi)	160.00	106.67	390.40
SAE	3X3X0.1875	A7 (33ksi)	45.25	45.25	167.90
SAU	3X2.5X0.25	A7 (33ksi)	22.63	20.74	101.82
SAE	2.5X2.5X0.1875	A7 (33ksi)	22.63	18.86	69.47
SAE	2X2X0.3125	A7 (33ksi)	60.66	40.44	237.79
SAU	3X2X0.3125	A7 (33ksi)	62.41	58.25	349.52
SAE	2X2X0.25	A7 (33ksi)	62.41	41.61	199.09
SAU	3.5X2.5X0.25	A7 (33ksi)	71.43	71.43	349.99

Sections:
The adjustment factors below only apply to dead load and wind areas that are calculated for members in the model.
They do not apply to equipment or to manually input dead load and drag areas.

Section Joint Defining Dead Load Transverse Longitudinal Transverse Longitudinal Adj. Factor Adj. Factor Adj. Factor
Label Section Bottom Adj. Factor Drag Area Drag Area Drag Area Drag Area For Face For Face For Face For Face
(Also SAPS)

1	100P	1.100	0.000	0.000	1.100	1.100	0.000	0.000	0.000
2	101P	1.100	0.000	0.000	1.100	1.100	0.000	0.000	0.000
3	103P	1.100	0.000	0.000	1.100	1.100	0.000	0.000	0.000
4	105P	1.100	0.000	0.000	1.100	1.100	0.000	0.000	0.000
5	108P	1.100	0.000	0.000	1.100	1.100	0.000	0.000	0.000
6	111P	1.100	0.000	0.000	1.100	1.100	0.000	0.000	0.000

Angle Member Connectivity:

Member Group	Section	Symmetry	Origin	End	Rest.	Ratio	Ratio	Ratio	Bolt	# Bolt	# Shear	Connect	Short	Long	End	Bolt Rest.
Label	Label	Code	Joint	Joint	Code	RLX	RLY	RLZ	Type	Bolts	Holes	Leg	Edge	Dist.	Spacing	Coef.
Legs1P	7	XY-Symmetry	100P	101P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs2P	5	XY-Symmetry	101P	102P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs3P	4	XY-Symmetry	102P	103P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs4P	3	XY-Symmetry	103P	104P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs5P	2	XY-Symmetry	104P	105P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs6P	1	XY-Symmetry	105P	106P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs7P	1	XY-Symmetry	106P	107P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs8P	2	XY-Symmetry	107P	108P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs9P	2	XY-Symmetry	108P	109P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs10P	2	XY-Symmetry	109P	110P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs11P	1	XY-Symmetry	110P	111P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs12P	1	XY-Symmetry	111P	112P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs13P	1	XY-Symmetry	112P	113P	1	4	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs14P	1	XY-Symmetry	113P	114P	3	6	2	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs15P	1	XY-Symmetry	114P	115P	3	6	2	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs16P	1	XY-Symmetry	115P	116P	3	6	1	2	1	1	1	1 5/8 A-394	0	4	0	0
Legs17P	1	XY-Symmetry	116P	117P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs18P	1	XY-Symmetry	117P	118P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs19P	1	XY-Symmetry	118P	119P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs20P	1	XY-Symmetry	119P	120P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs21P	1	XY-Symmetry	120P	121P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs22P	1	XY-Symmetry	121P	122P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs23P	1	XY-Symmetry	122P	123P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs24P	1	XY-Symmetry	123P	124P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs25P	1	XY-Symmetry	124P	125P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs26P	1	XY-Symmetry	125P	126P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs27P	1	XY-Symmetry	126P	127P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs28P	1	XY-Symmetry	127P	128P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs29P	1	XY-Symmetry	128P	129P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs30P	1	XY-Symmetry	129P	130P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs31P	1	XY-Symmetry	130P	131P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs32P	1	XY-Symmetry	131P	132P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs33P	1	XY-Symmetry	132P	133P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs34P	1	XY-Symmetry	133P	134P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs35P	1	XY-Symmetry	134P	135P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs36P	1	XY-Symmetry	135P	136P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs37P	1	XY-Symmetry	136P	137P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs38P	1	XY-Symmetry	137P	138P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs39P	1	XY-Symmetry	138P	139P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs40P	1	XY-Symmetry	139P	140P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs41P	1	XY-Symmetry	140P	141P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs42P	1	XY-Symmetry	141P	142P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs43P	1	XY-Symmetry	142P	143P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs44P	1	XY-Symmetry	143P	144P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs45P	1	XY-Symmetry	144P	145P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs46P	1	XY-Symmetry	145P	146P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs47P	1	XY-Symmetry	146P	147P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs48P	1	XY-Symmetry	147P	148P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs49P	1	XY-Symmetry	148P	149P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs50P	1	XY-Symmetry	149P	150P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs51P	1	XY-Symmetry	150P	151P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs52P	1	XY-Symmetry	151P	152P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs53P	1	XY-Symmetry	152P	153P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs54P	1	XY-Symmetry	153P	154P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs55P	1	XY-Symmetry	154P	155P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs56P	1	XY-Symmetry	155P	156P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs57P	1	XY-Symmetry	156P	157P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs58P	1	XY-Symmetry	157P	158P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs59P	1	XY-Symmetry	158P	159P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs60P	1	XY-Symmetry	159P	160P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs61P	1	XY-Symmetry	160P	161P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs62P	1	XY-Symmetry	161P	162P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs63P	1	XY-Symmetry	162P	163P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs64P	1	XY-Symmetry	163P	164P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs65P	1	XY-Symmetry	164P	165P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs66P	1	XY-Symmetry	165P	166P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs67P	1	XY-Symmetry	166P	167P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs68P	1	XY-Symmetry	167P	168P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs69P	1	XY-Symmetry	168P	169P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs70P	1	XY-Symmetry	169P	170P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs71P	1	XY-Symmetry	170P	171P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs72P	1	XY-Symmetry	171P	172P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs73P	1	XY-Symmetry	172P	173P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs74P	1	XY-Symmetry	173P	174P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs75P	1	XY-Symmetry	174P	175P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs76P	1	XY-Symmetry	175P	176P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs77P	1	XY-Symmetry	176P	177P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs78P	1	XY-Symmetry	177P	178P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs79P	1	XY-Symmetry	178P	179P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs80P	1	XY-Symmetry	179P	180P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs81P	1	XY-Symmetry	180P	181P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs82P	1	XY-Symmetry	181P	182P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs83P	1	XY-Symmetry	182P	183P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs84P	1	XY-Symmetry	183P	184P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs85P	1	XY-Symmetry	184P	185P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs86P	1	XY-Symmetry	185P	186P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs87P	1	XY-Symmetry	186P	187P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs88P	1	XY-Symmetry	187P	188P	3	6	1	1	1	1	1	1 5/8 A-394	0	4	0	0
Legs89P	1	XY-Symmetry	188P	189P	3	6										

Page 8/29

Joint Label	Dead Load (kips)	X-Drags (ft ²)	Y-Drags (ft ²)	Area (ft ²)	26.611	12.29	26.611	0.000	0.000	0.000	37.663	0.000	0.000	0.000	0.000	0.000	20.800	L/R	20.800	Net Sect
Di5bY	14	26.611																L/R	20.800	Net Sect
Di6aX	13	49.948																L/R	44.900	Net Sect
Di6aY	13	49.948																L/R	44.900	Net Sect
Di6bX	13	49.948																L/R	44.900	Net Sect
Di6bY	13	49.948																L/R	44.900	Net Sect
Di6cX	13	49.948																L/R	44.900	Net Sect
Di6cY	13	49.948																L/R	44.900	Net Sect
Di7aX	13	49.948																L/R	44.900	Net Sect
Di7aY	13	49.948																L/R	44.900	Net Sect
Di7bX	13	49.948																L/R	44.900	Net Sect
Di7bY	13	49.948																L/R	44.900	Net Sect
Di8aX	12	26.283																L/R	23.700	Net Sect
Di8aY	12	26.283																L/R	23.700	Net Sect
Di8bX	12	26.283																L/R	23.700	Net Sect
Di8bY	12	26.283																L/R	23.700	Net Sect
Di8cX	12	26.283																L/R	23.700	Net Sect
Di8cY	12	26.283																L/R	23.700	Net Sect
Di9aX	12	26.283																L/R	23.700	Net Sect
Di9aY	12	26.283																L/R	23.700	Net Sect
Di9bX	12	26.283																L/R	23.700	Net Sect
Di9bY	12	26.283																L/R	23.700	Net Sect
Di10aX	11	9.713																L/R	9.200	Net Sect
Di10aY	11	9.713																L/R	9.200	Net Sect
Di10bX	11	9.713																L/R	9.200	Net Sect
Di10bY	11	9.713																L/R	9.200	Net Sect
Di11aX	11	9.713																L/R	9.200	Net Sect
Di11aY	11	9.713																L/R	9.200	Net Sect
Di11bX	11	9.713																L/R	9.200	Net Sect
Di11bY	11	9.713																L/R	9.200	Net Sect
Di12aX	33	22.157																L/R	20.000	Net Sect
Di12aY	33	22.157																L/R	20.000	Net Sect
Di12bX	33	22.157																L/R	20.000	Net Sect
Di12bY	33	22.157																L/R	20.000	Net Sect
Di13aX	32	14.571																L/R	12.200	Net Sect
Di13aY	32	14.571																L/R	12.200	Net Sect
Di13bX	32	14.571																L/R	12.200	Net Sect
Di13bY	32	14.571																L/R	12.200	Net Sect
Di14aX	27	61.558																L/R	58.800	Net Sect
Di14aY	27	61.558																L/R	58.800	Net Sect
Di14bX	27	61.558																L/R	58.800	Net Sect
Di14bY	27	61.558																L/R	58.800	Net Sect
Di15aX	26	72.552																L/R	63.400	Net Sect
Di15aY	26	72.552																L/R	63.400	Net Sect
Di15bX	26	72.552																L/R	63.400	Net Sect
Di15bY	26	72.552																L/R	63.400	Net Sect
Di16aX	47	2.721																L/R	2.721	Net Sect
Di16aY	47	2.721																L/R	2.721	Net Sect
Di16bX	47	2.721																L/R	2.721	Net Sect
Di16bY	47	2.721																L/R	2.721	Net Sect
Di17aX	30	2.546																L/R	2.546	Net Sect
Di17aY	30	2.546																L/R	2.546	Net Sect
Di17bX	25	61.558																L/R	58.800	Net Sect
Di17bY	25	61.558																L/R	58.800	Net Sect
Di18aX	46	1.801																L/R	1.801	Net Sect
Di18aY	46	1.801																L/R	1.801	Net Sect
Di19aX	29	13.291																L/R	12.546	Net Sect
Di19aY	29	13.291																L/R	12.546	Net Sect
Di20aX	28	12.546																L/R	12.546	Net Sect
Di20aY	28	12.546																L/R	12.546	Net Sect
Di21aX	24	49.451																L/R	46.000	Net Sect
Di21aY	24	49.451																L/R	46.000	Net Sect
Di22aX	24	47.410																L/R	46.000	Net Sect
Di22aY	24	47.410																L/R	46.000	Net Sect

Sum of Dead Load and Drag Areas From Equipment, Input and Calculated:

Joint Label	Dead Load (kips)	X-Drags (ft ²)	Y-Drags (ft ²)	Area (ft ²)
100P	0.45	0.000	0.000	0.000
101P	0.763	0.000	0.000	0.000
101aP	0.242	0.000	0.000	0.000
101bP	0.242	0.000	0.000	0.000
102P	0.647	0.000	0.000	0.000
102aP	0.116	0.000	0.000	0.000
102bP	0.116	0.000	0.000	0.000
103P	0.521	0.000	0.000	0.000
104P	0.378	0.000	0.000	0.000
105P	0.505	0.000	0.000	0.000
106P	0.186	0.000	0.000	0.000
107P	0.417	0.000	0.000	0.000
108P	0.543	0.000	0.000	0.000
109P	0.143	0.000	0.000	0.000
110P	0.251	0.000	0.000	0.000
111P	0.319	0.000	0.000	0.000
112P	0.131	0.000	0.000	0.000

113P	0.197	0.000	0.000
Arm1aP	0.224	5.024	0.000
Arm1bP	0.234	5.267	3.338
Arm2aP	0.318	6.695	0.000
Arm2bP	0.33	6.968	3.908
Arm3aP	0.213	5.067	0.000
Arm3bP	0.223	5.307	3.522
Arm4aP	0.272	7.141	0.000
Arm4bP	0.283	7.426	4.514
100X	0.45	0.000	8.872
100XY	0.45	8.872	8.872
100Y	0.45	8.872	0.000
101X	0.763	0.000	13.536
101XY	0.763	13.536	13.536
101Y	0.763	13.536	0.000
101aX	0.242	0.000	14.199
101bY	0.242	14.199	0.000
102X	0.647	0.000	12.080
102XY	0.647	12.080	12.080
102Y	0.647	12.080	0.000
102aX	0.116	0.000	5.199
102bY	0.116	5.199	0.000
103X	0.521	0.000	11.644
103XY	0.521	11.644	11.644
103Y	0.521	11.644	0.000
104X	0.378	0.000	9.482
104XY	0.378	9.482	9.482
104Y	0.378	9.482	0.000
105X	0.509	0.000	9.704
105XY	0.509	14.701	9.704
105Y	0.505	14.516	0.000
106X	0.188	0.000	4.053
106XY	0.188	6.137	4.053
106Y	0.186	6.079	0.000
107X	0.417	0.000	9.779
107XY	0.417	9.779	9.779
107Y	0.417	9.779	0.000
108X	0.547	0.000	9.874
108XY	0.547	15.559	9.874
108Y	0.543	15.374	0.000
109X	0.144	0.000	3.466
109XY	0.144	6.371	3.466
109Y	0.143	6.282	0.000
110X	0.251	0.000	7.334
110XY	0.251	7.334	7.334
110Y	0.251	7.334	0.000
111X	0.343	0.000	7.299
111XY	0.343	12.296	7.299
111Y	0.339	12.112	0.000
112X	0.193	0.000	5.576
112XY	0.193	9.111	5.576
112Y	0.191	8.954	0.000
113X	0.201	0.000	2.979
113XY	0.201	8.663	2.979
113Y	0.197	8.479	0.000
Total	25.7	358.381	266.038

