APPENDIX A LETCHWORTH

Permits and Regulatory Correspondence

Approval of Plans for Public Water Supply Improvement

hie approval is issued:	under the provisions of 10 NVCDD	Port 5:	
1. Applicant	2. Location of Works (C, V, T)	3. County	4. Water District
Jnited Water New York, Inc.	(T) Haverstraw	Rockland	(Specific Area Served) Ramapo High
. Type of Project			
- 1 Causa	S. 2 Dumning blotte		7 Distribution
□ 1 Source □ 2 Transmission	• •	5 Fluoridation □ 6 Other Treatment ■	8 Storage 9 Other
to approximately a polyaluminum chicoports bypassing a disconnected); instance convert two exist flocculators (three basins with tube a troughs; upsized a continued post fill contact within and standby generator	oride (PACI) coagulant, pota mixing pot (to be demolished tallation of transverse mason sedimentation basins into each) and masonry baffle was ettlers, mechanical sludge recommended to exist pipes from the settle tration additions of sodium and existing clear well; a new runit on a fenced outdoor se	of a 14 inch diameter issium permanganater and a mixing base only walls with 2' 8' to flocculation basins walls sequentially following basins to the expression of the express	r line and a static mixer with e, and future use injection in (to be physically wide by 5' high exits which with variable speed llowed by upflow settling and reinforced fiberglass exit isting duel media filters; soda and Seaquest; chlorine
a. THAT the p this day or a b. THAT the p Issued in ac	approved amendments thereto. roposed works not be placed into occordance with Part 5 of the New Y	omplete conformity with to	he plans and specifications approved
ee attached for Specia	al Conditions c through u.		
	15	SSUED FOR THE STATE	E COMMISSIONER OF HEALTH
June 11, 2007		Doolgrafed Dans	, P.E.
		Designated Repre	esemative
		Michael J. Montysko, PE Chief, Design Section, Bu	reau of Water Supply Protection

6. Type of Ownership						
	□ 68 Priva		Authority	□ 30 Inter	+ +	
□ Municipal □ Commercial			Federal	□ 40 Inter		
□ Industrial ■ 9 Water Works Co			O State		n Reservation	
7. Estimated Total Cost	8. Populati			ainage Bas		
\$4,000,000		300,000		dson Rive		
10. Federal Aid Involved? □ Yes	■ 2 No		oject? ■ Yes	□ 2	No	
		WS	4 9947 issue	a 7/18/06		
Source						
	Class:		13. Est. S	ource Dev	elopment Cost	
☐ Ground Name:			1		·	
14. Safe Yield	15. Descrip	tion				
- 11 - Ga.G 1 (G)G	101 20001.15					
F						
Freatment						
16. Type of Treatment □ 1 Aeration	■ 4 Se	edimentation 7 Ire	on Removal	□ 10 So	ftening	
□ 2 Microst	rainers ■ 5 Cl	larifiers □ 8 C	hlorination		rosion Control	
\ □ 3 Mixing	-	· ·	luoridation	■ 12 Otl	ner	
	8. Max. Treatment		rade of Plant Oper	rator Req.		
	eak 3.0 MGD,		IA & IIA		\$3,000,000	
a	nnual, and 45 M	MG/month				
21. Description: See Remarks 5.						
21. Boodipaon. Ooo Komarko O.						
					-	
Distribution 22. Type of Project Distribution Mains		00 T		04 5	st. Distribution Cost	
☐ 1 Cross Connection	1	23. Type of Storage		24. E	St. Distribution Cost	
		Elevated	gals.			
□ 2 Interconnection		1				
☐ 3 Transmission		Underground	gals.			
☐ 4 Fire Pump Cl₂						
			26. D	esigned for fire flow?		
25. Anticipated Distribution (This Pro				- 1		
25. Anticipated Distribution (This Pro	Man. 45 MO	Deals Of A MA	n	1	□ Yes □ No	
	Max. 45 MG	D Peak 61.2 MG	D		□ Yes □ No	
25. Anticipated Distribution (This Pro	Max. 45 MG	D Peak 61.2 MG	D		□ Yes □ No	
25. Anticipated Distribution (This Pro System Demand: Avg. 31 MGD	Max. 45 MG	D Peak 61.2 MG	D 		□ Yes □ No	
25. Anticipated Distribution (This Pro	Max. 45 MG	D Peak 61.2 MG	D 		□ Yes □ No	
25. Anticipated Distribution (This Pro System Demand: Avg. 31 MGD	Max. 45 MG	D Peak 61.2 MG	D		□ Yes □ No	

SPECIAL CONDITIONS FOR LOG NO. 17981

Letchworth Water Treatment Plant United Water New York, Inc., NY 4303673

Upgrade of Letchworth WTP with a Static Mixer, Variable Speed Flocculators, Tube Settlers, a Diesel Power Generator and various Hydraulic Improvements and Electrical Modifications

(T) Haverstraw, Rockland County

- c. THAT the proposed works be constructed in compliance with all applicable Federal, State and local regulations.
- d. THAT United Water New York shall amend, revise and/or develop emergency plans as necessary to properly account for the new source, new transmission main and other new water system facilities and UWNY shall advise municipalities within the service area of the changes.
- e. THAT locked access to vaults/chambers and other facilities, access gates and other security measures shall be at least annually inspected and reevaluated for adequacy by UWNY, Inc. with findings included within each December operator's report.
- f. THAT a professional engineer shall supervise the construction of this project and shall implement any changes and/or additions agreed to by the New York State Department of Health.
- g. THAT the engineer of record shall provide manuals and parts lists for installed equipment and shall also provide a set of as-built plans (with equipment manufacturers and model numbers noted) to United Water New York, Inc., who shall then maintain and update these documents as needed.
- h. THAT disinfection of all water mains conform to AWWA standard C651, with exclusion of "Sec. 5.1 Tablet Method"; disinfection of the treatment plant conforms to AWWA standard C653; and disinfection of clear wells and other water storage facilities conforms to AWWA standard C652.
- i. THAT the existing mixing pot and the existing mixing basin will be bypassed and physically disconnected.
- j. THAT, in addition to the overflow and drain, each flocculation chamber and each clarifier will be provided a down turned air vents with screens.
- k. THAT a locked manhole or observation port will be placed in each clarifier roof to provide view of the tube settlers and troughs.
- 1. THAT all observation ports and accesses shall be capped or covered with overhanging edges, curbed and locked to prevent entrance of rain, runoff or unauthorized people.
- m. THAT prior to completed works approval, with New York State endorsement, the temporary diesel booster pump will be replaced with a package finished water booster pumping station with two or

- more pumps capable of pumping approximately 1.5 MGD each at 235 TDH with documentation by make and model included within or appended to the as-built plans.
- n. THAT replaced motors for each existing Worthington 3LR-12 backwash water pump will be documented by make and model within the as-built plans.
- o. THAT existing lines labeled for Calgon will be used for Sequest.
- p. THAT all chemical feed lines will be provided with an anti-siphon device in addition to any device which may be within the chemical pump housing.
- q. THAT devices permanently installed at the three reservoirs for accurately measuring storage and releases in accord with WSA 9947 Special Condition 2, may be included within the as-built plans or may be appended to them and temporary stage recording devices may be installed and used for demonstration studies at plant startup.
- r. THAT demonstration studies, based upon performance criteria within New York State "Table A", will be conducted at startup to evidence that the upgraded plant can be operated with polyaluminum chloride (PACl) coagulant to progressively produce potable water at approximately 1.5 MGD, 2.2 MGD and finally 3.0 MGD, according to a proposal endorsed by the New York State Department of Health with intermediate test results daily emailed to the Rockland County Department of Health and the New York State Department of Health and immediate reduction or curtailment of production with any aberrant monitoring observations or test results.
- s. THAT the plant will comply with the operating schedule in WSA 9947 Special Condition 1 with an average annual production not exceeding 1.0 MGD and monthly production not exceeding 45 MG [1.5 MGD monthly average or two weeks at 3.0 MGD].
- t. THAT plant production will be lowered to 0.5 MGD or less each fall/winter so that the flocculation basins and clarifiers may be inspected/repaired.
- u. THAT based upon reservoir operation records, studies of fisheries, and other factors, a critique of the reservoir operation schedule will be annually included within an operators report with refinements to the operation schedule proposed to assure 45 MG is available for taking during August and during September with successive years of a drought.

Flanigan Square, 547 River Street, Troy, New York 12180-2216

Richard F. Daines, M.D. Commissioner

June 11, 2007

Gary Albertson, VP. United Water New York 700 Kinderkamack Road Oradell, NJ 07649

RE: Log No. 17981, NY4303673, WSA 9947
Approval of Plans and Specifications
Upgrade of Letchworth WTP to Seasonal Peak
Operation of 3.0 MGD with Static Mixer, Variable
Speed Flocculators, Tube Settlers and Hydraulic
Improvements and Electrical Modifications
United Water New York
(T) Haverstraw, Rockland County

Dear Mr. Albertson:

We have, this day, approved the plans and specifications submitted by Janine B. Witko, P.E., of Black & Veatch, L.L.P., for a project to increase seasonal peak plant production to approximately 3.0 MGD through addition of a 14 inch diameter line and a static mixer with polyaluminum chloride (PACl) coagulant, potassium permanganate, and future use injection ports bypassing a mixing pot (to be demolished) and a mixing basin (to be physically disconnected); installation of transverse masonry walls with 2'8" wide by 5' high exits which convert two existing sedimentation basins into flocculation basins with variable speed flocculators (three each) and masonry baffle walls sequentially followed by upflow settling basins with tube settlers, mechanical sludge removal to lagoons, and reinforced fiberglass exit troughs; upsized 20" exit pipes from the settling basins to the existing duel media filters; continued post filtration additions of sodium hypochlorite, caustic soda and Seaquest; chlorine contact within and existing clear well; a new transformer, automatic switch and diesel standby generator unit on a fenced outdoor slab; a new booster pump station (two pumps at 1.5 MGD, 235 feet TDH); new backwash water pump motors; various new valves, monitoring and control and appurtenances.

Application for this project was duly made by you and received in this office on January 19, 2007, and final drawings were received on April 6, 2007, with additional documentation subsequently provided.

We call your attention to Standard Conditions a & b and Special Conditions c through u of our DOH form 1017 entitled: "Approval of Plans for Public Water Supply Improvement."

We are enclosing a Certificate of Approval. A set of the approved plans and specifications is being retained in our files, a second set is being sent to the Rockland County Department of Health, and the third set is being returned to your consultant.

Sincerely,

Kenneth W. Caffrey, P.E. Senior Sanitary Engineer Bureau of Water Supply Protection

Enclosure

cc: Black & Veatch, L.L.P., Attn: Janine B. Witko, P.E.

Rockland County Department of Health, Attn: Daniel Miller, Ph.D.

NYDEC- Albany, Attn: Michael Holt, P.E. NYDEC- Region 3, Attn: Michael Merriman

Public Service Commission, Attn: Thomas G. Dvorsky

NYSDOH, Attn: George Philip, P.E.

Palisades Interstate Park Commission Administration Building Bear Mountain, NY 10911-0427 Tel: 845-786-2701 Fax: 845-786-2776

Michael Tesik
Capital Facilities Regional Manager I

FAX: 845-786-5387 February 9, 2007 139099 C-3.11 PIPC

Albert J. Capuzzi, P.E. Project Manager Black & Veatch New York, LLP 120 White Plains Rd., Suite 110 Tarrytown, NY 10591

RE: Proposed Modifications to Letchworth Village WTP

Dear Mr. Capuzzi,

I have reviewed your responses to my January 31, 2007 letter regarding the project noted above.

I will reserve most comments until I receive the revised drawings.

I do, however, have the following comments/questions regarding the standby generator:

- 1. The additional information which you provided is satisfactory to justify the diesel powered unit.
- 2. Because the fuel tank is 1000 gallons, you will need to register the tank (Petroleum Bulk Storage) with the New York State Department of Environmental Conservation. Because the threshold for tank registration is set at 1000 gallons for a facility, any other fuel storage tanks, regardless of size, must also now be registered. Please provide us with a copy of the registration certificate when you receive it.
- 3. The Rockland County Department of Environmental Health has additional PBS testing requirements above and beyond those of the DEC. Please contact them for this information.

Sincerely,

Larry Soeller, P.E. Senior Park Engineer

cc: Jim Hall, PIPC
Michael Tesik, PIPC
Bob Raczko, UWNY

Palisades Interstate Park Commission Administration Building Bear Mountain, NY 10911-0427 Tel: 845-786-2704

Tel: 845-786-2701 Fax: 845-786-2776

Michael Tesik Capital Facilities Regional Manager I

FAX: 845-786-5367 January 31, 2007

> Albert J. Capuzzi, P.E. Project Manager Black & Veatch New York, LLP 220 White Plains Rd., Suite 275 Tarrytown, NY 10591

RE: Proposed Modifications to Letchworth Village WTP

Dear Mr. Capuzzi,

Jim Hall, the Acting Executive Director of the Palisades Interstate Park Commission, asked me to review the proposed modifications to the Letchworth Village Water Treatment Plant and respond to your proposal on his behalf. As per the water plant license agreement between UWNY and NYSOPRHP, all renovations and modifications to the existing facilities must be approved by the Palisades Region (PIPC) of NYSOPRHP.

I have reviewed the Engineer's Report and drawings which were prepared for submission to the NYS and Rockland County Departments of Health. The PIPC defers comment on the needs assessment and feasibility of the proposed improvements to those agencies responsible for protecting the public health. Our comments will be related the site plan, aesthetics, recreational access and environmental protection.

The projects proposed are:

- 1. Improve the hydraulics through the plant to achieve a 3 MGD peaking capacity.
- 2. Upgrade the electrical service and provide a stand-by emergency generator.
- 3. Replace the temporary diesel powered pump with a packaged finished water pumping station and a stand-by emergency generator.

I have the following comments/questions:

- Please provide a Storm Water Pollution Prevention Plan (SWPPP) for all projects.
 The SWPPP must follow the guidelines established by the NYS Department of
 Environmental Conservation (DEC).
- 2. Sheet 2 of 7 shows the overflow from the settling tanks being discharged directly into the Minisceongo Creek. Since this overflow has chemicals added for coagulation, discharge into the creek is regulated by DEC. Please provide a copy of the SPDES permit for this discharge. It would be preferable to discharge the overflow of partially treated water to the backwash settling lagoons.

An Equal Opportunity Employer

Sprinted on recycled paper



- 3. All grassed areas are to be restored with top soil and seeded upon completion of excavation work.
- 4. Sheet 2 of 7 shows the location of the new transformer and generator to be in the middle of the access road for the reservoirs. Please relocate them in order to maintain the road access.
- 5. Please provide an artist's rendering of the proposed transformer and generator so that the visual impacts can be evaluated.
- 6. Neither the drawings nor the engineer's report indicate the specifications for the proposed generator. An LP gas powered generator with an EPA approved sound attenuating enclosure is preferred. Prior to soliciting bids for the generator, please provide a copy of the specification for our review.
- 7. Please provide a site plan showing the proposed route for the new underground electric service from Call Hollow Rd.
- 8. Please provide an artist's rendering of the proposed pumping station and generator so that the visual impacts can be evaluated.
- Prior to construction of the pump station, you must submit plans stamped by a NYS licensed Architect or Engineer and obtain a building permit from PIPC Office of Design and Construction.
- 10. Access to the reservoirs by Park staff must be maintained during all phases of construction.

Sincerely,

Larry Soeller, P.E. Senior Park Engineer

cc: Jim Hall, PIPC

Michael Tesik, PIPC Bob Raczko, UWNY



January 12, 2007

Mr. Kenneth Caffrey
Bureau of Water Supply Protection, Room 400
NewYork State Department of Health
Flanigan Square
547 River Street
Troy, NY 12180-2216

Subject:

United Water New York Short Term Water Supply Program

Letchworth Water Treatment Facility Upgrade Permit Application

Dear Mr. Caffrey:

United Water New York (UWNY) is pleased to submit the attached Engineer's Report and permit application for a project to increase Letchworth plant capacity from 1.0 to 3.0 mgd. The enclosed information includes New York State Department of Health (NYSDOH) "Application for Approval of Plans for Public Water Supply Improvement", and an Engineer's Report completed by Black & Veatch.

The project will take place over an 18-month period, beginning after NYSDOH approval and finishing by mid-2008. The first phase will consist primarily of process, mechanical and hydraulic plant upgrades, which are expected to be completed by June 2007. The second phase will consist primarily of external electrical work, plant automation and installation of a new high lift pump station.

Upon your review of this package, we expect to have detailed design drawings for the first phase available within a week, in order to begin work immediately. Second phase detailed drawings will be available one to two months later.

This is an important project for UWNY, as the increased capacity from the upgrade will be applied directly to our water supply commitments. Upon completion of the first phase, the plant will be commissioned and put on line. We expect this milestone to be achieved by June 2007. Given this accelerated schedule, we would appreciate a determination on the attached package at your earliest convenience.

We look forward to working with you on this important project. If you have questions, please feel free to call me at 201-634-4236, or Michael McDonald at 201-986-4740.

Yours Truly;

Gary Albertson, P.E.

Vice President - United Water

Attachments: Application for Approval of Plans for Public Water Supply Improvement

Engineer's Report

Cc: Dan Miller, PhD, RCDOH

Michael Pointing - GM, UWNY

John Dillon - Corporate Attorney, UW

Alan Weland, P.E. – Director of Engineering, UWNY Michael McDonald, P.E. – Program Manager, B&V

Bureau of Water Supply P.	rotection				Pabl	lic Water Supply Improvement			
Applicant United Water New York		Location of works (Haverstraw (T)	C,V,T)	County Rockland		Water District (area served) Ramapo High (PD20)			
Type of Ownership Municipal Industrial	unicipal								
Modifications to exist New System? If chec If che				NY43036	73				
	a new sys	tem, new water distr If digital boundary	rict, or a distri	ct extension pr s are not avails	_	<u> </u>			
	Private ovide DW	DWSRF**	Federal	l Dothe	er				
Estimated Project Cost Source \$ Pumping \$_500,000		atment \$_3,000,0 incering \$_350.00			age \$ ting \$150,000	Distribution \$ Total \$4,000,000			
Type of Project Source Transmission		Corresion Con Pumping Unit Chlorination		U.V. Di Fluorida Other T		Distribution Storage Other			
Water Treatment plant for sedimentation basins with	om 1 mgd 1 tube settl	to 3 mgd maximum ers. Hydraulic impr	day capacity. ovements incl	The treatment lude replacing	includes retrofi the 8" filter inle	rease the capacity of the Letchworth tting the existing gravity t piping and valves with 12." A new full back-up power and full			
Population Total population of Service area_300	,000	% population actually se	rved_85%_		•	opulation served ected by project 30%			
Avg. day 31 Max. day 45 Peak hr. 61.2	Yea Ye a	r 2006 r 2006 r 2006 r 2006	-	NYS Profes Licensed Es Stamp & Si	ngineer	OE NEW MINISTRAL PROPERTY OF THE BENT MINISTRAL PROPERTY OF TH			
Name of design engineer Janine B. Witko, P.E Address 120 White Plair E-Mail witkoib@bv.c	ns Road, S	uite 110, Tarrytown	NY 10591 Fax No91		. 914-524-8316	OFESSIONA			
Name and title of application Address 700 KINGS				AELTEON, NJ 076		TED WATER,			
		e of Applicant		-		JAN. 11, 2007			

NOTE: All applications must be accompanied by 3 sets of plans, 3 sets of specifications and an engineer's report describing the project in detail. The project must first be discussed with the appropriate city, county, district or regional public health engineer. Signature by a designated representative must be accompanied by a letter of authorization.

*Additional information regarding capacity development may be found at: http://www.health.state.nv.us/nysdoh/water/main.htm

http://www.health.state.ny.us/nysdoh/water/main.htm

^{**}Current DWSRF project listings may be found at:

http://www.health.state.nv.us/nvsdoh/water/main.htm

***By affixing the stamp and signature the Design Engineer agrees that the plans and specifications have been prepared in accordance with the most recent version of the recommended standards for water works and in accordance with the NYS Sanitary Code.