Unadjusted Dead Load and Drag Areas by Section:

Section Label	Unfactored Dead Load (kips)	X-Drag Area (ft^2)	Y-Drag Area (ft^2)	X-Drag Area Face (ft^2)	Y-Drag Area Face (ft^2)
1	3.269	80.778	80.778	32.261	32.261
2	5.662	137.855	137.855	56.380	56.380
3	3.067	92.742	92.742	38.447	38.447
4	4.676	163.208	125.259	66.937	47.963
5	3.646	148.521	100.049	62.761	38.525
6	3.035	153.363	71.887	69.015	28.277
Total	23.356	776.468	608.571	325.801	241.852

Angle Member Weights and Surface Areas by Section:

Section Label	Unfactored Weight (kips)	Factored Weight (kips)	Unfactored Surface Area (ft^2)	Factored Surface Area (ft^2)
1	3.269	3.596	318.529	350.382
2	5.662	6.229	618.476	680.323
3	3.067	3.374	449.884	494.873
4	4.676	5.144	646.976	711.674

5	3.646	4.010	579.330	637.263
6	3.035	3.339	586.374	645.012
Total	23.356	25.691	3199.569	3519.526

*** Insulator Data

Clamp Properties:

Label	Stock Number	Holding Capacity (lbs)
Misc		1e+005

Clamp Insulator Connectivity:

Clamp Label	Structure And Tip Attach	Property Set
Arm4aP	Arm4aP	Misc
Arm4bP	Arm4bP	Misc
Arm3aP	Arm3aP	Misc
Arm3bP	Arm3bP	Misc
Arm2aP	Arm2aP	Misc
Arm2bP	Arm2bP	Misc
Arm1aP	Arm1aP	Misc
Arm1bP	Arm1bP	Misc
100P	100P	Misc
101P	101P	Misc
102P	102P	Misc
103P	103P	Misc
104P	104P	Misc
105P	105P	Misc
105X	105X	Misc
105XY	105XY	Misc
105Y	105Y	Misc
106P	106P	Misc
107P	107P	Misc
108P	108P	Misc
109P	109P	Misc
110P	110P	Misc
111P	111P	Misc
112P	112P	Misc
112X	112X	Misc
112XY	112XY	Misc
112Y	112Y	Misc

*** Loads Data

Loads from file: g:\2356-t-mobile\2356-09215a\2356-09215a-analysis\twrloads\proposed\lca

Vector Load Cases:

Load Case Description	Dead Load Factor	SF for Steel Arms, Poles and Towers	SF for Wood Poles	SF for Conc. Ult.	SF for Conc. First Crack	SF for Conc. Zero Tens.	SF for Guys	SF for Non-Steel Arms	SF for Braces	SF for Insuls.	SF for Found.	Trans. Wind Pressure (psf)	Longit. Wind Pressure (psf)	Point Loads	Wind Model	EDF Load Type	Joint Displ.
ConEdStd(int)	1.2700	1.00000	0.0000	0.0000	0.0000	0.0000	1.0000	0.0000	0.0000	1.0000	1.0000	49.53	0	22 loads	Standard	Hyp 1	
ConEdEmerIce(int)	1.0000	1.00000	0.0000	0.0000	0.0000	0.0000	1.0000	0.0000	0.0000	1.0000	1.0000	30	0	22 loads	Standard	Hyp 1	
ConEdHurr(int)	1.0000	1.00000	0.0000	0.0000	0.0000	0.0000	1.0000	0.0000	0.0000	1.0000	1.0000	75	0	22 loads	Standard	Hyp 1	

Point Loads for Load Case "ConEdStd(int)":

Joint Label	Vertical Load (lbs)	Transverse Load (lbs)	Longitudinal Load (lbs)
Arm1aP	12615	9538	0
Arm1bP	12615	9538	0
Arm2aP	12615	9538	0
Arm2bP	12615	9538	0
Arm3aP	12615	9538	0
Arm3bP	12615	9538	0
Arm4aP	1894	2517	0
Arm4bP	1894	2517	0
105P	535	720	0
105X	535	720	0

105Y	535	720	0
105XY	535	720	0
112P	32	151	0
112X	32	151	0
112XY	32	151	0
112Y	32	151	0
100P	100	215	0
101P	246	528	0
103P	244	523	0
105P	216	464	0
108P	237	510	0
111P	156	335	0

Point Loads for Load Case "ConEdEmerIce(int)":

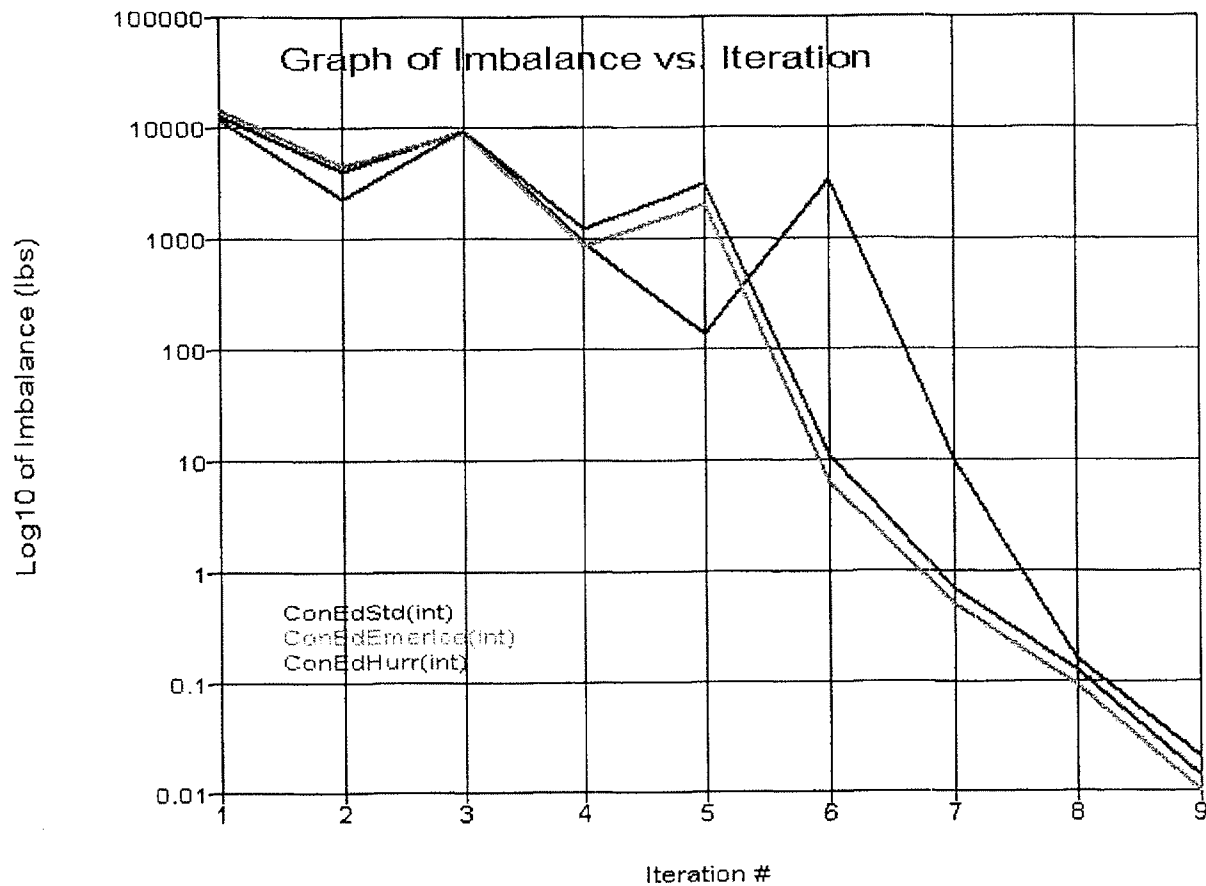
Joint Label	Vertical Load (lbs)	Transverse Load (lbs)	Longitudinal Load (lbs)
Arm1aP	14511	9088	0
Arm1bP	14511	9088	0
Arm2aP	14511	9088	0
Arm2bP	14511	9088	0
Arm3aP	14511	9088	0
Arm3bP	14511	9088	0
Arm4aP	2827	2620	0
Arm4bP	2827	2620	0
105P	845	726	0
105X	845	726	0
105Y	845	726	0
105XY	845	726	0
112P	26	93	0
112X	26	93	0
112XY	26	93	0
112Y	26	93	0
100P	79	132	0
101P	194	325	0
103P	192	322	0
105P	170	285	0
108P	187	314	0
111P	123	206	0

Point Loads for Load Case "ConEdHurr(int)":

Joint Label	Vertical Load (lbs)	Transverse Load (lbs)	Longitudinal Load (lbs)
Arm1aP	6408	11434	0
Arm1bP	6408	11434	0
Arm2aP	6408	11434	0
Arm2bP	6408	11434	0
Arm3aP	6408	11434	0
Arm3bP	6408	11434	0
Arm4aP	606	1808	0
Arm4bP	606	1808	0
105P	111	720	0
105X	111	720	0
105Y	111	720	0
105XY	111	720	0
112P	26	233	0
112X	26	233	0
112XY	26	233	0
112Y	26	233	0
100P	79	330	0
101P	194	813	0
103P	192	804	0
105P	170	714	0
108P	187	784	0
111P	123	516	0

*** Analysis Results:

Maximum element utilization is 80.91% for Angle "Arm2dY" in load case "ConEdEmerIce(int)"
Maximum insulator utilization is 17.46% for Clamp "Arm2bP" in load case "ConEdEmerIce(int)"



Angle Forces For All Load Cases:
Positive for tension - negative for compression

Group Label	Angle Label	Max. Tens. For All LC (kips)	Max. Comp. For All LC (kips)	LC 1 (kips)	LC 2 (kips)	LC 3 (kips)
7	Legs1P	0.000	-148.026	-138.817	-130.155	-148.026
5	Legs2aP	0.000	-147.017	-137.544	-129.155	-147.017
5	Legs2bP	0.000	-137.882	-130.729	-124.345	-137.882
4	Legs3aP	0.000	-136.832	-130.354	-124.964	-136.832
4	Legs3bP	0.000	-117.225	-112.909	-109.085	-117.225
3	Legs4aP	0.000	-97.017	-94.189	-91.343	-97.017
3	Legs4bP	0.000	-93.601	-87.587	-83.857	-93.601
3	Legs4cP	0.000	-68.503	-68.503	-66.693	-68.368
2	Legs5aP	0.000	-40.209	-40.209	-40.003	-37.272
2	Legs5bP	0.000	-33.825	-33.562	-32.478	-33.825
2	Legs5cP	0.000	-20.009	-19.712	-20.009	-16.568
1	Legs6aP	0.000	-10.693	-9.705	-10.693	-5.898
1	Legs6bP	0.107	-0.369	-0.081	0.107	-0.369
Hor1	Hor1aP	0.000	-0.083	-0.083	-0.036	-0.026
Hor1	Hor1bP	0.595	0.000	0.271	0.131	0.595
18	Hor2aP	0.000	-0.959	-0.933	-0.796	-0.959
18	Hor2bP	0.000	-5.822	-4.181	-3.001	-5.822

16	Hor3AP	0.000	-0.473	-0.473	-0.473	-0.303
16	Hor3BP	0.000	-3.560	-3.509	-3.116	-3.350
16	Hor3CP	0.000	-20.923	-18.146	-20.923	-9.354
27	Hor6P	19.717	0.000	17.439	19.717	8.766
45	Hor6P	0.000	-27.312	-23.869	-27.312	-12.010
26	Hor9P	24.203	0.000	21.195	24.203	10.802
44	Hor9P	0.000	-22.170	-19.519	-22.170	-9.982
25	Hor11P	11.941	0.000	10.368	11.941	5.854
43	Hor12P	0.759	-0.687	-0.219	0.759	-0.687
24	Hor13P	0.759	0.000	0.000	0.000	0.000
21	Dia1AP	0.015	-0.131	0.015	0.000	0.000
21	Dia1BP	0.015	0.000	0.000	0.000	0.000
19	Dia2AP	9.227	0.000	6.394	4.463	9.227
19	Dia2BP	0.048	-0.041	0.048	-0.041	-0.022
17	Dia3AP	3.674	0.000	2.605	1.933	3.674
17	Dia3BP	0.395	0.000	0.395	0.243	0.316
15	Dia4AP	6.928	-2.546	-1.333	-0.739	-2.546
15	Dia4BP	0.000	0.000	5.963	5.686	6.928
14	Dia5AP	0.000	-5.309	-5.108	-5.193	-5.193
14	Dia5BP	0.000	-14.721	-12.204	-10.885	-14.721
13	Dia6AP	0.000	-22.822	-18.761	-17.097	-22.822
13	Dia6BP	0.000	-9.239	-9.036	-8.119	-8.119
13	Dia7AP	0.000	-22.669	-19.039	-17.594	-22.669
13	Dia7BP	3.166	0.000	3.366	3.033	3.339
12	Dia8AP	0.000	-12.642	-10.790	-9.879	-12.642
12	Dia8BP	0.000	-4.310	-3.956	-4.310	-2.569
12	Dia9AP	0.000	-12.067	-10.364	-9.566	-12.067
12	Dia9BP	0.000	-1.428	-1.226	-1.428	-0.862
11	Dia10AP	0.000	-2.688	-2.666	-2.688	-2.239
11	Dia10BP	0.000	-1.196	-1.196	-1.128	-0.976
11	Dia11AP	0.217	-0.563	-0.052	0.217	-0.563
11	Dia11BP	0.000	-3.982	-3.538	-3.982	-1.830
33	IntDiAP	5.089	-2.116	-2.116	-2.070	-1.444
33	IntDiBP	8.963	0.000	4.597	5.089	2.399
33	IntDiAl1P	2.688	0.000	8.004	8.963	4.194
33	IntDiAl1BP	0.000	-16.632	-13.690	-16.632	-3.564
27	Arm1AP	0.000	-27.380	-24.880	-27.380	-16.316
48	Arm1CP	22.570	0.000	19.797	22.570	10.040
31	Arm2AP	21.717	0.000	20.825	21.717	10.737
26	Arm2BP	0.000	-20.513	-17.191	-20.513	-5.488
47	Arm2CP	26.255	-31.100	-28.154	-31.100	-17.980
30	Arm3AP	2.347	0.000	23.005	26.255	11.838
25	Arm3BP	0.000	-12.310	-9.949	-12.310	-1.725
25	Arm3CP	0.000	-22.625	-20.452	-22.625	-14.028
46	Arm3BP	18.562	0.000	16.242	18.562	8.348
28	Arm4AP	18.254	0.000	16.242	18.254	8.565
28	Arm4BP	0.000	-4.516	-3.921	-4.516	-1.320
24	Arm4CP	5.938	0.000	4.384	5.938	2.130
24	Arm4BP	2.953	0.000	1.776	2.953	0.136
7	Leg81X	98.044	0.000	70.186	60.117	99.557
7	Leg81Y	0.000	68.459	62.345	98.044	0.000
5	Leg82AX	100.468	-143.261	-134.345	-124.642	-143.261
5	Leg82AX	98.961	0.000	71.285	60.978	100.468
5	Leg82AY	0.000	-142.362	-133.292	-123.806	-142.362
5	Leg82BX	107.478	0.000	76.317	65.101	107.478
5	Leg82BY	106.484	0.000	75.366	64.749	106.484
4	Leg93AX	107.457	0.000	76.780	65.854	107.457
4	Leg93AX	108.773	0.000	78.010	66.657	108.773
4	Leg93BX	0.000	-137.418	-130.821	-125.096	-137.418
4	Leg93BY	90.439	0.000	63.508	54.301	90.439
4	Leg93BX	93.862	0.000	66.937	56.244	93.862
4	Leg93BY	0.000	-119.907	-115.521	-110.284	-119.907
3	Leg94AX	74.665	0.000	52.879	45.587	74.665
3	Leg94AX	78.287	0.000	56.551	47.611	78.287
3	Leg94BY	0.000	-100.082	-97.260	-92.833	-100.082
3	Leg94BX	81.733	0.000	63.204	55.153	81.733
3	Leg94BY	0.000	-96.614	-90.610	-85.320	-96.614
3	Leg94CX	49.901	0.000	34.314	28.757	49.901
3	Leg94CY	51.516	0.000	35.987	29.699	51.516
2	Leg95AX	23.748	0.000	-69.613	-67.161	-69.502
2	Leg95AX	24.233	0.000	14.472	10.980	23.748
2	Leg95AY	0.000	-40.474	-40.474	-40.104	-37.539
2	Leg95BX	27.209	0.000	21.157	18.549	27.209
2	Leg95BY	27.687	0.000	21.657	18.858	27.687
2	Leg95BY	0.000	-34.091	-33.826	-32.578	-34.091