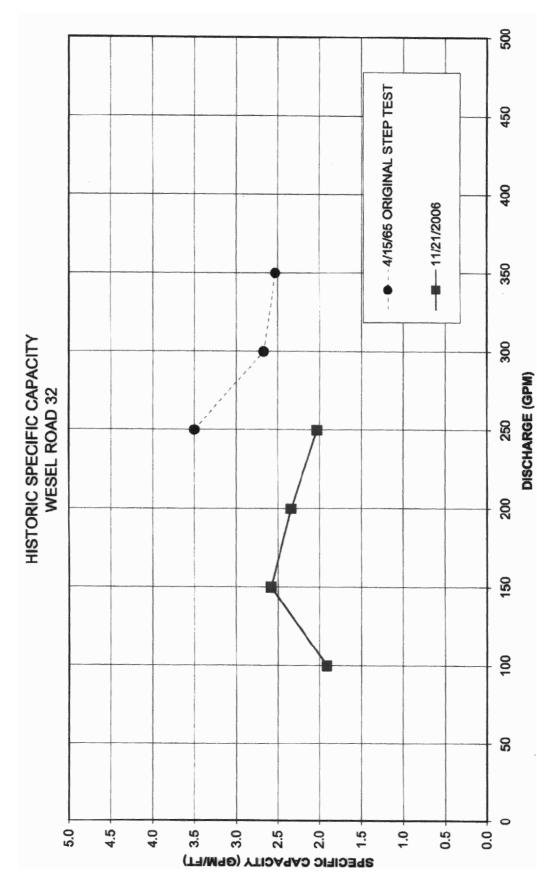
APPENDIX B BEDROCK WELLS

Wesel 32

Wesel 32

Wesel 32 is permitted by NYSDEC for 300 gpm and taps the underlying bedrock aquifer. Recent production capacity indicates Wesel 32 has been utilized at rates between 175 gpm and 225 gpm, less than the original long-term operating capacity estimate of 300 gpm (and permitted rate). Wesel 32 was operating at approximately 175 gpm with an excess of 50 feet of submergence above the pump intake during the August 2001 3-day system peak demand period. Current pumping test data results indicate the specific capacity of Wesel 32 is similar to the results of the original testing, with no significant production-limiting decline noted. Based on the results of original testing, operational history, and results of current testing, Wesel 32 is capable of sustaining the short-term (3-day) peaking at a rate of 300 gpm during a drought demand period similar to August 2001. This 3-day peak pumping rate represents an increase of 125 gpm over the 2001 peak system demand operating rate.

UNITED WATER NEW YORK ROCKLAND COUNTY, NEW YORK



12852M

Celour Length = 175' GI. AR ANTEED FIELD PERFORMANCE: Capacity 225 350 4; Eff 79 %, BHP 24.6 Head 6x6x12 Blectric 30HP PUMP DESCRIPTION: Driver 2/16" × 5"5/16" 1691882 2616465 CURVE Fluid free of gas, air & zbraalvo Proper lateral setting of impolls 1,91/5 × 1,91/5 Laboratory Performance 37225 SCIEVEL CIE SQ 10402 BOME Proper submergence Proper flow to pump suction Peerless Ref. No: 1191/gg × 1191/12S z 37215 S97919Z "31/21 Z 32481 **5919197** WARRAUTT DIA ON. HADBYLLIC PERFORMANCE CURVE IMPELLER 20 30 12 320 Ŷ 998 LOZER

Route 30, P.O. Box 917 • Schoharie, New York 12157 • Phone; (\$18) 295-8288 • Fax: (\$18) 295-8289

May 31, 2007

United Water New York 360 West Nyack Road West Nyack, NY 10994

Attention:

Mr. Robert Raczko, Engineer

Reference: UWNY Wesel #32 Pump Replacement Report

Dear Mr. Raczko:

Layne Christensen Company is pleased to submit this letter report with respect to the installation of a new pump complete with motor, discharge head, steel base plate, 6-inch column piping and shafting. The work was completed on May 24, 2007.

Layne Christensen pulled the existing pump setting installed a new Gould's 10WALC, 8-stage vertical turbine pump cast iron bronze fitted with stainless steel collets and bolting. The design conditions for the new pump are 300 GPM @ 375' TDH with a secondary design point of 230 GPM @316' TDH. The new motor is a U.S. Electric 40 HP, 1800 RPM, premium efficient and inverter duty, compatible for use with a VFD. The installation also included those items listed in the first paragraph. A flow test was conducted as a final step to determine if the new pump was operating on its curve. The results of this test are summarized on Attachment 1. The pumping levels during the flow test were measured in the well with an electric water level meter.

Conclusions

Based on the results of the flow test the new pump is operating above curve. A plot of the data points obtained during the flow test is provided in Attachment 2.

Please contact us should you have any questions or wish to discuss these results in further detail.

We thank you for this continued opportunity to be of service.

Sincerely,

LAYNE CHRISTENSEN COMPANY

Stephen Crook, P.G. District Manager



11147 m 14 moo

ATTACHMENT 1



Pumping Test Record

5/24/2007		UWNY WE	SEL ROAL	WELL				
Time (Minutes)	Flow (Q)	Pressure	Pressure	Pumping		Drawdown	Total Dynamic	Specific Capacity
	(GPM)	(PSI)		Level (FT)	Level (FT)	(FT)	Head (FT)	(GPM/FT)
15	137	186	429.66	60.02	9.92	50.10	489.68	2.73
30	280	131	302.61	116.72	9.92	106.8	419.33	2.62
45	312	114	263.34	138.60	9.92	128.68	401.94	The second secon
	-		0		0.02	120.00	701.04	2.42
		1	to the same of the		L	0	0	#DIV/0!

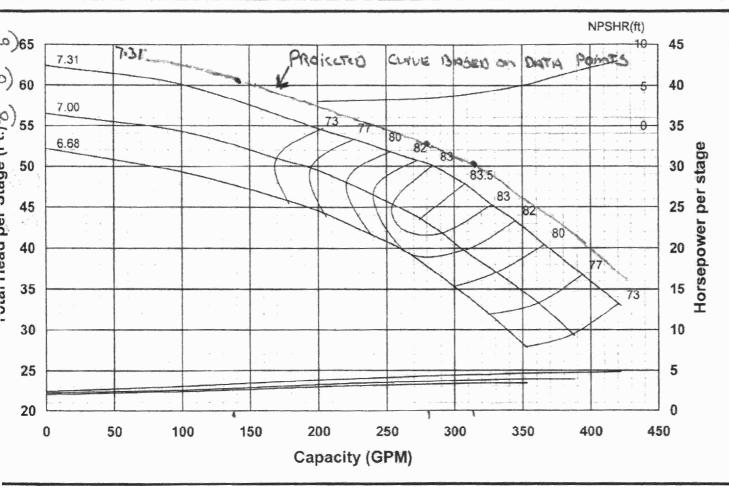


ATTACHMENT 2





IS PROPOSAL NO	GOULDS S.O. NO.	INQUIRY NO.	CUSTOMER PO. NO.	P.O. DATE	ITEM NO.	CUSTOMER	ED WATER	O.F	NEW	YORK		
esel Rom	0 H32	SERVICE	•		300	TY	375		* EFFICIENCY	S	PPM 770	

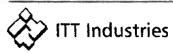


	Curve No. E6410WBPC1							
Model	10WA	LC						
RPM	1770							
EFFICIENC	Y CORRE	CTION						
1-STAGE	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	-3.0						
2-STAGE		-2.0						
3-STAGE		-1.0						
4-STAGE		0.0						
Impeller		SED						
Ns =	1700							
K =	4.60 LE	SS/FT						
K(Bal.)=	N/A							
Bowl O.D.	9,50"							
Bowl Lateral								
Max. PSI	375							
Disch size	4",6",8"							
TURBINE Lubbock, BOWL PERI	Гехаѕ							
BASED ON								
NON-AERA								
RATED POI								
GUARANTE REPRESEN								
PERFORMA								
TEST OF M								
ASSEMBLY								
CORRECTION								
FOR LESSE	R STAGES	S						

Model 10WALC

June 2005

Goulds Pumps



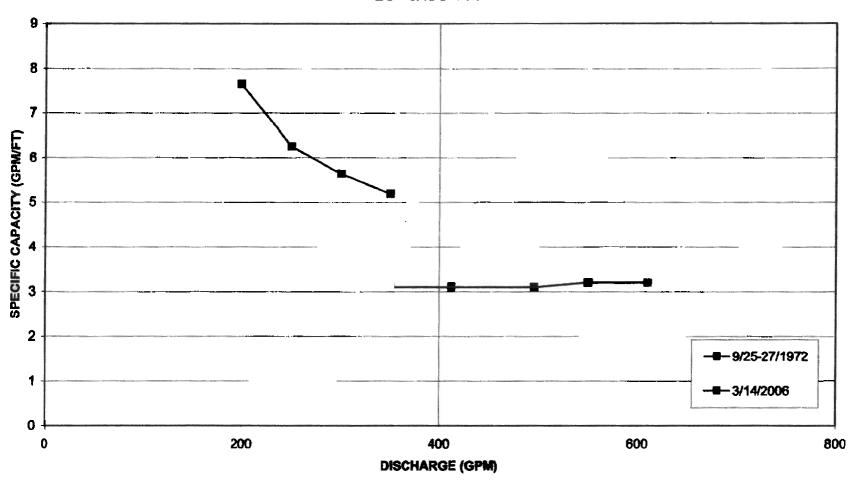
ITT Water Technology, Inc 2 November, 2005 pulds.com .C2 Eckerson 71

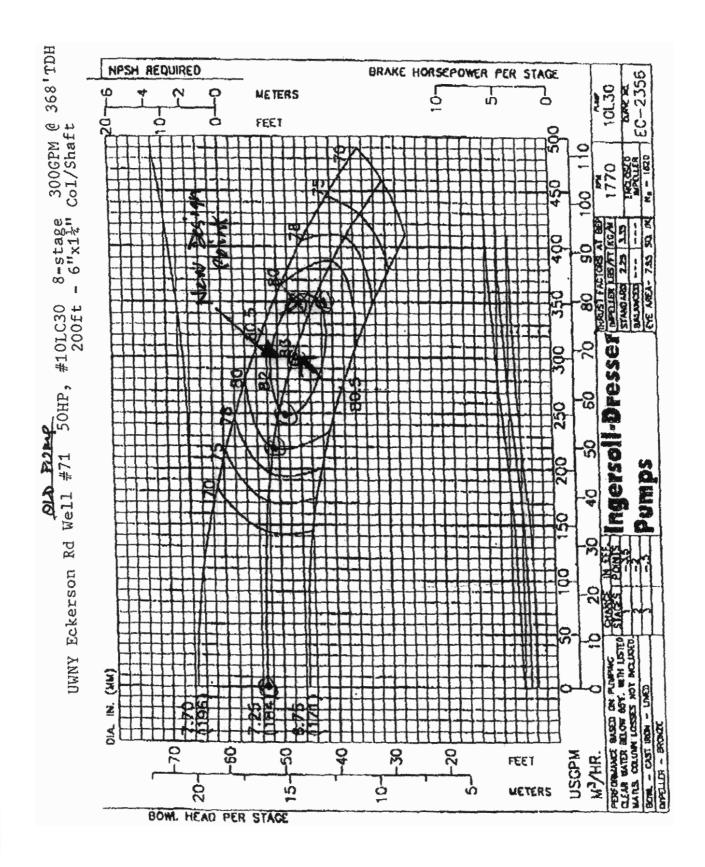
Eckerson 71

Eckerson 71 is permitted by NYSDEC for 500 gpm and taps the underlying bedrock aquifer. Recent production capacity indicates Eckerson 71 has been utilized at rates between 200 gpm and 250 gpm, not at the permitted rate of 500 gpm. Eckerson 71 was operating at approximately 165 gpm with approximately 10 feet of submergence above the pump intake during the August 2001 3-day system peak demand period. Current pumping test data results indicate the specific capacity of Eckerson 71 is similar, if not higher, than the results of the original testing, with no significant production-limiting decline noted. Based on the results of original testing, operational history, and results of current testing, Eckerson 71 is capable of short-term (3-day) peaking at a rate of 300 gpm during a drought demand period similar to August 2001. This 3-day peak pumping rate represents an increase of 135 gpm over the 2001 peak system demand operating rate.

UNITED WATER NEW YORK ROCKLAND COUNTY, NEW YORK

HISTORIC SPECIFIC CAPACITY ECKERSON 71





NBRAND WELL DRILLING COM

ARTESIAN WELL CONTRACTORS pumps • water works installations

14 WALDRON AVENUE GLEN ROCK, NJ 07452-2831 TEL (201) 652-4274 / FAX (201) 445-4591 established 1919

May 31, 2007

United Water New York 360 W. Nyack Road West Nyack, NY 10994

To: Bob Raczko 1-845-623-1500 ex2267 / 1-845-620-3318fax

From:: Steve Rinbrand

RE: Eckerson #71

Bob, The New Gould Bowl Unit #8RJHC-16Stage w/5.25 trim impellers Regular Construction Bowl Unit (300GPM @ 400 TDH) was installed on 5-23-07 and step test on 5-24-07

The Pump appears to pump on curve - see attached curve.

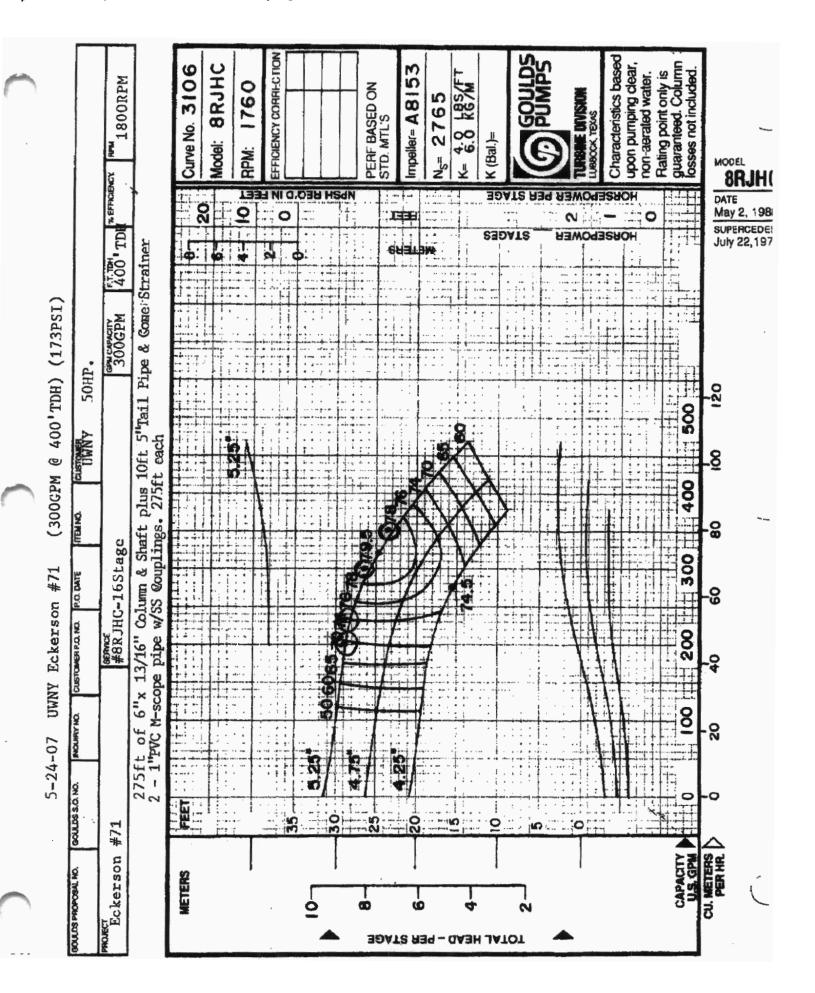
Sincerely,

Stephen Rinbrand

CC: Bill Prehoda @ LBG.



2page fax - hard copy in mail



Rinbrand Well Drilling Co., Inc. 14 Waldron Ave., Glen Rock, NJ 07452 201-852-4274 / 201-445-4591fex Est. 1919

Page 1

United Water New York - Step Test Well No. Eckerson #71 Date 5/24/2007

Line Pressure= Shut Off TDH=

TDH= 497TDH or 215PSI

Time 1:10	PWL'	PSI			1		1			
4:40	The state of the s	FOI	Orfice"	GPM	Time	PWL'	PSI	Orfice	GPM	
1;10	start	175	9.5	200	1:56	58.44	140	29	350	
1:11	31.68	180	9.5	200	1:57	59.17	140	29	350	
1:13	34.35	180	9.5	200	2:00	60.88	139	29	350	
1:14	35	180	9.5	200	2:03	62.62	137	29	350	
1:16	36.14	180	9.5	200	2:06	63.93	137	29	350	
1:20	38.05	180	9.5	200	2:09	65.02	137	·29	350	
1:24	39.4	180	9.5	200	2:10	65.4	137	29	350	
1:25	open up	stokik	rich.	250	Misc Info					
1:26	41.66	177	15	250	1:24	455.2°TDH	1779RPM			
1:27	42.54	176	15	250	1:39	454.3'TDH	1781RPM			
1:30	44.36	176	15	250	1:54	425.8'TOH	1782RPM	Amps 46-45-47		
1:33	45.69	176	15	250	. 2:10	381.9TDH	1775RPM	Amps 47-46.5-49.5		
1:35	46.39	176	15	250						
1:39	47.7	176	15	250						
1:40	орел ир	***	949	300						
1:41	50	161	21.5	300						
1:42	50.79	161	21,5	300		·				
1:45	52.57	160	21.5	300						
1:50	54.72	160	21.5	300				·		
1:54	56.2	160	21.5	300						
1:55	open up	***	***	350						
Type/HP	Turbine 50	HP 1800F	RPM		Orifice"	4ª	Pipe" .	6"		
Pump	Gould #8R	JHC (300	GPM @ 40	O'TDH)	Airline		M-scope	2 - 1"FVC	275fteach	
Stages	16 Stage	(5.25"Trim	impellers)		SWL	23.3ft				
Discharge	6"				Well Día	18"x12" 28"2" of 18"/55'3" of 12" casing				
Setting	275ft				Depth	406₹				
Witness	Max Diebe	r			RWD	Cliff Rinbra	and			

Rinbrand Well Drilling Co., Inc. 14 Waldron Ave., Glen Rock, NJ 07452 201-652-4274 / 201-445-4591fax Est. 1919

Page 1 of 3

United Water New York - Step Test Well No. Eckerson Rd #71

Date 3-14-06 Line Pressure= 74PSI Shut Off TDH= 424'TDH or 183PSI

				-		T		T	
Time	PWL'	PSI	Orfice"	GPM	Time	PWL'	PSI	Orfice"	GPM
10:00	47.42	150	9 1/2	200	11:01	75.3	138	15	250
10:01	54.67	150	9 1/2	200	11:02	76.12	138	15	250
10:02	56.94	150	9 1/2	200	11:03	76.8	138	15	250
10:03	57.78	150	9 1/2	200	11:04	77.25	138	15	250
10:04	58.6	150	9 1/2	200	11:05	77.7	138	15	250
10:05	59.3	150	9 1/2	200	11:06	78.11	138	15	250
10:06	60.02	150	9 1/2	200	11:07	78.45	138	15	250
10:07	20.65	148	9 1/2	200	11:08	78.76	138	15	250
10:08	61.22	148	9 1/2	200	11:09	79.12	138	15	250
10:09	61.79	148	9 1/2	200	11:10	79.4	138	15	250
10:10	62.3	148	9 1/2	200	11:15	80.58	132	15	250
10:15	64.26	148	9 1/2	200	11:20	81.8	132	15	250
10:20	66.23	148	9 1/2	200	11:25	82.77	132	15	250
10:25	67.56	148	9 1/2	200	11:30	83.6	132	15	250
10:30	68.7	145	9 1/2	200	11:35	84.44	132	15	250
10:35	69.88	145	9 1/2	200	11:40	85.03	132	15	250
10:40	70.8	145	9 1/2	200	11:45	85.74	132	15	250
10:45	71.41	145	9 1/2	200	11:50	86.34	130	15	250
10:50	72.2	145	9 1/2	200	11:55	86.94	130	15	250
10:55	72.91	145	9 1/2	200	12:00	87.42	130	15	250
11:00	73.6	145	9 1/2	200	12:01	89	118	21	300
Type/HP	Turbine 50	HP,230/460	V,3PH 180	0RPM	Orifice"	4"	Pipe"	6"	
Pump	IDP #10LC	30			Airline	200ft	M-scope	2-1" PVC 2	00ft each
Stages	8 Stage	(300GPM (@ 368'TDH)	SWL	47.42ft			
Discharge	6"				Well Dia	18"x12" (2	8'2" of 18" /	55'3" of 12'	'casing)
Setting	200ft 6"x 1	1/4"Col/Sh	naft		Depth	406ft			
Witness	Max Diebe	r			RWD	Adam F Ri	nbrand Jr.		