2	Legs5cX	7.960	0.000	3.191	1.273	7.960
2	Legs5cY	8.137	0.000	3.176	1.394	8.137
2	Legs5cZ	19.969	-19.969	-19.969	-16.587	-16.587
1	Legs6aX	0.000	-5.254	-3.997	-5.254	-1.017
1	Legs6aY	0.000	-5.233	-3.972	-5.233	-0.988
1	Legs6aZ	0.000	-10.700	-9.711	-10.700	-5.901
1	Legs6bX	1.720	0.000	1.578	1.720	1.049
1	Legs6bY	1.726	0.000	1.585	1.726	1.057
1	Legs6bZ	0.102	-0.376	-0.087	0.102	-0.376
1	Legs6bY	0.000	-0.275	-0.275	-0.235	-0.233
Hor1	Hor1aX	0.000	-0.267	-0.267	-0.236	-0.226
Hor1	Hor1aY	0.000	-0.204	-0.204	-0.169	-0.142
Hor1	Hor1bX	0.000	-1.570	-1.146	-0.755	-1.570
Hor1	Hor1bY	0.000	-1.621	-1.176	-0.886	-1.621
Hor1	Hor1bZ	0.000	-0.206	-0.206	-0.100	-0.171
18	Hor2aX	0.000	-4.113	-2.695	-2.138	-4.113
18	Hor2aY	0.000	-0.638	-0.536	-0.570	-0.638
18	Hor2bX	0.000	-1.136	-0.914	-0.582	-1.136
18	Hor2bY	1.393	-1.251	-1.059	1.393	-1.251
18	Hor2bZ	0.000	-3.928	-2.689	-0.173	-3.928
16	Hor3aY	0.000	-0.663	-0.663	-0.525	-0.615
16	Hor3bX	2.493	0.000	1.996	1.819	2.493
27	Hor5Y	0.000	-20.970	-18.486	-20.970	-9.618
45	Hor6Y	19.714	0.000	17.229	19.714	8.751
26	Hor8Y	0.000	-27.433	-24.086	-27.433	-12.349
44	Hor9Y	24.203	0.000	21.190	24.203	10.792
25	Hor11Y	0.000	-22.243	-19.652	-22.243	-10.154
43	Hor12Y	11.945	0.000	10.972	11.945	5.859
24	Hor13Y	0.764	-0.683	-0.215	0.764	-0.683
21	Dia1aX	10.107	0.000	6.859	4.677	10.107
21	Dia1aY	6.595	-0.681	4.216	-0.681	6.595
21	Dia1aZ	0.000	-3.985	0.000	-3.985	0.000
21	Dia1bX	9.065	0.000	6.247	4.962	9.065
21	Dia1bY	8.796	0.000	5.853	4.779	8.796
21	Dia1bZ	0.000	0.000	0.000	0.000	0.000
19	Dia2aX	0.000	-0.135	-0.209	-0.214	-0.335
19	Dia2aY	0.043	-1.347	0.843	-3.347	-0.111
19	Dia2aZ	6.025	-0.384	3.885	-0.384	6.025
19	Dia2bX	7.467	0.000	4.391	4.120	7.467
19	Dia2bY	7.832	0.000	5.350	4.381	7.832
19	Dia2bZ	0.271	-3.522	-1.782	-1.782	-1.522
17	Dia3aX	0.000	-1.579	-1.156	-1.126	-1.978
17	Dia3aY	2.131	0.000	1.348	1.278	2.131
17	Dia3bX	0.000	-0.159	-0.159	-0.044	-0.097
17	Dia3bY	0.348	0.000	0.348	0.196	0.243
17	Dia3bZ	0.000	-0.215	-0.215	-0.104	-0.178
15	Dia4aX	2.925	0.000	2.666	2.439	2.925
15	Dia4aY	1.845	0.000	1.599	1.845	1.743
15	Dia4aZ	0.000	-1.435	-0.329	-0.208	-1.435
15	Dia4bX	0.000	-6.303	-5.245	-4.935	-6.303
15	Dia4bY	0.000	-6.655	-5.773	-5.186	-6.655
15	Dia4bZ	7.202	0.000	6.391	5.849	7.202
14	Dia5aX	2.280	-0.588	0.177	-0.588	2.280
14	Dia5aY	0.751	-1.334	-1.192	-1.334	0.751
14	Dia5bX	0.000	-4.378	-3.920	-4.378	-3.645
14	Dia5bY	15.518	0.000	13.994	13.035	15.518
14	Dia5bZ	15.048	0.000	13.278	12.694	15.048
14	Dia6bY	0.000	-14.290	-11.559	-10.611	-14.290
13	Dia6aX	22.258	0.000	18.506	17.060	22.258
13	Dia6aY	22.461	0.000	18.888	17.237	22.461
13	Dia6aZ	0.000	-23.025	-19.134	-17.260	-23.025
13	Dia6bX	4.285	0.000	1.512	0.701	4.285
13	Dia6bY	5.378	0.000	2.440	1.237	5.378
13	Dia6bZ	0.000	-9.921	-9.921	-9.722	-9.164
13	Dia7aX	22.015	0.000	18.459	17.123	22.015
13	Dia7aY	22.111	0.000	18.742	17.239	22.111
13	Dia7aZ	0.000	-22.780	-19.334	-17.723	-22.780
13	Dia7bX	0.000	-1.945	-1.068	-0.531	-1.945
13	Dia7bY	0.037	-0.784	-0.073	0.037	-0.784
13	Dia7bZ	2.532	0.000	2.427	2.532	2.231
12	Dia8aX	12.149	0.000	10.281	9.424	12.149
12	Dia8aY	12.044	0.000	10.250	9.399	12.044
12	Dia8aZ	0.000	-12.543	-10.761	-9.853	-12.543
12	Dia8bX	0.000	-3.116	-2.624	-3.116	-0.858
12	Dia8bY	0.000	-2.965	-2.384	-2.965	-0.554
12	Dia8bZ	0.000	-4.429	-4.168	-4.429	-2.843
12	Dia9aX	11.925	0.000	10.680	10.092	11.925
12	Dia9aY	11.811	0.000	10.642	10.063	11.811
12	Dia9aZ	0.000	-11.951	-10.326	-9.538	-11.951

12	DisBxX	0.373	-0.024	0.050	-0.024	0.372			
12	DisBxY	0.677	0.000	0.289	0.125	0.677			
12	DisBxZ	0.500	-1.545	-1.435	-1.132	0.500			
11	DisA0X	1.287	0.000	0.687	0.250	1.287			
11	DisA0Y	0.000	-2.695	-2.695	-2.695	0.000			
11	DisA0Z	0.000	-0.805	-0.798	-0.805	-0.238			
11	DisA10X	0.000	-0.793	-0.783	-0.793	-0.222			
11	DisA10Y	0.000	-1.207	-1.207	-1.133	-0.991			
11	DisA10Z	0.000	3.476	3.476	3.476	2.128			
11	DisA11X	0.218	0.000	0.000	0.218	-0.559			
11	DisA11Y	0.000	-3.425	-3.052	-3.425	-1.534			
11	DisA11Z	0.000	-3.414	-2.998	-3.414	-1.517			
11	DisA12X	0.000	-3.989	-3.548	-3.989	-1.844			
11	DisA12Y	0.000	-1.751	-1.459	-1.751	-0.922			
11	DisA12Z	0.000	3.924	4.720	4.720	1.633			
33	IntcD1aX	4.720	0.000	7.809	8.848	3.944			
33	IntcD1aY	8.848	0.000	7.809	8.848	3.944			
33	IntcD1aZ	2.680	0.000	2.610	2.680	1.482			
32	IntcD1a1X	0.000	-16.583	-13.805	-16.583	-3.688			
27	Arm1aY	0.000	-27.284	-24.717	-27.284	-16.124			
27	Arm1aZ	0.000	22.653	19.854	22.653	10.202			
31	Arm1cY	23.654	0.000	20.678	23.654	10.564			
26	Arm2aY	0.000	-20.482	-17.162	-20.482	-5.456			
26	Arm2aZ	0.000	-31.068	-28.116	-31.068	-17.894			
47	Arm2cY	26.257	0.000	23.034	26.257	11.843			
30	Arm2cZ	27.347	0.000	23.995	27.347	12.321			
25	Arm3aY	0.000	-12.279	-9.914	-12.279	-1.680			
25	Arm3aZ	0.000	-22.600	-20.630	-22.600	-14.000			
46	Arm3cY	18.559	0.000	16.234	18.559	8.332			
29	Arm3cZ	19.258	0.000	16.836	19.258	8.569			
28	Arm4aY	0.000	-4.508	-3.250	-4.508	-1.262			
28	Arm4aZ	0.000	-4.687	-3.391	-4.687	-1.330			
24	Arm4cY	5.599	0.000	4.360	5.599	2.123			
24	Arm4cZ	3.004	0.000	1.789	3.004	0.152			

*** Analysis Results for Load Case No. 1 "ComStd(int)" - Number of iterations in SAPS 9

Equilibrium Joint Positions and Rotations:

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
100P	0	0	0	-0.0534	-0.1030	0.1133	13.87	13.87	0
101P	-0.01595	0.01102	-0.00766	-0.0637	-0.0409	0.1243	10.98	11.01	15.99
101aP	-0.01591	-0.008142	-0.01541	-0.0565	-0.0305	0.0043	-0.01591	10.99	15.98
101bP	8.266e-005	0.01287	-0.005561	-0.0185	-0.0492	0.0986	11	0.01297	15.98
102P	-0.01172	0.0027	-0.01129	-0.0786	0.0174	0.1279	8.188	8.227	31.59
102aP	-0.01156	0.01658	-0.01457	-0.0733	0.0114	0.0079	-0.01155	8.217	31.59
102bP	0.004889	0.02775	-0.003328	-0.0448	-0.0180	0.0768	8.205	0.02775	31.6
103P	-0.009722	0.03616	-0.01258	-0.0967	-0.0772	0.1345	6.779	6.833	32.19
104P	-0.00359	0.0417	-0.01385	-0.0981	-0.0281	0.1391	5.788	5.848	47.19
105P	-0.009721	0.06169	-0.01899	-0.1839	-0.0836	0.1389	3.739	4.062	54.98
106P	-0.01039	0.07965	-0.01902	-0.2279	-0.1056	0.1210	3.939	4.079	59.73
107P	0.00456	0.09805	-0.02093	-0.2184	0.1163	0.1158	4.003	4.098	64.48
108P	-0.004538	0.1356	-0.0233	-0.2583	0.1824	0.1068	3.995	4.136	71.58
109P	-0.0175	0.1591	-0.02549	-0.2739	0.0592	0.1030	3.983	4.159	78.72
110P	0.003288	0.1827	-0.02685	-0.2739	0.2304	0.0937	4.003	4.183	83.47
111P	-0.002199	0.228	-0.02844	-0.2916	-0.1102	0.0957	3.998	4.228	92.97
112P	0.00452	0.2589	-0.02971	-0.2865	0.0569	0.0957	4.005	4.259	98.97
113P	-0.002755	0.2874	-0.0298	-0.2650	-0.1113	0.0567	3.997	4.287	105
Arm1aP	-0.01147	0.0586	-0.01499	-0.0463	0.0113	0.0286	-0.01147	17.56	54.9
Arm1bP	0.006325	0.06738	-0.01499	0.1533	0.0050	0.0283	0.006325	-18.1	54.99
Arm1cP	-0.01061	0.1307	-0.1589	-0.5661	0.0081	0.0312	-0.01061	20.13	73.84
Arm2aP	0.01095	0.1415	-0.003907	0.1540	0.0087	0.0311	0.01095	-20.53	74
Arm2bP	-0.006692	0.2247	-0.1218	-0.4273	0.0083	0.0313	-0.006692	17.72	92.88
Arm2cP	0.01261	0.2338	0.02713	0.0116	0.0089	0.0313	0.01261	-17.94	93.03
Arm3aP	-0.00627	0.2872	-0.117	0.0000	0.0000	0.0000	-0.00627	20.29	104.9
Arm3bP	0.01574	0.2869	0.09786	0.0000	0.0000	0.0000	0.01574	-20.38	105.1
100X	0	0	0	0.0097	-0.0539	-0.2933	13.87	-13.87	0
100Y	0	0	0	-0.0339	0.0624	0.3450	-13.87	13.87	0
100Z	0	0	0	-0.0435	-0.0548	-0.0553	-13.87	-13.87	0
101X	-0.001288	0.01325	0.0009301	0.0027	-0.0581	-0.2938	11	-10.99	16
101Y	0.0005933	0.009793	0.001597	0.0201	0.0657	0.3469	-11	10.99	16
101Z	-0.001584	0.009363	-0.002385	-0.0501	0.0012	-0.0842	-11.02	11.01	16
101aX	-0.000188	0.1617	-0.02484	0.0070	0.0005	0.0005	-0.000188	-10.84	15.98
101bY	-0.01638	0.009413	-0.00163	-0.0080	-0.0332	-0.0000	-0.009413	16	
101cZ	0.00192	0.00192	0.00192	-0.0198	-0.0191	-0.2114	8.199	-8.172	31.6
102XY	9.852e-005	0.02356	0.002742	-0.0102	0.0272	0.2660	-8.2	-8.176	31.6

102Y	-0.01145	0.02289	-0.007423	-0.0678	0.0534	-0.0729	-8.211	8.221	31.59
102X	-0.0004977	0.05681	-0.0009305	-0.0145	0.0050	0.0092	-0.004927	-8.143	31.6
102Y	-0.01683	0.02336	-0.001062	-0.0318	0.0397	0.0123	-8.217	0.02336	31.6
103X	-0.0002927	0.03063	0.002429	-0.0277	-0.0562	-0.1621	-6.808	-6.766	39.4
103Y	-0.0008459	0.03403	-0.003274	-0.0187	0.0648	0.2172	-6.803	-6.822	39.39
103X	-0.0008459	0.03183	-0.009519	-0.0688	0.0037	-0.0813	-6.802	-5.354	47.2
104X	-0.0048567	0.04597	0.002826	-0.0698	0.0288	-0.1532	-3.397	-5.354	47.2
104Y	-0.003299	0.0418	0.003658	-0.0597	-0.0202	0.2087	-3.412	-5.444	47.12
104X	-0.01243	0.04352	-0.01161	-0.0899	0.0528	-0.0845	-4.006	-3.937	55
105X	-0.005058	0.0634	0.003747	-0.1124	0.0829	-0.1093	-4.006	-3.941	55
105Y	-0.006408	0.05946	0.004669	-0.1089	0.0951	-0.1651	-4.006	-3.941	55
105Y	-0.0001789	0.05775	-0.01561	-0.1805	0.1045	-0.0712	-3.992	-3.925	59.75
106X	-0.007316	0.07512	0.004834	-0.1882	0.1129	-0.1378	-3.992	-4.075	59.76
106Y	0.00715	0.0711	-0.005831	-0.1870	0.1293	-0.1378	-3.998	-4.075	59.73
106Y	0.002402	0.07542	-0.0177	-0.2268	-0.0885	-0.0646	-3.991	-3.905	64.51
107X	-0.007408	0.0947	0.006053	-0.2669	0.0918	-0.0532	-3.991	-3.905	64.51
107Y	-0.008671	0.09051	-0.007127	-0.2651	-0.0736	0.1111	-4.01	-3.863	74.01
107Y	-0.01017	0.09383	-0.01967	-0.2164	-0.1210	-0.0587	-4.008	-3.863	74.01
108X	-0.008193	0.1175	0.007193	-0.2090	0.0517	-0.0036	-4.004	-3.867	74.01
108Y	-0.004041	0.1313	0.008544	-0.2091	0.0691	-0.0553	-4.004	-3.867	74.01
108Y	0.0004373	0.1313	-0.02269	-0.2585	0.1988	-0.0482	-4.004	-3.867	74.01
109X	-0.01111	0.1542	-0.007871	-0.2430	0.1502	-0.0174	-3.983	-3.846	78.76
109Y	0.01668	0.15	0.009031	-0.2430	0.1675	-0.0421	-3.983	-3.85	78.76
109Y	0.01475	0.1549	-0.02429	-0.2926	-0.0425	-0.0438	-3.985	-4.155	78.73
110X	-0.005336	0.1793	0.008595	-0.3329	0.1733	0.0297	-3.985	-3.821	83.51
110Y	0.01232	0.175	0.009777	-0.3323	-0.1557	-0.0304	-4.005	-3.825	83.51
110Y	-0.004686	0.175	-0.02566	-0.2732	-0.2139	-0.0399	-4.014	-3.774	93.01
111X	0.01357	0.2229	0.008682	-0.2428	0.0145	-0.0582	-4.014	-3.774	93.01
111Y	-0.003596	0.2229	-0.008682	-0.2429	0.0031	0.0030	-3.997	-3.743	92.97
112X	-0.003484	0.2237	-0.02725	-0.2916	0.1268	-0.0355	-3.997	-3.743	92.97
112Y	-0.002903	0.2527	0.008108	-0.2784	-0.0710	-0.0583	-3.985	-3.747	99.01
112Y	0.01455	0.2528	-0.009307	-0.2785	0.0881	0.0029	-4.001	-4.255	98.97
113X	-0.01349	0.2876	-0.02852	-0.2867	-0.0398	-0.0346	-4.013	-3.712	105
113Y	0.01349	0.2876	0.008201	-0.2958	0.2701	0.0566	-4.013	-3.712	105
113Y	-5.8E-005	0.2832	0.009406	-0.2959	-0.2527	0.0046	-4.013	-3.712	105
113Y	0.007554	0.2831	-0.02861	-0.2651	0.1485	-0.0355	-3.992	-4.283	105

Joint Support Reactions:

Joint Label	X-Force (kips)	Y-Force (kips)	Z-Force (kips)	Comp. (kips)	Uplift (kips)	X-Moment (ft-k)	Y-Moment (ft-k)	Z-Moment (ft-k)	Max. Usage
100P	-24.254	0.0	-24.272	0.0	135.336	0.0	0.0	0.0	0.0
100X	17.174	0.0	-18.025	0.0	-77.256	0.0	0.0	0.0	0.0
100Y	-16.261	0.0	-15.924	0.0	-73.316	0.0	0.0	0.0	0.0
100Y	23.340	0.0	-23.277	0.0	130.810	0.0	0.0	0.0	0.0

Joint Displacements, Loads and Member Forces on Joints:

Joint Label	X External Load (kips)	Y External Load (kips)	Z External Load (kips)	X Member Force (kips)	Y Member Force (kips)	Z Member Force (kips)	X Disp. (ft)	Y Disp. (ft)	Z Disp. (ft)
100P	0.0000	0.2150	-0.709	24.2536	24.0570	-134.6646	0.0000	0.0000	0.0000
101P	0.0000	0.5280	-1.2148	-0.0000	-0.5280	1.2148	-0.0160	0.0130	-0.0077
101P	0.0000	0.0000	-0.3076	-0.0000	-0.0000	0.3076	-0.0159	0.0139	-0.0154
101P	0.0000	0.0000	-0.3076	-0.0000	-0.0000	0.3076	-0.0001	0.0130	-0.0025
102P	0.0000	0.0000	-0.8212	0.0000	0.0000	0.8212	-0.0117	0.0270	-0.013
102P	0.0000	0.0000	-0.1476	-0.0000	-0.0000	0.1476	-0.0115	0.0166	-0.0145
102P	0.0000	0.5230	-0.1476	-0.0000	-0.5230	0.1476	-0.0049	0.0278	-0.0033
103P	0.0000	0.0000	-0.9054	-0.0000	-0.5230	0.9054	-0.0097	0.0162	-0.0126
103P	0.0000	0.0000	-0.4805	-0.0000	0.0000	0.4805	-0.0016	0.0477	-0.0139
104P	0.0000	1.1840	-1.3929	-0.0000	-1.1840	1.3929	-0.0097	0.0617	-0.0170
105P	0.0000	0.0000	-0.2367	-0.0000	-0.0000	0.2367	-0.0104	0.0794	-0.0190
106P	0.0000	0.0000	-0.5300	-0.0000	0.0000	0.5300	-0.0035	0.0981	-0.0209
107P	0.0000	0.0000	-0.9262	-0.0000	-0.5100	0.9262	-0.0045	0.1355	-0.0239
108P	0.0000	0.0000	-0.1811	-0.0000	-0.0000	0.1811	-0.0175	0.1591	-0.0255
109P	0.0000	0.0000	-0.3183	-0.0000	-0.0000	0.3183	-0.0033	0.1827	-0.0284
110P	0.0000	0.0000	-0.5865	-0.0000	-0.3350	0.5865	-0.0022	0.2280	-0.0284
111P	0.0000	0.0000	-0.2743	-0.0000	-0.1510	0.2743	-0.0045	0.2589	-0.0297
112P	0.0000	0.1510	-0.2743	-0.0000	0.0000	0.2743	-0.0028	0.2874	-0.0298
113P	0.0000	0.0000	-0.2503	-0.0000	-0.0000	0.2503	-0.0115	0.0586	-0.1040
Arm1AP	0.0000	9.5380	-12.8990	-0.0000	-9.5380	12.8990	-0.0063	0.0674	-0.1040
Arm2AP	0.0000	9.5380	-12.9116	-0.0000	-9.5380	12.9116	-0.0063	0.0674	-0.1040
Arm3AP	0.0000	9.5380	-13.0183	-0.0000	-9.5380	13.0183	-0.0106	0.1307	-0.1589
Arm4AP	0.0000	9.5380	-13.0338	-0.0000	-9.5380	13.0338	-0.0110	0.1415	-0.0039
Arm5AP	0.0000	9.5380	-12.8858	-0.0000	-9.5380	12.8858	-0.0067	0.2247	-0.1218
Arm6AP	0.0000	9.7124	-12.8977	-0.0000	-9.7124	12.8977	-0.0126	0.2138	0.0271
Arm7AP	0.0000	2.5170	-2.2339	-0.0000	-2.5170	2.2339	-0.0063	0.2872	-0.1170
Arm8AP	0.0000	2.7406	-2.2530	-0.0000	-2.7406	2.2530	-0.0157	0.2869	0.0979
100X	0.0000	0.4394	-0.5709	-17.1740	17.5859	77.8271	0.0000	0.0000	0.0000

100XX	0.0000	0.4394	-0.5709	16.2606	15.4846	73.8871	0.0000	0.0000	0.0000	0.0000	0.0000
100Y	0.0000	0.0000	-0.5709	-23.3401	23.2766	-130.5387	0.0000	0.0000	0.0000	0.0000	0.0000
101X	0.0000	0.6705	-0.9688	-0.0000	-0.5705	0.5688	0.0000	0.0000	0.0000	0.0000	0.0000
101Y	0.0000	0.0000	-0.9688	-0.0000	0.0000	0.5688	0.0000	0.0000	0.0000	0.0000	0.0000
102X	0.0000	0.7033	-0.3076	-0.0000	-0.7033	0.3076	0.0000	0.0000	0.0000	0.0000	0.0000
102Y	0.0000	0.0000	-0.3076	-0.0000	0.0000	0.3076	0.0000	0.0000	0.0000	0.0000	0.0000
103X	0.0000	0.5983	-0.8212	-0.0000	-0.5983	0.8212	0.0000	0.0000	0.0000	0.0000	0.0000
103Y	0.0000	0.0000	-0.8212	-0.0000	0.0000	0.8212	0.0000	0.0000	0.0000	0.0000	0.0000
104X	0.0000	0.2575	-0.1476	-0.0000	-0.2575	0.1476	0.0000	0.0000	0.0000	0.0000	0.0000
104Y	0.0000	0.0000	-0.1476	-0.0000	0.0000	0.1476	0.0000	0.0000	0.0000	0.0000	0.0000
105X	0.0000	0.5767	-0.5614	-0.0000	-0.5767	0.5614	0.0000	0.0000	0.0000	0.0000	0.0000
105Y	0.0000	0.0000	-0.5614	-0.0000	0.0000	0.5614	0.0000	0.0000	0.0000	0.0000	0.0000
106X	0.0000	0.4696	-0.4805	-0.0000	-0.4696	0.4805	0.0000	0.0000	0.0000	0.0000	0.0000
106Y	0.0000	0.0000	-0.4805	-0.0000	0.0000	0.4805	0.0000	0.0000	0.0000	0.0000	0.0000
107X	0.0000	0.4843	-0.5300	-0.0000	-0.4843	0.5300	0.0000	0.0000	0.0000	0.0000	0.0000
107Y	0.0000	0.0000	-0.5300	-0.0000	0.0000	0.5300	0.0000	0.0000	0.0000	0.0000	0.0000
108X	0.0000	0.4891	-0.6948	-0.0000	-0.4891	0.6948	0.0000	0.0000	0.0000	0.0000	0.0000
108Y	0.0000	0.0000	-0.6948	-0.0000	0.0000	0.6948	0.0000	0.0000	0.0000	0.0000	0.0000
109X	0.0000	0.1717	-0.1833	-0.0000	-0.1717	0.1833	0.0000	0.0000	0.0000	0.0000	0.0000
109Y	0.0000	0.0000	-0.1833	-0.0000	0.0000	0.1833	0.0000	0.0000	0.0000	0.0000	0.0000
110X	0.0000	0.3633	-0.3183	-0.0000	-0.3633	0.3183	0.0000	0.0000	0.0000	0.0000	0.0000
110Y	0.0000	0.0000	-0.3183	-0.0000	0.0000	0.3183	0.0000	0.0000	0.0000	0.0000	0.0000
111X	0.0000	0.3615	-0.4351	-0.0000	-0.3615	0.4351	0.0000	0.0000	0.0000	0.0000	0.0000
111Y	0.0000	0.0000	-0.4351	-0.0000	0.0000	0.4351	0.0000	0.0000	0.0000	0.0000	0.0000
112X	0.0000	0.4272	-0.2777	-0.0000	-0.4272	0.2777	0.0000	0.0000	0.0000	0.0000	0.0000
112Y	0.0000	0.0000	-0.2777	-0.0000	0.0000	0.2777	0.0000	0.0000	0.0000	0.0000	0.0000
113X	0.0000	0.1476	-0.2548	-0.0000	-0.1476	0.2548	0.0000	0.0000	0.0000	0.0000	0.0000
113Y	0.0000	0.0000	-0.2548	-0.0000	0.0000	0.2548	0.0000	0.0000	0.0000	0.0000	0.0000