Rinbrand Well Drilling Co.,Inc. 14 Waldron Ave., Glen Rock, NJ 07452 201-652-4274 / 201-445-4591fax Est.1919

Page 2 of 3

United Water New York - Step Test Well No. Eckerson Rd #71 Date 3-14-06 Line Pressure= 74PSI Shut Off TDH= 424'TDH or 183PSI

	1				T		T	T	
Time	PWL'	PSI	Orfice"	GPM	Time	PWL'	PSI	Orfice"	GPM
12:02	89.84	118	21	300	1:02	103.37	93	29	350
12:03	90.44	118	21	300	1:03	104.15	93	29	350
12:04	90.88	118	21	300	1:04	104.75	93	29	350
12:05	91.31	118	21	300	1:05	105.06	93	29	350
12:06	91.67	118	21	300	1:06	105.45	93	29	350
12:07	91.95	118	21	300	1:07	105.78	93	29	350
12:08	92.3	118	21	300	1:08	106.17	93	29	350
12:09	92.28	118	21	300	1:09	106.37	93	29	350
12:10	92.88	118	21	300	1:10	106.68	93	29	350
12:15	94.1	118	21	300	1:15	107.9	93	29	350
12:20	94.95	118	21	300	1:20	109	92	29	350
12:25	95.88	118	21	300	1:25	109.92	92	29	350
12:30	96.77	118	21	300	1:30	110.77	92	29	350
12:35	97.48	118	21	300	1:35	111.53	91	29	350
12:40	98.21	118	21	300	1:40	112.3	91	29	350
12:45	98.9	118	21	300	1:45	113	91	29	350
12:50	99.5	118	21	300	1:50	113.65	91	29	350
12:55	100.12	118	21	300	1:55	114.35	91	29	350
1:00	100.7	118	21	300	2:00	114.86	91	29	350
1:01	102.52	93	29	350					
Type/HP	Turbine 50	HP,230/460	V,3PH 180	0RPM	Orifice"	4"	Pipe"	6"	
Pump	IDP #10LC	30			Airline	200ft	M-scope	2-1"PVC 2	00ft each
Stages	8Stage (30	0GPM @ 3	68'TDH)		SWL	47.42ft			
Discharge	6"				Well Dia	18"x12" (2	8'2" of 18"/	55'3" of 12"	casing)
Setting	200ft 6"x 1	1/4"Col/Sh	aft		Depth	406ft			
Witness Max Dieber					RWD	Adam F. F	Rinbrand Jr.		

Rinbrand Well Drilling Co., Inc. 14 Waldron Ave., Glen Rock, NJ 07452

201-652-4274 / 201-445-4591fax Est.1919

United Water New York - Step Test Well No. Eckerson Rd #71 •

of 3

Date 3-16-06 Line Pressure= 74PSI Shut Off TDH= 424'TDH or 183PSI

Page 3

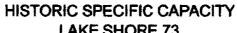
Time	PWL'	PSI	Orfice"	GPM	Time	PWL'	PSI	Orfice	GPM
Recovery					Misc Info				
1 Minute					10:15	Airline 64%			
2 Minute					10:50	RPM=178	39 Amps 37	7-37-38	
3 Minute					10:55	PH=7.0	Temp 53F		
4 Minute					11:00	408.55'TDH	H Drawdown	n 26.15' GI	PF=7.63'
5 Minute					11:15	Airline 5	66%		
6 Minute					11:40	Airline 5	3%		
7 Minute					11:50	RPM=1	786	Amps39	-40-41
8 Minute					11:55	PH=7.0	Temp 53F		
9 Minute					12:00	387.72'TD	H Drawdow	n 40.00' GI	PF=6.25'
10 Minute					12:30	Airline 4	8%		
15 Minute					12:45	RPM=1785 Amps 42-43-44			
20 Minute					12:50	PH=7.0	Temp 53F		
25 Minute					12:55	Airline 47%			
30 Minute					1:00	368.66'TDH Drawdown 53.28' GPF=5.63			PF=5.63'
					1:15	Airline 42%			
					1:45	PH=7.2	Temp 53F		
					1;50	RPM=178	3 Amps 44	-45-47	
					2:00	325.07'TDI	H Drawdow	n 67.44' GF	PF 5.18'
Type/HP	Turbine 50	HP,230/460	V,3PH 180	0RPM	Orifice"	4"	Pipe"	6"	
Pump	IDP #10LC	30			Airline	200ft	M-scope	2- 1"PVC	200ft each
Stages	8 Stage (3	00GPM @	368'TDH)		SWL	47.42ft			
Discharge	6"				Well Dia	18"x12" (28'2"of 18"/ 55'3" of 12"casing)			
Setting	200ft 6" x	1 1/4"col/sh	aft		Depth	406ft			
Witness	Max Diebe	r			RWD	Adam F. R	inbrand Jr.		

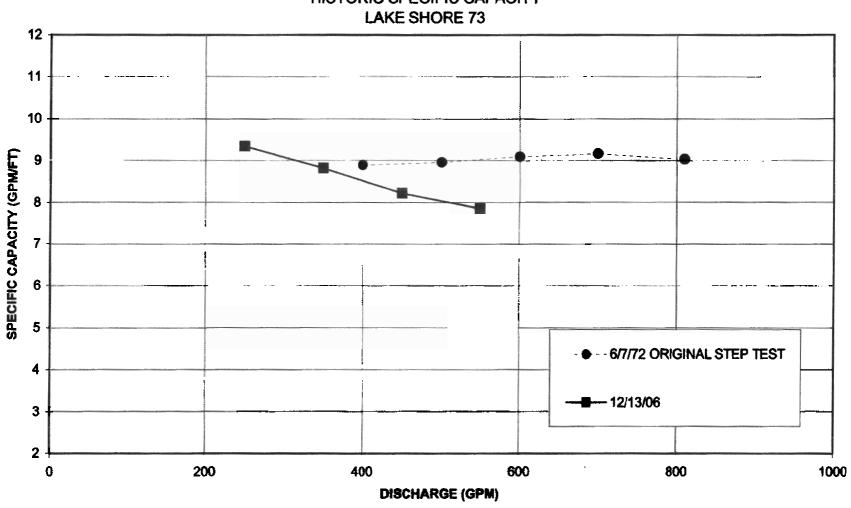
Lakeshore 73

Lakeshore 73

Lakeshore 73 is permitted by NYSDEC for 700 gpm and draws from the underlying bedrock aquifer. Recent production capacity indicates Lakeshore 73 has been utilized at rates between 400 gpm and 450 gpm, less than the original long-term operating capacity estimate of 500 gpm and not at the peaking estimate (and permitted rate) of 700 gpm. Lakeshore 73 was operating at approximately 370 gpm with an excess of 50 feet of submergence above the pump intake during the August 2001 3-day system peak demand period. Current pumping test data results indicate the specific capacity of Lakeshore 73 is similar to the results of the original testing, with no significant production-limiting decline noted. Based on the results of original testing, operational history, and results of current testing, Lakeshore 73 is capable of short-term (3-day) peaking at a rate of 700 gpm during a drought demand period similar to August 2001. This 3-day peak pumping rate represents an increase of 330 gpm over the 2001 peak system demand operating rate.

UNITED WATER NEW YORK ROCKLAND COUNTY, NEW YORK





UWNY - Lakeshore Drive #73 150ft - 6" x 1 3/16"ColShaft 10ft - 6"Tail 500GPM @ 310'TDH Layne #8TCH 13stage 60HP 1800RPM

)	000	JGE	M	6	21,	υ.	תיון	,	ьа у	116	#10	16	H	τ 2	SLA	ge		60	(TE		10	OOI	RPM	ı	
	-	-08	03:	487						EY N									1805					-238	P.1	05/0	9 1	F-\$88			
	*				H	HE	PG	AP VI	500	SHICH	CHA	RACT	MER	IL S	HAME	DER		_		=	54.			=	75	**	4	e !	걐	1	
		1	+	1	Щ	AT A	3	10	LER	ANC	190	PLL	GI A	MI	REG	CAL CAL		e sa		1	- 0	\$		2	- 12	25	<u> </u>	Ne's		<u>~Y</u>	<u>\\</u>
-45		-	-	-	4	POINT	1.80	2	P	LIMP	NO	HON	FAR	2 AC	UBH.				PUHI	خان	3	, F	9	INGE		RP	٠. ١٠	17			اند
235					4	EL. E		(B	* 1	GED	+			+	-	-	-1:		i	}		nelo	PER	FORM	ANG		3/4	لنن	1 6	· · ·	
		+:	1-	-	+		-		~]	- 1		4	عد	<u> 7</u>	24		COL	4	1	- 1	100	7.1	AIVE	- 1	Clea	dey!	900	355	-	75	2
		-		+		===		+	-			4	- 4 10	-			MCD	SUR	100	LIF	140	M	ELE T	PINE	PLUS	TH	- 1	SEN	ACC.	HEA	5
177		-			=		.:	1	+	-	4	+			***	-4	CO.	nmid.	-	INS	بإنب	ENG	ZZ.	فإن	-47	1	SHA	7.0	1		
-	500		4	+		=	-	#	+	+	:	\dashv	}	;	-	`	-	ACITY		-		S. GP	_	HELD.	_		_		 		+
-	-	+	+	+	;-	***	=	#	+	-	끆	-	12	-	2.4	-:-	MO	DR	FE	53	1	14		.10	WAT	H3	इलव्	ENC			-
	1	5	-	1	•			+				4		Ve	4	-	744	60		1	10144	-	=	-		+	-1.	-:-		- :	-
			-	1	. :		-=	#		V	4		-	-	-			=			\Rightarrow	_	=		-		-			_	
3	Qe	4	Ŧ	-	~	\		#	7		7	-	40			-	-	1	-		+	16	-3	7		12		100			
FÉE	1		+	+	-			+	1			\rightarrow	10	P	O		퓜								=		-7				± 62 ·
	1			-	-		=	#	1	7			7.75	-			4	7	1	₹:	interestination					***	11		*****	7	
dA						;. · · ·	1	*		7.7	-				-		-	2	-	-		*		-	;=				7.		
7	2	+	- i	7		1	Fi	车	1	Đ	1	:			X	-						<u> </u>	V		<u> </u>	-	-			·	
		=	-	=		1 · E	1	#		-		•	1.1		-	3			£1.),	-	-		- 1		Va.		-:7	::			17.
-	2	54-	1		•	1:		+		1.		1: :		-	1			1	(P.		=		13.	12		<	1.30	11.	- *	-	
ii.	1 :	+	=			1.2.3	+	-	7						•	-								==	-		1	1.00	-	1.	1
	-2	4	-		` .	1	+		-			+		1		117		113	6	~					***	٠٠.	1		 -	1.	1.0
23	-			EN.	. · · · · · · · · · · · · · · · · · · ·	1.2	-	+		7,2	-	. 14.		-				11.			1.3	7	-			-	1.5	-	1	-	-
-	- 13	-	: 1	FFIC	8	1	+		7.	,			-		1	-		C	5	-		\rightarrow	÷		-	-	1	1-40	+	1	1.22
		-		·w				* 1		. "	712	:		-		5		-			1		è	-	-		7		<u> </u>	-	1 7.
14.	-100	×G	뺖	与	7.6	1	+	= 1	₹.		1.7	2		5					772			A1		×			K				1
		1		REE	7.	1 5		7			1		-		-	1	1:3				15	,E;	. 7	-122	***	-		-			1::
		-	:	-	600	4	(3	-	Ξ								4				The second lives				37,1		-	7.5	<u>ب</u>	1	
==		=			1			773	7	1		-	-		1.3		1	-	-	3	E	-	:5.		72				-	1	77
		= 11	200	÷	3		1:1			1	-24			-			-11	-	1	2.	1 100					-			.7 .0	1	<u> 50</u>
	1					- 3	1				4		1 .	J.::	15			11.7		1		247	- 1	V		D	+	#	1		1.1
-	1				4	/		=	133	7			1	-	1	+	1-	1		-				-	.1	-				1	W.
15					1	1 7			-	1			1.7			i	<u> </u>	1							W				_!	-	13
	3			1	10			7.	1	1:		+		-	-	1	T.	-			Po	w	IE.		1	Ċ.]:	1	1	-6-6	-1.0
_			1				7				70.		1			7	7	-	1	1	1	-		1	7.00		-	7			D.
-		1	-	1.2	1				-		1	T	ļ.,	1.			1			+	1.	<u> </u>	7.	1	-	1 =	74.	_	7.	He	া যু
5		1	-				- 1		1	-	3		- ==		- 3	·					-5	F	-	4 401 5			7	7		1	1 by
	7				7	-	- 4		1		_						3	T			103	17.		-		-	T.				कर क
- 10	1		=	A			7		-	1.2		-	-	-	F		1.	: 7			1=	=						+	-	1	++
	17.4	-				10		- 11	r.b.				-	-		-1					1.70	-	-	-	~~~	شملت	-	-	<u> </u>		-
	137 1			-	7				I.	T				1	1.	-			11:2	. 1.	-	. -			13	.]	_			۔ لمار	
TAL GENERAL				-	J	2				40		.[.;	: 1 3	iec	1. 5	G/	ulc	ico Svs	REI	· ·	- =	iaa IF			6	ماء	+	- 1		200	1