Moments for Angles Modeled as Beams:

Angle Label	Torsion (ft-lbs)	Origin X Moment (ft-lbs)	Origin Z Moment (ft-lbs)	End X Moment (ft-lbs)	End Z Moment (ft-lbs)
LegsAP	-0.00	-0.00	0.00	1312.90	1334.76
LegsAP	22.85	-1471.68	-1306.29	-636.62	-1092.73
LegsAP	84.34	606.44	1074.26	2868.01	1846.52
LegsAP	207.43	-2911.58	-1875.87	-4192.31	-3210.38
LegsAP	204.48	4213.42	3182.81	5067.57	-1018.37
LegsAP	-141.86	3283.62	3022.89	158.57	-10546.65
LegsAP	-139.58	-158.99	10546.68	530.24	8346.20
LegsAP	-141.90	-529.71	-8346.19	-1982.06	3072.39
LegsAP	-21.77	1014.68	-3485.68	234.53	-8995.55
LegsAP	-20.34	-234.67	8995.57	205.18	5091.22
LegsAP	-20.47	-205.09	-5091.22	-412.85	-1207.97
LegsAP	-0.07	37.46	836.50	49.94	443.54
LegsAP	-0.01	-49.94	-443.47	-0.00	0.03
LegsAP	-2.87	85.85	-67.08	173.78	-166.99
LegsAP	3.29	138.64	81.30	83.22	60.71
LegsAP	-2.69	21.09	97.14	37.41	-159.04
LegsAP	0.44	17.63	-32.77	0.76	-141.89
LegsAP	29.18	32.23	-12.66	-39.29	-320.52
LegsAP	-3.98	-0.48	137.90	-14.09	-86.02
LegsAP	-3.13	352.70	-625.41	-360.37	-1483.19
LegsAP	-407.07	626.66	-249.65	45.78	-698.10
LegsAP	-327.97	281.11	-319.71	-208.39	-460.09
LegsAP	248.18	1350.25	525.67	70.80	321.82
LegsAP	-224.11	1330.28	-374.71	67.09	-104.63
LegsAP	392.10	1545.79	367.90	97.95	209.10
LegsAP	-207.96	1919.41	-44.31	51.11	99.30
LegsAP	227.90	617.30	339.08	67.34	203.63
LegsAP	-90.94	1367.15	338.58	26.54	284.95
LegsAP	0.00	0.00	-0.00	-236.89	-60.42

Label	(kips)	(kips)	1
Legs1XY	0.00	-0.00	-0.00
Legs1Y	0.00	0.00	-360.22
Legs2aX	1062.53	-87.39	-31.86
Legs2aY	-1060.58	49.15	-121.57
Legs2bX	-12.55	800.03	-1274.46
Legs2bY	998.61	-277.71	717.73
Legs2bX	-989.34	-239.79	-729.44
Legs2bY	-69.52	-91.94	-235.97
Legs3aX	503.36	2241.87	-2652.07
Legs3aY	-492.22	2251.52	2657.22
Legs3bX	-197.72	-2877.06	1633.54
Legs3bY	506.71	-3463.19	6277.45
Legs3bX	-495.81	-3501.30	6254.69
Legs3bY	194.51	4271.12	-3078.24
Legs4aX	771.75	841.24	-11498.49
Legs4aY	-756.46	1126.06	11643.22
Legs4bX	161.40	3603.62	-3131.23
Legs4bY	775.34	4585.09	9339.00
Legs4bX	-760.25	4464.54	-9176.88
Legs4bY	158.83	-300.71	-10710.07
Legs4cX	775.59	1076.32	5330.12
Legs4cY	-160.51	1166.26	-5352.56
Legs4cX	161.10	-445.65	8382.35
Legs4cY	-119.22	-1192.23	-9821.18
Legs5aX	-214.21	-1198.68	9813.98
Legs5aY	27.48	1020.32	3492.79
Legs5bX	132.05	1977.87	7574.72
Legs5bY	-115.98	1969.07	-7567.81
Legs5bX	26.11	-247.92	-8997.30
Legs5cX	122.00	61.15	-195.61
Legs5cY	-116.91	72.17	198.89
Legs5cX	-26.27	-197.91	5088.95
Legs5cY	0.20	44.17	-1003.31
Legs6aX	0.18	42.66	1001.80
Legs6aY	0.06	38.65	-835.03
Legs6bX	-0.00	40.00	802.33
Legs6bY	-0.00	41.05	-801.80
Legs6bX	-0.00	50.58	443.11
Legs6bY	-1.55	353.44	1441.15
Hor1aX	-5.26	345.39	-1397.83
Hor1aY	-2.57	209.73	931.29
Hor1bX	3.15	108.78	-389.41
Hor1bY	-12.82	103.13	344.10
Hor2aX	-2.71	12.15	-407.31
Hor2aY	-2.81	16.87	-13.51
Hor2bX	0.49	13.22	-473.51
Hor2bY	6.41	12.83	403.91
Hor3aX	-24.26	32.08	-44.44
Hor3aY	-3.58	-59.38	-167.36
Hor3bX	26.14	346.43	596.14
Hor3bY	404.47	646.58	244.14
Hor4X	325.45	283.70	316.86
Hor4Y	-246.81	1376.56	-528.51
Arm1aX	226.08	1301.85	374.93
Arm1bY	-192.12	1543.90	-376.06
Arm2aX	208.45	1918.35	-51.01
Arm2bY	-227.83	617.02	-342.09
Arm3aX	91.29	1366.55	-141.29
Arm3bY			25.26

Summary of Clamp Capacities and Usages:

Clamp Force Allowable Usage

Label	(kips)	(kips)	1
Arm4aP	3.37	100.00	3.37
Arm4bP	3.55	100.00	3.55
Arm3aP	16.03	100.00	16.03
Arm3bP	16.15	100.00	16.15
Arm2aP	16.11	100.00	16.11
Arm2bP	16.27	100.00	16.27
Arm1aP	16.04	100.00	16.04
Arm1bP	16.15	100.00	16.15
101P	0.71	100.00	0.70
102P	1.32	100.00	1.32
103P	0.82	100.00	0.82

103P	1.05	100.00	1.05
104P	0.48	100.00	0.48
105P	1.83	100.00	1.83
105X	1.68	100.00	1.68
105XY	1.68	100.00	1.68
105Y	1.38	100.00	1.38
106P	0.24	100.00	0.24
107P	0.53	100.00	0.53
108P	1.06	100.00	1.06
109P	0.18	100.00	0.18
110P	0.32	100.00	0.32
111P	0.68	100.00	0.68
112P	0.31	100.00	0.31
112X	0.51	100.00	0.51
112XY	0.51	100.00	0.51
112Y	0.31	100.00	0.31

Equilibrium Joint Positions and Rotations:

Joint Label	X-Disp1 (ft)	Y-Disp1 (ft)	Z-Disp1 (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
100P	0	0	0	-0.0518	-0.0960	0.1124	13.87	13.87	0
101P	-0.01503	0.01074	-0.007552	-0.0570	-0.0395	0.1219	-10.98	11.01	15.99
101aP	-0.015	0.01078	-0.01594	-0.0482	0.0238	0.1024	-10.98	10.99	15.99
101bP	-0.009197	0.01031	-0.005505	-0.0181	-0.0391	0.10750	0	0.01021	15.99
102P	-0.01106	0.02233	-0.01103	-0.0733	-0.0139	0.1266	8.189	8.223	31.99
102aP	-0.01091	0.01175	-0.0133	-0.0709	0.0090	0.10607	-0.01091	0.01247	31.67
102bP	-0.003317	0.02347	-0.003748	-0.0420	-0.011	0.10810	8.203	8.234	31.67
103P	-0.009162	0.03332	-0.0135	-0.0937	0.0138	0.1351	6.791	6.833	39.99
103aP	-0.009162	0.0432	-0.0123	-0.0937	0.0244	0.1471	5.399	5.442	47.19
103bP	-0.006833	0.05503	-0.01657	-0.1187	-0.0961	0.1378	3.991	4.055	54.98
104P	-0.01107	0.07225	-0.01863	-0.2182	0.0896	0.1304	3.989	4.072	59.73
104aP	-0.003313	0.08891	-0.02046	-0.2015	0.1495	0.1236	4.003	4.09	64.48
104bP	-0.00362	0.1124	-0.02334	-0.2456	-0.1947	0.1115	3.996	4.124	73.98
108P	-0.01911	0.1349	-0.02342	-0.3793	0.0450	0.1073	3.981	4.147	78.73
109P	-0.01911	0.1692	-0.0265	-0.3550	0.2555	0.1035	3.999	4.169	83.47
110P	-0.002879	0.2112	-0.02785	-0.3778	0.1140	0.0996	4.211	92.97	92.97
111P	-0.004447	0.2405	-0.02923	-0.3700	0.0514	0.0986	4.004	4.21	98.97
112P	-0.000000	0.2672	-0.02928	-0.3479	-0.1173	0.0994	3.998	4.267	105.97
113P	-0.01221	0.05095	-0.1068	-0.4779	0.0098	0.0333	-0.01221	17.55	54.98
Arm1aP	-0.008392	0.1861	-0.02653	0.2085	0.0068	0.0326	-0.008392	18.12	54.98
Arm1bP	-0.01125	0.1165	-0.1654	-0.5972	0.0082	0.0347	-0.01125	20.11	73.98
Arm1dP	0	0.1109	-0.02129	0.2236	0.0084	0.0348	0.013	-20.52	73.98
Arm2aP	-0.007065	0.207	-0.1227	-0.4427	0.0084	0.0345	-0.007065	17.71	92.98
Arm2bP	-0.01446	0.2173	-0.01564	0.0570	0.0089	0.0040	-0.01446	-17.95	92.98
Arm2dP	-0.006784	0.2569	-0.1134	0.0000	0.0000	0.0000	-0.006784	20.27	104.97
Arm3aP	-0.01777	0.2659	0.08689	0.0000	0.0000	0.0000	0.01777	-20.4	105.11
100X	0	0	0	0.0026	-0.0235	-0.1955	-13.87	-13.87	0
100Y	0	0	0	0.0029	0.0437	0.3260	-13.87	-13.87	0
100Z	0	0	0	-0.0056	-0.0325	-0.1910	-13.87	-13.87	0
101X	-0.0005688	0.0104	0.001115	-0.0061	-0.0032	0.1260	-10.99	-10.99	16.01
101Y	-0.0004747	0.001805	0.002791	0.0192	0.0005	0.2608	-11.01	-11.01	16.01
101aX	-0.01495	0.001551	-0.003444	-0.0401	0.0003	-0.0757	-8.2	-8.18	16.01
101bX	-8.738E-005	0.1056	-0.01557	0.0046	0.0003	0.0174	-8.738E-005	-10.69	15.99
102X	-0.0003978	0.02358	0.0031828	-0.0283	-0.0054	0.1474	8.2	8.18	16.01
102aX	-0.000435	0.0157	0.00345	-0.0266	0.0224	0.2144	8.2	8.18	16.01
102Y	-0.01081	0.01597	-0.007803	-0.0685	0.0482	0.0532	-8.211	-8.216	31.67
102bX	-5.48E-006	0.0416	-0.0007145	-0.0272	0.0053	-0.0542	-5.48E-006	-8.18	0.016
102bY	-0.01241	0.016	-0.0001416	-0.0353	0.0352	-0.0211	-8.214	0.016	31.67
103X	-0.0003235	0.03146	0.002147	-0.0311	-0.0459	-0.1175	-6.768	39.4	4.04
103Y	-0.0001796	0.02493	0.003551	-0.0054	0.0568	-0.1839	-6.808	6.825	39.4
103aX	-0.0008296	0.04748	-0.009758	-0.0852	0.0042	-0.0735	-5.396	47.19	5.396
103Y	-0.003313	0.04034	0.00234	-0.0709	0.0217	-0.1287	5.397	-5.36	47.19
104X	-0.003313	0.04745	0.00234	-0.0651	-0.0346	-0.1348	-5.397	-5.36	47.19
104Y	-0.0112	0.0367	-0.0114	-0.0897	0.0456	-0.0835	-5.412	5.417	47.19
105X	-0.005667	0.05697	-0.002959	-0.0993	0.0936	-0.0939	4.006	3.943	54.98
105Y	-0.005667	0.05697	-0.002959	-0.0993	0.0936	-0.0939	-4.005	-3.948	54.98
105Z	-0.005184	0.05237	0.004031	-0.0974	0.1074	0.1664	-4	4.05	54.98
106X	-4.886E-005	0.10504	-0.01588	-0.1459	0.1150	-0.0733	-3.993	-3.993	59.73
106Y	-0.007322	0.05729	0.003893	-0.1723	-0.1075	0.1036	-3.991	-3.927	59.73
106Z	-0.009008	0.06266	0.004987	-0.1717	0.1244	-0.1404	3.994	4.068	59.73
107X	-0.003687	0.05762	-0.01737	-0.2176	-0.0779	-0.0653	3.996	-3.914	64.5
107Y	-0.009248	-0.08115	0.00617	-0.2562	0.1073	-0.0478	-3.991	-3.919	64.5
107Z	-0.009248	-0.08436	-0.01922	-0.2004	-0.1834	-0.1448	-4.009	4.085	64.5
108X	-0.009314	0.1266	0.006097	-0.1893	0.0979	0.0662	-4.003	-3.873	74.01
108Y	-0.009314	0.1266	0.006097	-0.1894	0.0959	0.0614	-4.003	-3.878	74.01

Legs4CX	734.64	1135.26	4711.62	3568.12	11170.90
Legs4CY	-727.48	1182.99	-4720.65	3583.71	-11165.52
Legs4CZ	198.19	-634.05	8063.57	-2274.48	-4295.95
Legs5AX	113.17	-1495.13	-10512.84	-2248.95	-8745.51
Legs5AY	-110.58	-1502.47	10529.48	-2247.85	8742.70
Legs5AZ	29.65	1023.59	4857.05	268.74	10121.92
Legs6AX	116.29	2249.69	8745.28	-18.64	702.55
Legs6AY	-113.62	2248.58	-8742.47	-24.41	-705.63
Legs6AZ	28.42	-258.96	-10121.91	294.38	-5121.69
Legs7CX	116.28	18.68	-702.55	1151.73	1553.93
Legs7CY	-113.60	24.44	705.63	1154.86	-1553.93
Legs7CZ	28.64	-294.25	5221.70	-560.74	1108.00
Legs8AX	-0.20	44.36	-522.71	-45.61	-784.81
Legs8AY	0.19	43.08	524.27	-46.63	784.47
Legs8AZ	0.07	33.73	-785.26	51.83	-396.74
Legs9AX	-0.00	45.61	784.83	0.00	0.01
Legs9AY	-0.00	45.61	-784.45	-0.00	-0.00
Legs9AZ	0.00	-51.83	326.74	0.00	0.00
Hor1AX	-4.25	222.71	925.48	275.41	1103.20
Hor1AY	-5.77	217.89	-900.48	275.26	-1103.23
Hor1AZ	-3.26	188.14	52.74	156.98	134.43
Hor2AX	-0.06	83.66	-272.08	69.22	-46.33
Hor2AY	-0.06	83.66	272.08	69.22	-46.33
Hor2AZ	-9.92	57.64	-226.21	10.85	20.63
Hor3AX	-9.91	36.61	-95.30	10.86	-20.62
Hor3AY	-9.91	36.61	95.30	10.86	-20.62
Hor3AZ	-0.54	4.77	211.60	12.06	578.07
Hor4AX	-0.34	3.53	-171.90	12.03	-578.07
Hor4AY	-1.25	15.74	-37.34	35.38	141.56
Hor4AZ	-1.92	9.12	-332.65	2.31	112.47
Hor5AX	6.58	6.86	309.36	2.57	-103.28
Hor5AY	6.61	19.15	-76.77	2.56	103.28
Hor5AZ	-21.70	31.20	-68.19	-28.62	199.00
Hor6AX	-1.46	-52.06	-138.83	56.52	174.09
Hor6AY	21.57	351.80	480.89	-440.80	1354.98
Hor6AZ	377.58	577.21	196.39	-86.73	658.23
Hor7AX	306.69	286.47	290.28	-266.23	442.05
Hor7AY	-279.08	1556.74	-540.07	83.37	-319.46
Hor7AZ	257.58	1525.94	366.03	70.61	102.21
Arm1AX	-426.70	1778.63	-362.60	106.09	-195.72
Arm1AY	255.37	2170.14	-44.79	60.87	-98.21
Arm1AZ	-246.07	775.66	-317.37	72.79	-181.30
Arm2AX	116.56	1488.49	-329.43	32.63	-281.25

Summary of Clamp Capacities and Usages:

Clamp Force Allowable Usage

Label	(kips)	(kips)	%
Arm4AP	4.06	100.00	4.06
Arm4BP	4.15	100.00	4.15
Arm3AP	17.30	100.00	17.30
Arm3BP	17.37	100.00	17.37
Arm2AP	17.39	100.00	17.39
Arm2BP	17.46	100.00	17.46
Arm1AP	17.31	100.00	17.31
Arm1BP	17.37	100.00	17.37
100P	0.54	100.00	0.54
101P	1.01	100.00	1.01
102P	0.65	100.00	0.65
103P	0.78	100.00	0.78
104P	0.38	100.00	0.38
105P	1.83	100.00	1.83
105X	1.69	100.00	1.69
105Y	1.69	100.00	1.69
105Z	1.53	100.00	1.53
106P	0.19	100.00	0.19
107P	0.42	100.00	0.42
108P	0.79	100.00	0.79
109P	0.14	100.00	0.14
110P	0.25	100.00	0.25
111P	0.51	100.00	0.51
112P	0.24	100.00	0.24
112X	0.34	100.00	0.34
112Y	0.34	100.00	0.34
112Z	0.24	100.00	0.24

*** Analysis Results for Load Case No. 3 "ConEdHurricane" - Number of iterations in SAPS 9

Equilibrium Joint Positions and Rotations:

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
100P	0	0	0	-0.0730	-0.1078	0.1053	13.87	13.87	0
101P	-0.01716	0.01608	-0.007809	-0.0722	-0.0408	0.1174	10.98	11.02	15.99
101BP	-0.01713	-0.002359	-0.01529	-0.0638	0.0324	0.0076	-0.01713	11.11	15.98
102P	0.003642	0.01596	-0.004829	-0.0206	-0.0560	0.1275	11	0.01596	16
102BP	-0.011269	0.03308	-0.01132	-0.0906	-0.0164	0.1184	8.187	8.233	31.59
102AP	-0.01233	-0.02333	-0.01445	-0.0848	0.0119	0.1117	-0.01251	8.223	31.59
102BP	0.005936	0.03412	-0.001804	-0.0511	-0.0184	0.0941	8.206	0.03412	31.6
103P	-0.01056	0.04408	-0.01242	-0.1042	-0.0263	0.1222	6.789	6.844	39.39
104P	-0.001718	0.05698	-0.01359	-0.1136	-0.0384	0.1098	5.398	5.457	47.19
105P	-0.01135	0.07456	-0.01652	-0.1971	-0.0410	0.0950	3.989	4.075	54.98
106P	-0.007443	0.0937	-0.01861	-0.2535	0.1316	0.0956	3.993	4.094	59.73
107P	0.004619	0.1154	-0.02065	-0.2662	0.0889	0.0973	3.993	4.115	64.48
108P	-0.006539	0.1609	-0.02366	-0.2906	-0.1311	0.1009	3.993	4.161	71.98
109P	-0.01065	0.187	-0.02513	-0.3248	0.1029	0.0997	3.989	4.187	78.72
110P	-0.005083	0.2138	-0.02791	-0.3223	0.1403	0.1016	3.995	4.214	83.47
111P	-0.004647	0.2678	-0.02874	-0.3234	0.0744	0.1006	4.006	4.268	92.97
112P	-0.00534	0.3021	-0.02874	-0.3234	0.0744	0.1006	4.006	4.302	98.97
113P	-0.003822	0.335	-0.02888	-0.3095	0.1815	0.1020	3.996	4.335	105
Arm1AP	-0.01318	0.07325	-0.08539	-0.1338	0.0119	0.0340	-0.01318	17.57	54.91
Arm1BP	-0.00779	0.07778	-0.02407	-0.0295	0.0056	0.0336	0.00779	18.09	55.02
Arm2AP	-0.01361	0.1581	-0.1367	-0.4415	-0.0087	0.0373	-0.01261	20.16	73.86
Arm2BP	-0.01307	0.1639	0.053	-0.0723	0.0052	0.0370	0.01307	20.51	74.05
Arm3AP	-0.0084	0.2661	-0.1112	-0.3530	0.0089	0.0377	-0.0084	17.77	92.89
Arm3BP	-0.01479	0.271	-0.06217	-0.1292	0.0094	0.0376	0.01479	17.9	93.06
Arm4AP	-0.008141	0.3341	-0.126	0.0000	0.0000	0.0000	-0.008141	20.33	104.9
Arm4BP	0.01829	0.3346	0.1178	0.0000	0.0000	0.0000	0.01829	20.34	105.1
100XY	0	0	0	0.0142	-0.0697	0.3689	13.87	13.87	0
100Y	0	0	0	0.0449	-0.0801	0.4283	-13.87	-13.87	0
101XY	-0.001946	0.01634	0.001822	0.0042	-0.0623	0.0835	-13.87	13.87	0
101Y	0.001776	0.0167	0.002618	0.0252	-0.0721	0.3703	11	10.98	16
101XY	0.0112	-0.002389	-0.0563	-0.0028	0.0712	-11.02	-11	-10.99	16
101AX	-0.0001282	0.2103	-0.01375	0.0070	0.0008	0.0030	-0.0001282	-10.79	15.97
101BY	-0.02218	0.1424	-0.003131	-0.0114	0.0391	0.0364	-11.02	0.01124	16
102XY	0.000115	0.02488	0.003765	-0.0213	0.0208	0.2537	8.199	-8.166	31.6
102XY	0.0005344	0.02488	-0.007274	-0.0788	0.0533	0.0355	-8.199	-8.171	31.6
102Y	-0.0124	0.02786	-0.007274	-0.0788	0.0533	0.0355	-8.212	8.228	31.59
102AX	-0.0003412	0.02725	0.002675	-0.0151	0.0035	0.0132	-0.0003412	8.127	31.6
102BY	-0.01866	0.02455	0.006672	-0.0401	0.0419	0.0025	-8.219	0.02855	31.6
103XY	-5.636e-005	0.01818	-0.004868	-0.0383	0.0614	-0.1802	-6.8	-6.756	39.4
103Y	-0.0007468	0.03868	-0.005593	-0.0934	0.0717	0.2440	-6.801	-6.761	39.41
104XY	-0.0005455	0.05109	-0.005702	-0.0901	0.0291	0.1409	-5.395	-5.344	47.21
104Y	-0.01358	0.044506	-0.006663	-0.0801	0.0135	0.2056	-5.395	-5.344	47.21
104XY	-0.005873	0.07547	-0.01191	-0.1042	-0.0637	0.0464	-5.414	-5.452	47.19
105XY	-0.006843	0.07082	-0.007555	-0.1564	-0.0413	0.0786	-4.006	-3.925	55.01
105Y	-0.0008155	0.06991	-0.01506	-0.1935	0.0526	-0.0299	-3.999	-4.07	54.98
106XY	-0.004002	0.09153	-0.009125	-0.2346	-0.1220	-0.0520	-3.996	-3.913	59.76
106Y	-0.004319	0.08678	-0.01071	-0.2331	0.1399	0.1194	-4.001	-4.089	59.73
106XY	-0.001472	0.08895	-0.01721	-0.2522	-0.1138	-0.0295	-3.992	-3.886	64.51
107XY	-0.00079	0.1138	0.01072	-0.2908	0.0404	-0.0262	-3.99	-3.891	64.51
107Y	-0.01219	0.1105	-0.01932	-0.2638	-0.0728	-0.0300	-4.012	-4.11	64.48
108XY	-0.003403	0.1619	-0.0127	-0.2651	0.0180	0.0371	-4.008	-3.838	74.01
108Y	-0.001698	0.1558	-0.01393	-0.2652	0.0007	0.0429	-4.003	-3.843	74.01
109XY	-0.003335	0.1845	-0.02237	-0.2907	0.1485	-0.0315	-3.998	-4.156	73.98
109Y	-0.009857	0.1795	0.01352	-0.3005	-0.1382	0.0406	-3.997	-3.815	78.76
109XY	-0.007237	0.1819	0.01477	-0.2998	0.1568	0.0302	-3.99	-3.821	78.76
110XY	-0.004486	0.2123	-0.02366	-0.3244	-0.0852	-0.0293	-3.983	-3.821	78.76
110Y	-0.01252	0.2071	0.01446	-0.3551	0.0791	0.0524	-3.986	-3.788	83.51
111XY	-0.00708	0.2087	-0.01572	-0.3541	-0.0600	0.0193	-3.987	-3.793	83.52
111Y	-0.01361	0.2698	-0.02524	-0.3233	-0.1228	-0.0284	-4.007	-4.209	83.47
111XY	-0.002481	0.2636	0.01487	-0.2969	0.0505	0.0785	-4.014	-3.731	93.01
111Y	-0.005467	0.2626	0.01616	-0.2971	-0.0316	-0.0051	-4.002	-3.736	93.02
112XY	-0.00157	0.3013	-0.02664	-0.3223	0.1083	-0.0283	-3.985	-4.263	92.97
112Y	-0.01458	0.2961	0.01464	-0.3195	-0.0716	0.0790	-3.998	-3.699	99.01
112XY	-0.003672	0.2969	-0.01592	-0.3236	-0.0562	-0.0273	-4.004	-4.297	98.97
113XY	0.0003852	0.33	0.01452	-0.3263	0.2663	0.0771	4.015	-3.665	105
113Y	0.00834	0.3298	-0.02762	-0.3095	0.1995	-0.0287	-3.992	-3.67	105

Joint Support Reactions:

Joint Label X-Force X Y Z-Force Comp. Uplift X-Moment Y-Moment Z-Moment Max.