Route 30, P.O. Box 917 * Schoharie, New York 12157 * Phone: (518) 295-8288 * Fax: (518) 295-8289

June 12, 2007

United Water New York 360 West Nyack Road West Nyack, NY 10994

Attention:

Mr. Robert Raczko, Engineer

Reference: UWNY Lakeshore #73 Pump Replacement Report

Dear Mr. Raczko:

Layne Christensen Company is pleased to submit this letter report with respect to the installation of a new pump complete with motor, discharge head, steel base plate, 6-inch column piping and shafting. The installation work was completed on May 16, 2007 and a final flow test run on June 11, 2007.

Layne Christensen pulled the existing pump setting installed a new Gould's 8RJHO, 5-stage vertical turbine pump cast iron bronze fitted with stainless steel collets and bolting. The design conditions for the new pump are 700 GPM @ 415' TDH with a secondary design point of 500 GPM @350' TDH. The new motor is a U.S. Electric 100 HP, 3600 RPM, standard efficient and compatible for use with a VFD. The installation also included those items listed in the first paragraph. A flow test was conducted as a final step to determine if the new pump was operating on its curve. The results of this test are summarized on Attachment 1. The pumping levels during the flow test were measured in the well with an electric water level meter.

Conclusions

Based on the results of the flow test the new pump is operating on curve. A plot of the data points obtained during the flow test is provided in Attachment 2.

Please contact us should you have any questions or wish to discuss these results in further detail

We thank you for this continued opportunity to be of service.

Sincerely,

LAYNE CHRISTENSEN COMPANY

Stephen Crook, P.G. District Manager



named and man

ATTACHMENT 1



Pumping Test Record

Time	Flow (Q)	Pressure	Pressure	Pumping	Static	Drawdown	Total Dynamic	Specific Capacity
(Minutes)	(GPM)	(PSI)	(FT)	Level (FT)	Level (FT)	(FT)	Head (FT)	(GPM/FT)
31	699	150	346.50	85.20	10.30	27.17	431.70	NA
35	550	175	404.25	72.90	10,30	62.60	477.15	8.79
40	498	194	448.14	65.50	10.30	55.20	513.64	9.02
52	403	204	471.24	54.30	10.30	44.00	525.54	9.16

ATTACHMENT 2



Company: CHRISTENSEN PUMPS Customer: United Water, Lakeshore 73

Name: MEMPHIS, TENNESSEE

Date: 04/11/07 Order No:



Pump:

8RJHO (5 stages)

Lineshaft Synch speed: 3600 rpm

Speed: 3530 rpm Dia: 5.0625 in

Curve: 3005

Specific Speeds: Ns: 2825

Pump Notes for Standard Sizes:

Suction Size-5" Discharge Sizes-5".6"

Vertical Turbine:

420

280

3530

3530

527

545

Bowl size: 7.5 in Max lateral: 0.63 in Thrust K factor: 5.3 lb/ft

Pump Limits for Standard Construction:

Temperature: 120 °F Sphere size: 0.43 in

Pressure: 425 psi g

Search Criteria:

Flow: 700 US gpm Head: 415 ft

Secondary Operating Point: 500 US gpm, 350 ft

Fluid:

Water Temperature: 60 °F

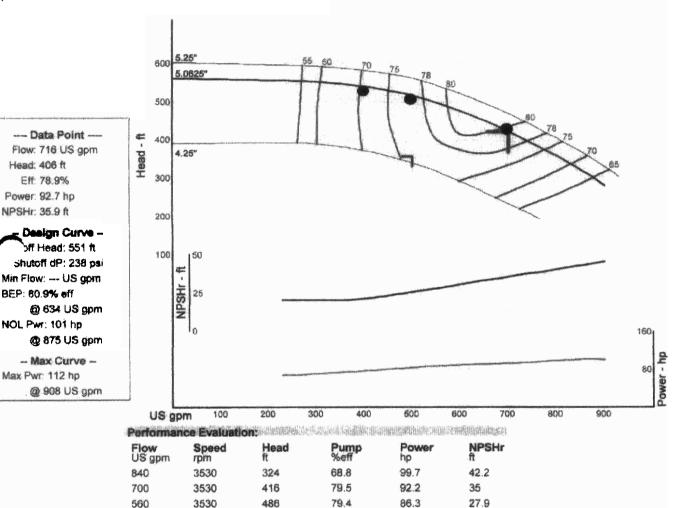
Vapor pressure: 0.2563 pei a Atm pressure: 14:7 psi a SG: 1 Viscosity: 1.105 cP

NPSHa: --- ft

Motor:

Size: 125 hp Speed: 3600 Standard: NEMA

Sizing criteria: Max Power on Design Curve



72.4

56.3

77

68.3

21.5

19.7

Permits and Regulatory Correspondence

٧	E١	х	"	V	n	T	Ł	7	S	7	٠,	١.	т	Ċ	ĭ)1	ĸ.	P	A	R	ľ	М	n	Ċ	٧	1	٠(D	И	1	H	Ù	14	ď	Ľ	T	Ή	1

Application of Approval of Plans for Public Water Supply Improvement

ureau of water Supply Fi		es pro-essential and a superior of the superio	rubile water supply improvement	792
pp leant of \$100 has	Location of works (C, V, T)	31 STORESTON AND ASSESSMENT OF THE PROPERTY OF	Water District (specific area served)	
nited Water New York	Clarkstown (T) - Lakeshore 73, Wesel 32	Rockland	United Water New York	1
	Ramapo (T) - Eckerson 71			
				_
perof Ownerships	Private - Other	Authority	☐ Interstate	
THE PROPERTY OF THE PARTY OF TH	Commercial Private - Institutiona	Federal	☐ International	
		☐ State		
Industrial	Water Works Corp. Board of Education		Native American Reservation	4
Modifications to existing	system. If checked, provide PWS ID # NY	NY4303673		
New System. If checked	provide capacity development (viability) analy	rsis*		
	system, new water district, or a district extensi		otion location details in	-
	py Disk. If digital boundary location details are			
=	☐ Digital CAD Data Provided ☐ Other Digital C		nt Description Provided	1
unding Source Pri				
	hecked please provide DWSRE#	d .		-
The Thirt is Ministrated				
AND SEASON STATES	Estimated Pro	(President Association States and Associatio		TECO.
ource	Treatment was not - in the way	Storage	Distribution 2 The Park of the	
TO THE PARTY OF TH		California de la califo		5498
umping'	Engineering The County of the	Legal/Permitting	Total Print Control of the Control o	张 名
\$200,000	.00			
ype of Project	☐ Corrosion Control ☐ U.V.	Light Disinfection	Distribution	
Source	✓ Pumping Unit ☐ Fluo	ridation	☐ Storage	1
Transmission	☐ Chlorination ☐ Other	er Treatment	Other	
	Replace pumps and motors at three well sit	es in UWNY system (Lakes	shore 73, Wesel 32 and Eckerson 71) to	1
Project Description	provide more flow for peak demand.			
Total	Population of Service Area	298,000		-
\$3002395_500A0064E3948A5_245A005863455_3499444685A0323A3446925E_30	opulation actually served	90	OF NEW	1
THE RESIDENCE OF A SEPTIMENT AND ADMINISTRATION OF THE RESIDENCE OF THE RESIDENCE OF THE PROPERTY OF THE PROPE	fulation affected by project	5	The same of the sa	1
			A FRANCIS	
Latest Total	l Consumption Data (in MGD)		100 8 400	
verage Day	Year We William 148	5	* #	1/200
		NYS Professional	11 = 12 11 20 11 10 11 10	The same
31.5	2006	Licensed Engineer		47
iximum Day	Keare and the season of the season of	Stamp and	To the second of	II.
		Signature***	00000	
44.8	2006		CLESSON	li
eak Hour	36.1795. 国格及《中国海域》			
57.2	2006			
37.2	SCORE STATE OF THE			=
	Robert F Raczko, P.E.			1
	TOURING			-
Name and Address of Des	UWNY 360 West Nyack Road			1
Engineer.	300 West Nyack Road			_
	West Nyack, NY 10994			
	845-623-1500 x2267 email: bob.raczko@u	mitedwater.com		
			5/24/2007	
A CONTRACTOR OF THE PROPERTY O	Signature of Applicant	进程1000年6月20日	Application Date of the service of the service of	1.4
OTE: All applications must be acc	companied by 3 sets of plans, 3 sets of specifications and an	engineer's report describing the p	project in detail. The project must first be discussed with	h
e appropriate city, county, distric	or regional public health engineer. Signature by a designat	ed representative must be accomp	anied by a letter of authorization.	
	capacity development may be found at:	http://www.health.state.nv.us/r		
*Current DWSRF project listings		http://www.health.state.ny.us/r		
	ture the Design Engineer agrees that the plans and specifica		dance with the most recent	
ersion of the recommended stands	rds for water works and in accordance with the NYS Sanita	ry Code.		