Tectonic Engineering - conode142-twranslisis

	(kips)	Usage	(kips)	Usage	(kips)	Usage	(ft-k)	Usage	(ft-k)	Usage	(ft-k)	Usage	(ft-k)	Usage	(ft-k)	Usage
100P -25.782	0.0	-25.939	0.0	144.425	0.0	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0
100X 24.550	0.0	-25.923	0.0	-110.372	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000
100XY -23.637	0.0	-23.292	0.0	-106.087	0.0	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0
100Y 24.858	0.0	-24.791	0.0	139.279	0.0	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0	0.000	0.0

Joint Displacements, Loads and Member Forces on Joints:													
Joint X		External Y		External X		Member Y		Member X		Disp.		Disp.	
Label	Load	Load	Load	Load	Force	Force	Force	Force	(ft)	(ft)			
	(kips)	(kips)	(kips)	(kips)	(kips)	(kips)	(kips)						
100P	0.0000	0.3130	-0.5585	25.7815	25.6095	-143.4962	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	
101P	0.0000	0.8130	-0.9658	0.0000	-0.8130	0.9658	-0.0177	0.0161	-0.0078				
101aP	0.0000	0.0000	-0.2422	-0.0000	-0.0000	0.2422	-0.0171	0.0024	-0.0153				
102aP	0.0000	0.0000	-0.2422	0.0000	0.0000	0.2422	0.0066	0.0060	-0.0048				
102P	0.0000	0.0000	-0.5665	-0.0000	-0.0000	0.6466	-0.0127	0.0331	-0.0145				
102aP	0.0000	0.0000	-0.1162	-0.0000	-0.0000	0.1162	-0.0125	0.0233	-0.0145				
102bP	0.0000	0.0000	-0.1162	-0.0000	-0.0000	0.1162	0.0059	0.0341	-0.0018				
103P	0.0000	0.8040	-0.7128	0.0000	-0.8040	0.7128	-0.0166	0.0441	-0.0124				
104P	0.0000	0.0000	-0.3784	-0.0000	0.0000	0.3784	-0.0017	0.0570	-0.0116				
105P	0.0000	1.4340	-0.7865	0.0000	-1.4340	0.7865	-0.0114	0.0746	-0.0165				
106P	0.0000	0.0000	-0.1863	0.0000	0.0000	0.1863	-0.0074	0.0937	-0.0186				
107P	0.0000	0.0000	-0.4173	-0.0000	0.0000	0.4173	0.0046	0.1154	-0.0207				
108P	0.0000	8.7840	-0.7297	0.0000	-0.7840	0.7297	-0.0065	0.1609	-0.0237				
109P	0.0000	0.0000	-0.1626	-0.0000	0.0000	0.1426	-0.0107	0.1870	-0.0251				
110P	0.0000	0.0000	-0.2507	-0.0000	0.0000	0.2507	0.0051	0.2138	-0.0275				
111P	0.0000	0.5160	-0.4619	0.0000	-0.5160	0.4619	-0.0046	0.2678	-0.0269				
112P	0.0000	0.2310	-0.2168	-0.0000	-0.2310	0.2168	-0.0068	0.3021	-0.0287				
113P	0.0000	0.0000	-0.1971	-0.0000	0.0000	0.1971	-0.0038	0.3350	-0.0289				
Atm1aP	0.0000	11.4340	-6.6316	-0.0000	-11.4340	6.6316	-0.0113	0.0732	-0.0241				
Atm1aP	0.0000	11.6844	-6.6315	0.0000	-11.6844	6.7235	-0.0078	0.0778	-0.0241				
Atm2aP	0.0000	11.4340	-6.7235	-0.0000	-11.4340	6.7235	-0.0125	0.1591	-0.0530				
Atm2aP	0.0000	11.7271	-6.7235	0.0000	-11.7271	6.7235	-0.0131	0.1639	-0.0530				
Atm2bP	0.0000	11.4340	-6.6312	-0.0000	-11.4340	6.6312	-0.0038	0.2651	-0.1112				
Atm2bP	0.0000	11.6842	-6.6306	0.0000	-11.6842	6.6306	0.0146	0.2740	-0.0522				
Atm3aP	0.0000	11.6842	-6.6306	0.0000	-11.6842	6.6306	-0.0081	0.3341	-0.1260				
Atm3aP	0.0000	2.1465	-0.8887	0.0000	-2.1465	0.8887	0.0183	0.3346	-0.1118				
Atm3bP	0.0000	0.0000	-24.5502	25.2572	110.8516	0.0000	0.0000	0.0000	0.0000				
100Y	0.0000	0.6654	-0.4495	23.6171	22.6289	106.5369	0.0000	0.0000	0.0000				
100Y	0.0000	0.0000	-0.4495	24.8684	24.7925	-138.8293	0.0000	0.0000	0.0000				
101X	0.0000	1.0152	-0.7628	-0.0000	-1.0152	0.7628	-0.0018	0.0117	-0.0026				
101X	0.0000	0.0000	-0.7628	-0.0000	0.0000	0.7628	0.0018	0.0117	-0.0026				
101Y	0.0000	1.0649	-0.2422	0.0000	-1.0649	0.2422	-0.0001	0.0113	-0.0023				
101Y	0.0000	0.0000	-0.2422	-0.0000	0.0000	0.2422	-0.0022	0.0112	-0.0023				
102X	0.0000	0.9660	-0.6466	-0.0000	-0.9660	0.6466	-0.0011	0.0243	-0.0028				
102X	0.0000	0.0000	-0.6466	-0.0000	0.0000	0.6466	0.0025	0.0243	-0.0028				
102Y	0.0000	0.3899	-0.1162	-0.0000	-0.3899	0.1162	-0.0003	0.0272	-0.0001				
102Y	0.0000	0.0000	-0.1162	-0.0000	0.0000	0.1162	-0.0187	0.0286	-0.0006				
102aY	0.0000	0.8733	-0.5208	0.0000	-0.8733	0.5208	0.0001	0.0442	-0.0049				
103X	0.0000	0.8733	-0.5208	0.0000	-0.8733	0.5208	-0.0007	0.0349	-0.0058				
103Y	0.0000	0.0000	-0.5208	-0.0000	0.0000	0.5208	-0.0096	0.0387	-0.0093				
104X	0.0000	0.7111	-0.3784	-0.0000	-0.7111	0.3784	-0.0055	0.0562	-0.0057				
104X	0.0000	0.0000	-0.3784	-0.0000	0.0000	0.3784	0.0045	0.0511	-0.0067				
104Y	0.0000	0.7111	-0.3784	-0.0000	-0.7111	0.3784	-0.0136	0.0519	-0.0107				
104Y	0.0000	0.0000	-0.3784	-0.0000	0.0000	0.3784	-0.0051	0.0755	-0.0076				
105X	0.0000	1.4478	-0.6201	0.0000	-1.4478	0.6201	0.0059	0.0785	-0.0086				
105X	0.0000	0.0000	-0.6201	-0.0000	0.0000	0.6201	-0.0068	0.0708	-0.0086				
106Y	0.0000	0.7200	-0.1615	-0.0000	-0.7200	0.1615	-0.0039	0.0699	-0.0151				
106Y	0.0000	0.3039	-0.1677	-0.0000	-0.3039	0.1677	-0.0040	0.0915	-0.0052				
106Y	0.0000	0.0000	-0.1677	-0.0000	0.0000	0.1677	0.0043	0.0868	-0.0102				
106Y	0.0000	0.3039	-0.1677	-0.0000	-0.3039	0.1677	-0.0015	0.0889	-0.0172				
107X	0.0000	0.7334	-0.4173	-0.0000	-0.7334	0.4173	-0.0098	0.1138	-0.0119				
107X	0.0000	0.0000	-0.4173	-0.0000	0.0000	0.4173	-0.0079	0.1088	-0.0119				
107Y	0.0000	0.7334	-0.4173	-0.0000	-0.7334	0.4173	-0.0132	0.1105	-0.0193				
107Y	0.0000	0.0000	-0.4173	-0.0000	0.0000	0.4173	-0.0122	0.1105	-0.0193				
108X	0.0000	0.7406	-0.5471	-0.0000	-0.7406	0.5471	-0.0084	0.1619	-0.0127				
108X	0.0000	0.0000	-0.5471	-0.0000	0.0000	0.5471	-0.0034	0.1569	-0.0139				
108Y	0.0000	0.7406	-0.5471	-0.0000	-0.7406	0.5471	-0.0017	0.1558	-0.0224				
109X	0.0000	0.2600	-0.1443	-0.0000	-0.2600	0.1443	-0.0033	0.1845	-0.0135				
109X	0.0000	0.0000	-0.1443	-0.0000	0.0000	0.1443	-0.0099	0.1795	-0.0148				
109Y	0.0000	0.5501	-0.2507	-0.0000	-0.5501	0.2507	-0.0045	0.1819	-0.0239				
109Y	0.0000	0.0000	-0.2507	-0.0000	0.0000	0.2507	-0.0071	0.2123	-0.0145				
110Y	0.0000	0.5501	-0.2507	-0.0000	-0.5501	0.2507	-0.0071	0.2071	-0.0157				
110Y	0.0000	0.0000	-0.2507	-0.0000	0.0000	0.2507	-0.0071	0.2087	-0.0252				
111X	0.0000	0.5475	-0.3426	0.0000	-0.5475	0.3426	0.0136	0.2608	-0.0149				
111X	0.0000	0.0000	-0.3426	0.0000	0.0000	0.3426	-0.0025	0.2636	-0.0162				
111Y	0.0000	0.5475	-0.3426	0.0000	-0.5475	0.3426	-0.0025	0.2636	-0.0162				
112X	0.0000	0.6512	-0.2194	-0.0000	-0.6512	0.2194	-0.0055	0.2626	-0.0266				
112X	0.0000	0.0000	-0.2194	-0.0000	0.0000	0.2194	-0.0055	0.2626	-0.0266				

Hor1aY	-3.10	216.70	63.63	173.50	129.28
Hor1bX	6.27	111.68	-441.77	82.02	-31.26
Hor1bXY	-15.80	103.78	393.15	59.47	20.12
Hor1bY	-16.64	114.44	-65.84	59.51	-20.13
Hor2aX	-3.06	18.93	659.86	30.55	1229.97
Hor2aXY	-2.12	16.34	-581.46	30.46	-1229.97
Hor2aY	-3.04	16.55	-10.22	37.28	133.04
Hor2bX	1.12	11.53	-564.65	-3.35	178.24
Hor2bXY	5.72	11.84	498.96	-2.82	-172.43
Hor2bY	5.79	16.35	48.52	-2.82	172.43
Hor3aY	-30.40	26.76	11.35	-40.05	327.68
Hor3bX	-3.80	-64.80	-190.59	73.11	249.72
Hor5Y	20.82	318.84	906.71	-87.84	1544.87
Hor8Y	460.17	781.19	390.43	479.83	713.83
Hor11Y	368.04	221.83	398.35	-31.69	482.82
Arm1aY	-132.30	726.82	-419.34	42.62	-286.84
Arm1bY	115.90	601.60	271.02	28.10	50.33
Arm2aY	-260.11	709.86	-375.93	65.40	-231.14
Arm2bY	53.57	1056.63	-110.35	11.12	-121.91
Arm3aY	-155.94	53.25	-404.85	46.34	-268.29
Arm3bY	9.30	941.78	-375.74	1.51	-290.89

Summary of Clamp Capacities and Usages:

Clamp Force Allowable Usage Label

	(kips)	(kips)	%
Arm4aP	2.01	100.00	2.01
Arm4bP	2.32	100.00	2.32
Arm3aP	13.21	100.00	13.21
Arm3bP	13.45	100.00	13.45
Arm2aP	13.27	100.00	13.27
Arm2bP	13.52	100.00	13.52
Arm1aP	13.22	100.00	13.22
Arm1bP	13.44	100.00	13.44
100P	0.62	100.00	0.62
101P	1.26	100.00	1.26
102P	0.65	100.00	0.65
103P	1.07	100.00	1.07
104P	0.38	100.00	0.38
105P	1.64	100.00	1.64
105X	1.57	100.00	1.57
105XY	1.57	100.00	1.57
105Y	0.95	100.00	0.95
106P	0.19	100.00	0.19
107P	0.42	100.00	0.42
108P	1.07	100.00	1.07
109P	0.14	100.00	0.14
110P	0.25	100.00	0.25
111P	0.69	100.00	0.69
112P	0.32	100.00	0.32
112X	0.69	100.00	0.69
112XY	0.69	100.00	0.69
112Y	0.32	100.00	0.32

*** Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress

Group Summary (Compression Portion):

Group Label	Group Desc.	Angle Type	Angle Size	Steel Strength (ksi)	Max Usage %	Max Use In Comp.	Comp. Control Member	Comp. Force (kips)	Comp. Control Load Case	L/R Capacity (kips)	Comp. Shear Capacity (kips)	Conn. Bearing Capacity (kips)	RLX	RLY	RLZ	L/R Length (ft)	Curve No.	No. Of Bolts	Of Comp.
1	1	SAE	3.5X3.5X0.25	33.0	52.97	52.97	Legs6aY	-10.700	ConEdEmerI	20.200	0.000	0.000	1.000	1.000	1.000	103.75	6.000	1	0
2	2	SAE	6X6X0.375	33.0	46.95	46.95	Legs5aY	-40.474	ConEdStd(i	86.200	0.000	0.000	1.000	2.000	1.000	60.64	4.750	1	0
3	3	SAE	8X8X0.5	33.0	49.11	49.11	Legs4aY	-100.082	ConEdHurr(	203.800	0.000	0.000	1.000	2.000	1.000	45.60	4.750	1	0
4	4	SAE	8X8X0.5	33.0	58.83	58.83	Legs3aY	-137.418	ConEdHurr(	233.600	0.000	0.000	1.000	1.000	1.000	60.73	8.047	1	0
5	5	SAE	8X8X0.75	33.0	56.72	56.72	Legs2aP	-147.017	ConEdHurr(	259.200	0.000	0.000	1.000	1.000	1.000	122.24	16.095	4	0
7	7	SAE	8X8X0.75	33.0	56.85	56.85	Legs1P	-148.026	ConEdHurr(	260.400	0.000	0.000	1.000	1.000	1.000	125.37	16.507	4	0
11	11	SAE	2X2X0.1875	33.0	43.36	43.36	Dial1bY	-3.989	ConEdEmerI	9.200	0.000	0.000	0.750	0.500	0.500	152.28	10.000	5	0
12	12	SAE	3X3X0.25	33.0	53.34	53.34	Dia8aP	-12.642	ConEdHurr(	23.700	0.000	0.000	0.750	0.500	0.500	125.88	12.420	5	0
13	13	SAU	4X3.5X0.3125	33.0	51.28	51.28	Dia6aY	-23.025	ConEdHurr(	44.900	0.000	0.000	0.750	0.500	0.500	102.08	12.420	2	0
14	14	SAE	3X3X0.25	33.0	74.61	70.77	Dia5bP	-14.721	ConEdHurr(	20.800	0.000	0.000	0.750	0.500	0.500	124.61	12.295	5	0
15	15	SAE	3X3X0.25	33.0	48.01	44.37	Dia4bXY	-6.655	ConEdHurr(	15.000	0.000	0.000	0.750	0.500	0.500	147.44	14.548	5	0
16	16	SAE	3.5X3.5X0.25	33.0	28.03	28.03	Hor3bP	-3.560	ConEdHurr(	12.700	0.000	0.000	1.000	1.000	1.000	235.16	13.600	6	0
17	17	SAE	3.5X3.5X0.25	33.0	26.24	25.15	Dia3aX	-3.522	ConEdHurr(	14.000	0.000	0.000	1.000	1.000	1.000	180.56	10.442	6	0