Bob Raczko, PE Operations Engineer

UNITED WATER
360 West Nyack Road, West Nyack, NY 10994
Tel: 845.623.1500 x2267 • Fax: 845.620.3318
bob.raczko@unitedwater.com



May 25, 2007

Dan Miller, Ph.D Rockland County Department of Health Environmental Health Division Dr. Robert L. Yeager Health Center 50 Sanatorium Rd Pomona, New York 10970

Subject: Pump Modifications at Three Well Sites

Dear Dr. Miller:

As requested, we have prepared a DOH Form 348 for three pump modification projects that we are undertaking to increase the peaking capacity of our water supply by June 15, 2007. These projects include:

- Wesel 32 Provide new pump, larger motor and VFD to provide an additional 0.18 mgd
- Eckerson 71 Provide new pump, larger motor and VFD to provide an additional 0.19 mgd
- Lakeshore 73 Provide new pump, Larger motor and VFD to provide an additional 0.48 mgd

Please note that the increase in capacity will capture the full, permitted amount of capacity, above and beyond that which has historically been realized at these wellfields, but not exceed the permitted capacity. The pumps have been identified as candidates for replacement because they represent pumps which could not achieve the desired Total Dynamic Head (TDH) required to meet desired flow conditions, as well as there being sufficient hydrogeologic capacity available to capture. The former was determined by comparing field pumping data to manufacturer's data, and the latter being determined by reviewing years of SCADA and specific capacity data.

The required TDH was obtained by assuming worst case, peak pressure conditions in the system, then modeling the impact of the new capacity to the system in terms of pressure layered on top of those conditions. From there the pressure required to meet peaking conditions was obtained from the model, as represented by the TDH requirement below (this also assumes maximum drawdown in the wells). In all cases, the pressure design exceeds that which was seen in the system during August 2001 conditions, and also under maximum pressure conditions. Were August 2001 conditions to occur again, these new pumps would meet their full permitted capacities and exceed the 2001 flow by the amount noted above.

A summary is shown below:

Pump ID	Old Pump	,	Larger Capac	city Pump	Aug 2001	Increase
	Flow (gpm)	TDH (ft)	Flow (gpm)	TDH (ft)	Capacity	(mgd)
Wesel 32	225	350	300	375	175	0.18
Eckerson 71	300	368	300	380	170	0.18
Lakeshore 73*	500	310	700	395	370	0.48

^{*}A copy of each of the permits has been attached for your reference. Note that a review of the permit for Lakeshore 73 indicates this well is permitted for 700 gpm, as opposed to 500 gpm.

If you have any questions or desire additional information, please contact me at 845.623.1500 X2267, or Michael McDonald, Program Manager, at 201.986.4740.

Yours Truly,

Robert F. Raczko, P.E. Operations Engineer

Att: Water Supply Permits

cc: Sam Rulli, P.E. - RCDOH Vito Spadavecchia - UWNY

Mike McDonald, P.E. - Black and Veatch

Bill Prehoda, P.G. - LBG



APPENDIX C CATAMOUNT WELLS 42 AND 54

APPROACH

Two existing wells comprise the Catamount Well Field, Wells 42 and 54 and the NYSDEC listed permitted rates are 510 gpm and 400 gpm, respectively. The Catamount wells tap the underlying unconsolidated sand and gravel aquifer. The historic capacity of the Catamount Well Field declines significantly with time due primarily to iron-related bacteria biofouling and iron hydroxide precipitates clogging the well screens. The declining specific capacity necessitated the rehabilitation of the wells on numerous occasions in order to regain capacity. Currently, both Catamount 42 and 54 exhibit the lowest recorded specific capacities to date.

To provide the ability to rehabilitate the Catamount Well Field wells on a more frequent basis to maintain production capacity, with minimal lost production time, a more efficient specific capacity maintenance program at the Catamount Well Field is being implemented. The capacity maintenance program being conducted includes a thorough rehabilitation of the Catamount wells utilizing carbon dioxide injection (AquaFreed®) with surge block and simultaneous discharge, followed by the installation of carbon dioxide injection pipes into each well. The injection pipes are part of the AquaGard™ system to periodically rehabilitate the well with carbon dioxide injection. The pumps remain in the well during the carbon dioxide injection and are utilized to purge the well after injection. The Aqua Gard™ process allows for more frequent carbon dioxide treatment while minimizing production down time.

The frequency of the Aqua Gard™ treatments will depend on the degree of specific capacity decline noted after the rehabilitation, however we anticipate that the first AquaGard™ injection will potentially occur at Catamount 42 as soon as three months after the current rehabilitation is completed. The frequency of the AquaGard™ rehabilitations will vary depending on the results, with the goal to maintain the maximum specific capacity with no additional permanent loss.

A complete AquaFreed® carbon dioxide injection and surge block rehabilitation were completed at each Catamount 42 and 54 prior to the installation of the Aqua Gard™ system at each well. The full rehabilitations insured the wells are operating at maximum capability prior to implementing the Aqua Gard™ system. The results of the prerehabilitation pumping tests conducted indicated the lowest specific capacities recorded to date. The pumping test results also indicated that the pump in Well 54 was operating off curve and inspection of the Well 42 pump showed shaft replacement and column replacement was necessary.

The rehabilitation results indicate that the maximum operating pumping rate at Catamount 42 was increased 25 gpm (from 175 gpm to 200 gpm) and Catamount 54 capacity increased from 135 gpm to 350 gpm. The rehabilitation results are similar to those obtained during the last major rehabilitation effort in 2004, but significantly less

than the original capacity. The results of the complete rehabilitations are summarized in the attached historic specific capacity graphs.

The existing pumps at both Catamount 42 and 54 were replaced with new submersible pumps, which were tested during the post-rehabilitation testing, and are operating on curve. The replacement pump curves are attached.



June 15, 2007

Mr. Vito Spadavecchia United Water New York 360 W. Nyack Road West Nyack, NY 10994

RE: Catamount #42A & #54A

Dear Vito,

The new Grundfos 230S400-13 was installed in well #42A on 6/12/07 and a step test was done on 6/13/07. The pump is operating on curve.

A new Berkeley 7T60-350 was installed in well #54A on 6/14/07 and a step test was done on 6/15/07. This pump is operating on curve.

Sincerely,

Stew Cataria

Steve Catania
V.P. Operations
Subsurface Technologies, Inc.

Attachments

Cc: Bill Prehoda

SC/dc

40 Stone Castle Road, Rock Tavern, NY 12575 845.567.0187 Fax: 845.567.1035 http://www.subsurfacetech.com









Catamount 42 Pump replacement test 6/13/2007

				Pumpi	ng	
Flow		Pressu	ıre	Level		TDH
GPM		PSI		Feet		Feet
	0				3.56	
	99		340	24	4.03	809.43
	150		310	33	3.03	749.13
	200		269	4	5.87	667.26
	225		250		53	630.5

Catamount 54 Pump replacement test 6/15/2007

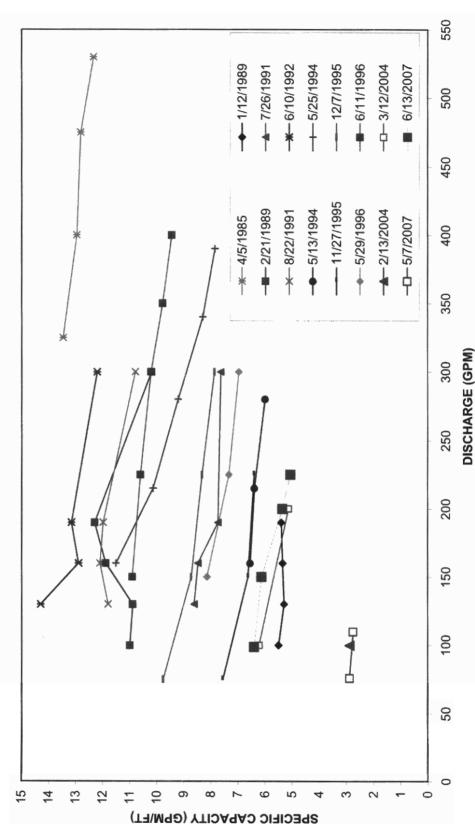
			Pumping	
Flow		Pressure	Level	TDH
GPM		PSI	Feet	Feet
	0		7.36	
	100	340	23.78	809.18
	200	290	42.42	712.32
	350	190	71.09	509.99