Insulator Label	Insulator Type	Maximum Usage %	Load case Weight (lbs)
Arm4aP	Clamp	4.06	ConEdBmerIce(int) 0.0
Arm4bP	Clamp	4.15	ConEdBmerIce(int) 0.0
Arm3aP	Clamp	17.30	ConEdBmerIce(int) 0.0
Arm3bP	Clamp	17.37	ConEdBmerIce(int) 0.0
Arm2aP	Clamp	17.39	ConEdBmerIce(int) 0.0
Arm2bP	Clamp	17.46	ConEdBmerIce(int) 0.0
Arm1aP	Clamp	17.31	ConEdBmerIce(int) 0.0
Arm1bP	Clamp	17.37	ConEdBmerIce(int) 0.0
100P	Clamp	0.70	ConEdStd(int) 0.0
101P	Clamp	1.32	ConEdStd(int) 0.0
102P	Clamp	0.82	ConEdStd(int) 0.0
103P	Clamp	1.07	ConEdHurr(int) 0.0
104P	Clamp	0.48	ConEdStd(int) 0.0
105P	Clamp	1.83	ConEdStd(int) 0.0
105X	Clamp	1.69	ConEdBmerIce(int) 0.0
105XY	Clamp	1.69	ConEdBmerIce(int) 0.0
105Y	Clamp	1.53	ConEdBmerIce(int) 0.0
106P	Clamp	0.24	ConEdStd(int) 0.0
107P	Clamp	0.53	ConEdStd(int) 0.0
108P	Clamp	1.07	ConEdHurr(int) 0.0
109P	Clamp	0.18	ConEdStd(int) 0.0
110P	Clamp	0.32	ConEdStd(int) 0.0
111P	Clamp	0.69	ConEdHurr(int) 0.0
112P	Clamp	0.32	ConEdHurr(int) 0.0
112X	Clamp	0.69	ConEdHurr(int) 0.0
112XY	Clamp	0.69	ConEdHurr(int) 0.0
112Y	Clamp	0.32	ConEdHurr(int) 0.0

*** Weight of structure (lbs):
 Weight of Angles*Section DLF: 25691.2
 Total: 25691.2

*** End of Report

**CON EDISON TOWER E-142 / T-MOBILE
W.O. 4216.MASTER (31.18)
EXISTING TRANSMISSION TOWER
MILLWOOD, NY
STRUCTURAL ANALYSIS REPORT – REVISION 1
DECEMBER 19, 2008**

1.0 INTRODUCTION

The Consolidated Edison (Con Edison) electrical transmission tower no. E-142 is located adjacent to Somerstown Turnpike (Route 100) at Station Place in Millwood, NY. The tower was designed as a part of the 345kV double-circuit transmission line system between the Sprain Brook and Millwood Substations. T-Mobile (aka Omnipoint) has an existing antenna installation on this tower.

Tectonic Engineering & Surveying Consultants P.C. has performed a structural analysis of the tower to verify its adequacy for supporting the existing antenna and cable configuration in accordance with Con Edison's requirements.

This revision incorporates field information regarding the existing T-Mobile installation. The previous report for this tower was prepared under Tectonic W.O. 2356.09215A in 2003.

1.1 Information Provided

For the purpose of the analysis, Tectonic retrieved the following relevant information from its files:

1. "Design Drawing, 25° Strain Tower Type "D1", Overhead Transmission Line, Dunwoodie to Millwood, N.Y.", drawing no. EO-7436-C, rev. 1, dated 3/7/67.
2. "Overhead Transmission Line, 1956 Line, 345 kV. Modification, Sprain Brook to Millwood Substations, Design of 2 Circuit, 25° Strain Tower, Type D-1", drawing no. EO-13575-B, rev. 1, dated 4/30/69.
3. "Typical Core Footing in Rock for Transmission Towers", drawing no. EO-7300-D, rev. 6, dated 4/17/56.
4. "Design & Construction Summary, Overhead Transmission Line, Sprain Brook to Millwood West Sub-Sta., 345 kV.-Rebuilt 1956 Line, Sheet 2 of 2", drawing no. EO-13610-A, rev. 5, dated 1/4/82.
5. "Jumper & Suspension Attachments, Assembly with Hardware, Bundled Conductors – 345 K.V.", drawing no. EO-13149-C, rev. 2, dated 2/19/69.
6. "Strain Attachment for Hot Line Operation, Assembly with Hardware, Bundled Conductors – 345 K.V.", drawing no. EO-13148-C, rev. 7, dated 3/30/71.

7. "Purchase and Test Manual No. 6 – Section 21, Lattice Steel Transmission Towers, EO-1018-1", dated 6/21/76.
8. "Con Edison Transmission System – P.C.S. Antenna/Cellular Installation Procedure" May 2000.

2.0 ORIGINAL TOWER DESIGN

2.1 Tower Structure

The existing tower E-142 was designed and fabricated by American Bridge based on Con Edison's standard specifications, and was designated as a Type D-1 25° strain tower. It was originally designed and built circa 1956, and was initially designated as tower #167. The tower was upgraded for double conductors circa 1969.

The tower is a four-legged, self-supporting square tower, consisting of an 89'-0" tall body and 16'-0" long extensions on all four (4) legs. It has a total height of 105'-0" from the base of the leg extensions to the top of the tower. Unless otherwise noted, levels throughout this report are relative to the base of the leg extensions.

The tower is approximately 27'-9" wide at the base. The tower tapers uniformly to a width of 8'-0" at the lower cross arm level, which is 55'-0" above the base. It has a uniform width of 8'-0" from the 55'-0" level to the top of the tower in the longitudinal and transverse faces. The lower, middle, and upper cross arms extend to total widths of 35'-8", 40'-8", and 35'-8", respectively. The shield wire peak has a total width of 40'-8".

The tower utilizes single angles for all members, except for the crossarm girts, which are flat bars. The arm hangers and girts for the conductor arms have been replaced with larger members. All connections are bolted.

A diagram of the structure is presented in Figure 1, attached.

2.2 Loading Criteria

The tower was originally designed based on Con Edison design specifications which provide tower strengths that equal or surpass the requirements of the National Electrical Safety Code for Grade "B" construction. The tower was designed to support two (2) 1/2" diameter EHS Copperweld shield wires and six (6) 1,033,500 CM 54/7 ACSR conductors, with a maximum line angle of 25° and an average span of 900'. The tower was designed to withstand loading

conditions of the original Con Edison specifications, including broken conductors and/or shield wire, with their appropriate overload factors.

We believe that at the time of the conductor upgrade, the tower was designed to support two (2) 7 No. 5 Alumoweld shield wires and six (6) twin-bundled 2493 MCM 54/37 ACAR conductors.

2.3 Tower Foundation

The Con Edison drawings indicate that the tower is supported on a rock foundation. The stub angles are encased in a reinforced concrete shaft, which bears at a maximum depth of 9'-0" below the rock surface.

3.0 EXISTING CONFIGURATION

According to the Con Edison drawings provided, the existing tower was installed at a line angle of 5°22', a wind span of 865', and a weight span of 1155', and is currently supporting the following:

Shield Wire:

9/16", 7 No. 5 AWG Alumoweld (2 positions)
Diameter = 0.546 in, Weight = 0.525 lb/ft

Conductor:

Twin Bundled 2493 KCM 54/37 ACAR (6 positions)
Diameter = 1.821 in, Weight = 2.341 lb/ft

It is believed that the tower centerline bisects the conductor line angle complement.

The tower is also supporting one (1) fiber optic cable, connected to the tower at the 55' level. This is believed to have been installed in 1999, and has the following properties:

Fiber Optic Cable:

288 Fiber ADSS (1 position)
Diameter = 1.042 in, Weight = 0.386 lb/ft

We also understand that T-Mobile has an existing installation on the tower as follows:

- 3 Andrew ADFD1820 panel antennas at the 94' level (centerline), pipe mounted one (1) each on three (3) tower legs
- 12 1-5/8" diameter coaxial cables to the 94' level in Valmont-Microflex Universal Snap-In Brackets on the inside of one (1) tower leg

4.0 PROPOSED INSTALLATION

It is our understanding that no additional antennas or other equipment are proposed to be installed on the tower at this time.

5.0 STRUCTURAL ANALYSIS

5.1 Current Loading Criteria

The design of electrical transmission line structures is governed by IEEE Standard C2-2007 "National Electrical Safety Code" (NESC) and ASCE 10-97 "Design of Latticed Steel Transmission Structures". Con Edison has imposed additional design constraints to accommodate local conditions and to account for special design considerations.

The loading conditions considered in our analysis of the tower are for the wind acting perpendicular (90°) to the longitudinal tower face acting in conjunction with the load due to the line angle. The loading conditions considered, including all applicable code requirements and the requirements stipulated by the utility, are summarized as follows:

Loading Condition (LC)	Pressure (psf)		Radial Ice (in)		Overload Factors		
	Wire	Tower	Wire	Tower	Wind	Weight	Wire Tension
1. Con Ed Standard	4	13	0.5	0	2.54	1.27	1.65
2. Con Ed Emergency Ice	8	20	1.0	0	1.00	1.00	1.00
3. Con Ed Hurricane	30	50	0	0	1.00	1.00	1.00
Note: Wind force on the tower is based on 1.5 times the net projected area of one face.							

The Con Edison Stringing & Sagging condition and Climbing condition were not evaluated in the analysis, because they are not affected by the existing antenna installation.

The design of antenna supports is governed by ANSI/TIA/EIA-222-F-1996 "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures". From this document, a basic wind speed of 80 mph applies to Westchester County, NY where the tower is located. Ice loads have been established based on a 0.5" radial ice thickness in accordance with industry standard practice. A reduced wind speed of 69 mph is used in conjunction with this ice load.

The provisions of ANSI/TIA/EIA-222-F-1996 should be used for the design of the antenna installation. However, the wind and ice loads stipulated by ANSI/TIA/EIA-222-F-1996 are overly conservative for the evaluation of the existing transmission line tower. Therefore, these loads were not considered in our analysis of the tower.

5.2 Procedure

The tower has been analyzed using PLS-Tower, a specialized, three-dimensional structural analysis program. The tower analysis included the existing conductors, shield wires, and fiber optic cable, and the existing T-Mobile antennas and cables described in Section 3.0, using Con Edison loading criteria summarized above.

A breakdown of the total factored loads acting on the tower is provided in the following table:

LC	Conductors (lbs)			Shield Wires (lbs)		
	Weight	Transverse		Weight	Transverse	
		Wind	Wire Tension		Wind	Wire Tension
1	75690	25428	31800	3788	2606	2428
2	87066	26422	28086	5654	2936	2304
3	38448	47256	21348	1212	2362	1254

LC	Fiber Optic Cable (lbs)			Tower (lbs)		Existing Installation (lbs)	
	Weight	Transverse		Weight	Wind	Weight	Wind
		Wind	Wire Tension				
1	2140	1680	1200	32640	13160	1522	2414
2	3380	1756	1148	25680	7980	1198	1485
3	444	2252	628	25680	19940	1198	3713

All conductors shield wires, and the fiber optic cable were considered intact, and no broken wire loading conditions were considered in the analysis.

5.3 Assumptions

Several assumptions were made in order to perform the analysis. Each of these is considered by Tectonic to be both reasonable and consistent with current standards of practice.

1. Redundant members, which are not intended to carry loads, are not included in the model.
2. Tower member sizes and material properties are as indicated on the Con Edison drawings.
3. The wind loads applied to the tower due to the antenna installation are

- conservatively based on the full projected area of all antennas in all loading cases.
4. The tower and its foundation were constructed according to the approved plans.
 5. The connection of the tower to its foundation is considered as pinned.
 6. The conductor and shield wire tensions are assumed to be the same as those used in the design of similar towers upgraded circa 1969.
 7. The fiber optic cable tension is equal to the shield wire tension.

5.4 Results

Our analysis indicates that the Con Edison Emergency Ice loading condition (LC2) is the most severe, therefore resulting in maximum member forces in the tower. The member capacities are as indicated in the Con Edison drawings. Details of the critical member forces in terms of their stress ratios are summarized in the following table:

Member Location	Con Edison Member Number	See Note	Stress Ratio	
			Compression	Tension
			%	%
16'-0" Leg Ext.	7		55	31
	21		60	48
39'-0" Body	4		58	51
	5		54	31
	14		71	74
	15		44	48
	16		25	18
	17		22	23
	18		41	1
	19		42	42
19'-0" Mid Body	3		48	36
	13		51	50
	33		9	36
19'-0" Upper Body	45	1	---	67
	2		47	22
	12		53	51
	34		---	16
12'-0" Body	44	1	---	75
	1		53	3
	11		43	38
	32		---	13
Lower arm	43	1	---	29
	27		46	---
	31	1	---	75
Middle arm	48	1	---	72
	26		49	---
	30	1	---	81
Upper arm	47	1	---	78
	25		38	---
	29	1	---	74
SW peak	46	1	---	72
	24		1	12
	28		73	---

Note: 1. Original member previously replaced with larger size in conjunction with conductor upgrade.

Details of all member forces are included in the computer output attached to this report.

The maximum foundation reactions occur for the Con Edison Hurricane loading condition (LC3), and are summarized as follows:

<u>Direction</u>	<u>Calculated Reaction</u>	
	<u>Per leg</u>	<u>Total</u>
Download per leg	140.9 kips	--
Uplift per leg	106.9 kips	--
Shear (transverse)	27.0 kips	98.8 kips
Shear (longitudinal)	26.8 kips	0.0 kips

Although the original foundation design reactions have not been provided, we have estimated them from the upgraded design loads. The calculated reactions are smaller than the estimated upgraded design reactions.

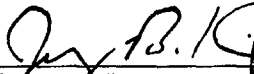
6.0 CONCLUSIONS AND RECOMMENDATIONS

As a result of our analysis, we conclude that the tower has sufficient capacity to support the existing T-Mobile installation under all loading conditions used in our analysis. The tower member stresses are generally low. This is due mainly to the difference between the original design line angle of 25° and the existing line angle of 5°22'. No structural problems for the tower are anticipated, and no modifications are necessary.

We further conclude that the existing foundation has sufficient capacity to support the installation, based on comparison to the foundation reactions resulting from the design loads listed on the Con Edison drawings. No structural problems for the foundation are anticipated, and no modifications are necessary.

Any further changes to the antenna configuration or other appurtenances should be reviewed with respect to their effect on structural loads prior to implementation.

Prepared by: Kavish M. Zavar, P.E.
Sr. Structural Engineer

Reviewed by: 
Jeffrey B. Kirby, P.E., 
Chief Structural Engineer

Date: 12/19/08