SPECIFIC CAPACITY AND PUMP CURVES

LEGGETTE, BRASHEARS AND GRAHAM,



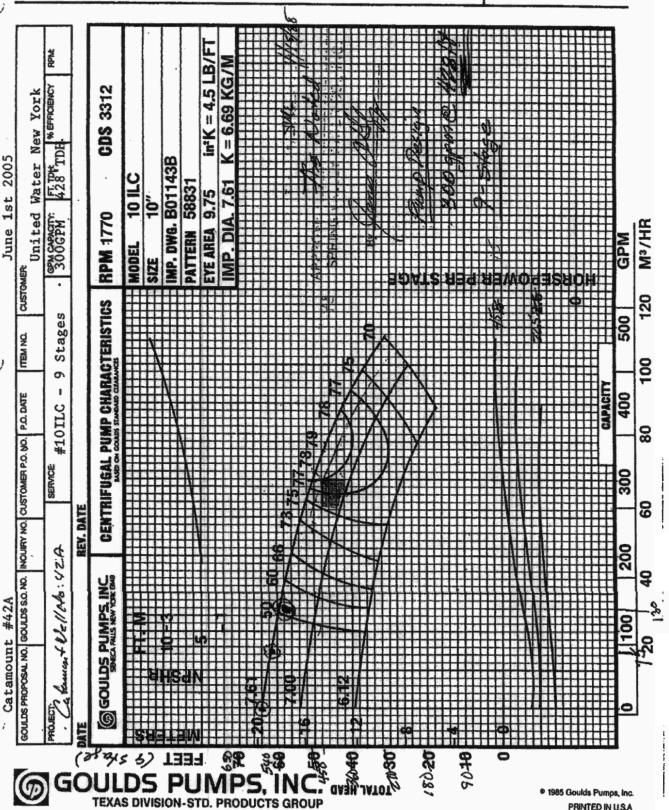
HISTORIC SPECIFIC CAPACITY CATAMOUNT 42



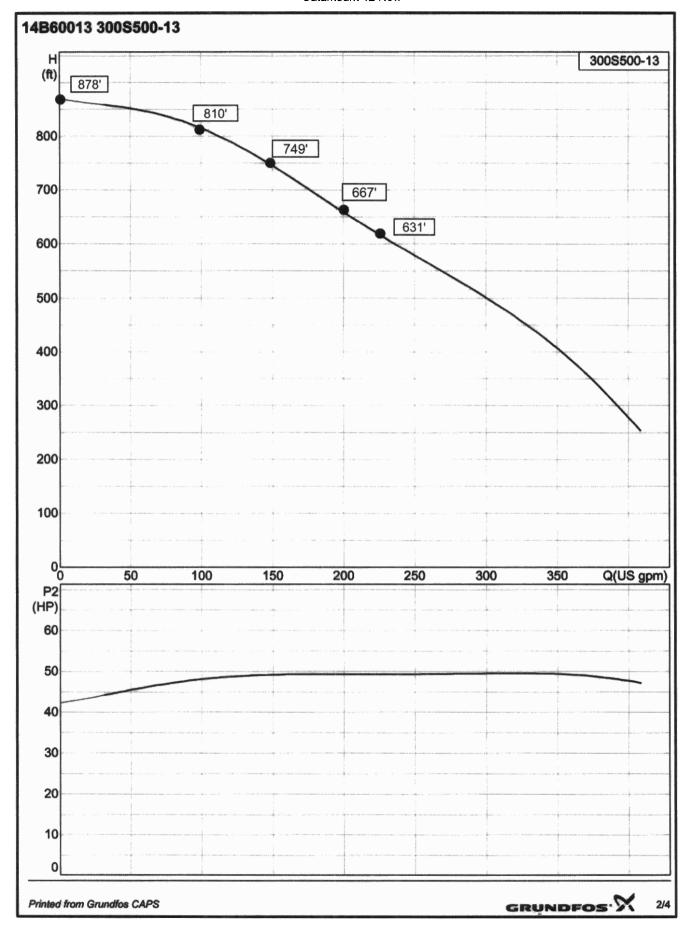


5C32.1a

July 1985 (New)



Position	Count	Description		Unit price
	1	300\$500-13		5463
			Product photo could vary from the actual product	
		Product No.: 14B60013 Multi-stage submersible pump for	raw water supply	
		groundwater lowering and pressu	re boosting. The	
		pump is suitable for pumping clea liquids without solid particles or fil	in, thin, non-agressive pers.	
		The pump is made entirely of Sta DIN WNr. 1.4301 DIN WNr. an horizontal and vertical installation The pump is fitted with a built-in n	d suitable for	
		Liquid: Maximum liquid temperature:	104 °F	
		Technical:		
		Speed for pump data:	3450 rpm	
		Rated flow: Rated head:	299.4 US gpm 505 ft	
		Curve tolerance:	ISO 9906 Annex A	
		Materials:		
		Pump:	Stainless steel 1,4301 DIN WNr. 304 AISI	
		Impeller:	Stainless steel 1.4301 DIN WNr. 304 AISI	
		Installation:		
		Pump outlet: Motor diameter:	4" NPT 6 inch	
		Minimum borehole diameter:	6" in	
		Electrical data:		
		Power (P2) required by pump:	50 HP	
		Start. method:	direct-on-line	
1				



Catamount 42 New

Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data:	0 300 Q(US gr
EAN number:: 5700391740961 800 Technical: Speed for pump data: 3450 rpm 700 Rated flow: 299.4 US gpm Flow range: 41.2 412 US gpm Flow range: 41.2 412 US gpm 800 Rated head: 505 ft 700 Rated head: 500 Rated head: 50	
Technical: Speed for pump data: 3450 rpm Rated flow: 299.4 US gpm Flow range: 41.2412 US gpm Rated head: 505 ft Curve tolerance: ISO 9906 Annex A Stages: 13 Model: A Valve: pump with built-in non-return valve Materials: Pump: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Installation: Pump outlet: 4" NPT Motor diameter: Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 "F Electrical data: MEMA Too 0 To	
Technical: Speed for pump data: 3450 rpm Rated flow: 299 4 US gpm Flow range: 41.2 412 US gpm Rated head: 505 ft Curve tolerance: ISO 9906 Annex A Stages: 13 Model: Valve: pump with built-in non-return valve Materials: Pump: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Installation: Pump outlet: Motor diameter: 6" in high multiplication of the first	
Speed for pump data:	
Rated flow:	
Flow range: 41.2 412 US gpm Rated head: 505 ft Curve tolerance: ISO 9906 Annex A Stages: 13 Model: A Valve: pump with built-in non-return valve Materials: Pump: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: 4" NPT Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data: MEALS 600 600 600 600 600 600 600 6	
Flow range: 41.2 412 US gpm Rated head: 505 ft Curve tolerance: ISO 9906 Annex A Stages: 13 Model: A Valve: pump with built-in non-return valve Materials: Pump: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Installation: Pump outlet: 4" NPT Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data: MEANS	
Rated head: 505 ft Curve tolerance: ISO 9906 Annex A Stages: 13 Model: A Valve: pump with built-in non-return valve Materials: Pump: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: 4" NPT Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data: MSA4 S500 400 400 400 400 400 400 4	
Curve tolerance: ISO 9906 Annex A Stages: 13 Model: A Valve: pump with built-in non-return valve Materials: 300 Pump: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Installation: 100 Pump outlet: 4" NPT Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: 50 Maximum liquid temperature: 104 °F Electrical data: 40	
Stages: 13	
Model: Valve: pump with built-in non-return valve Materials: Pump: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI Installation: Pump outlet: Motor diameter: Motor diameter: Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data:	
Valve: pump with built-in non-return valve 400 Materials: 300 300 Pump: Stainless steel 300 1.4301 DIN WNr. 304 AISI 200 Impeller: Stainless steel 1.4301 DIN WNr. 100 304 AISI 100 100 Installation: Pump outlet: 4" NPT 0 50 100 150 200 250 3 Motor diameter: 6 inch P2 (HP) Minimum borehole diameter: 6" in 50 40 Electrical data: 40 40	
Materials: Pump:	
Pump: Stainless steel 1.4301 DIN WNr. 304 AISI 200 200 200 200 200 200 200 200 200 20	
Pump: Stainless steel 1.4301 DIN WNr. 304 AISI 200 200 200 200 200 200 200 200 200 20	
1.4301 DIN WNr. 304 AISI Stainless steel 1.4301 DIN WNr. 304 AISI 200 Installation: Pump outlet: Pump outlet: 4" NPT Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data:	200
Minimum borehole diameter: 104 °F 200 20	200 0/15
Impeller: Stainless steel 1.4301 DIN WNr. 304 AISI 100	200 0/15
1.4301 DIN WNr. 304 AISI 100	200
304 AISI	200 0/10
Installation: Pump outlet: Motor diameter: Minimum borehole diameter: Maximum liquid temperature: 104 °F Electrical data:	200 0(15
Pump outlet: 4" NPT Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data:	200 0//10 20
Pump outlet: 4" NPT Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data:	200 0(1)0
Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data:	200 0(10 ==
Motor diameter: 6 inch Minimum borehole diameter: 6" in Liquid: Maximum liquid temperature: 104 °F Electrical data:) 300 OH 5 M
Minimum borehole diameter: 6" in (HP) Liquid: Maximum liquid temperature: 104 °F Electrical data:	, 000 Q(00 g)
Liquid: Maximum liquid temperature: 104 °F Electrical data:	
Maximum liquid temperature: 104 °F 50 40	
Maximum liquid temperature: 104 °F 50 40	
Electrical data:	
Electrical data.	
Applie modern	
Power (P2) required by pump: 50 HP	
Start. method: direct-on-line 20	
Others:	
Sales region: Namreg 0	an ann an mainte ann an Maria ann an ann an an an an an an an an an
V	
	; <u> </u>

14B60013 300S500-13 **4" NPT** All units are [mm] unless otherwise presented. Printed from Grundfos CAPS



LEGGETTE, L. ASHEARS & GRAHAM, INC.

SHEET 1 OF 2

6 ARROW ROAD, SUITE 103 RAMSEY, NEW JERSEY 07446 800-818-8524 www.lbgweb.com

Date: 6, 13, 07 Pumping Test of Well: 42A CATAMOUNT Well Number: 42A M Pumping Dobservation Job Code: Well Location: Street: RAMAPO Town: State: Measurements Taken By: Well Depth: 70 Well Location: Pump Setting / Type: Pure Installation: Power Source: Well Construction: 53 Feet Below 766 Portable Generator **DU Outdoors** Cverburden Messored Reported ☐ Temporary ☐ Well Pit Feet Below TOC Vertical Turbine De Permanent Electric Utility ☐ Badrock Pump Manufacturer: Other: Pump House Open Hote Inner Casing Diameter: Submersible ☐ Basement ☐ Screened Screen Diameter: ☐ Suction Other: Screen Setting: 53'- 68 Jet Horsegower / Model: ☐ Gravel Packed Drilled By: ☐ Télescoping Other: Pipe Size Method of Water-Lev Method of Flow Measurement: Distance From Pumping Well (ft): Type of Test: Measurement: Water-Level Measuring Point: M-SCOPE PORT ☐ Constant-Rate Electric Tape ☐ Flow Meter Static Water Level: Step /00 130,200 250 - Wetted Tape - Gallon Bucket/Drum Elevation of Monitoring Point: O. 1 ABOVE PUMP BASE ☐ Slug ☐ Airline Orifica Welr Pump On (Date/Time): 6/13 17:55 Pump Off (Date/Time): Recovery ☐ Transducer Orifica Table: Other: Other: Other: Discharge Notes (Observer, Water Quality, Weather, Elapsed Depth to Drawdown Discharge Notes (Observer, Water Quality, Weather, Depth to Drawdown Date Time Date Time Time (min) Rate (gpm) Discharge Quality, etc.) Time (min) Rate (gpm) Diacharge Quality, etc.) Water (ft) (ft) 29.16 6/13 182 32.82 1740 8.66 150 310 psi Shut off press = 380 pil 26 32.73 24.07 27 1822 23.84 1756 32.50 24.09 99 1823 28 32.75 32.80 24.14 1757 26.63 17.57 2 99 340 050 1824 17 58 3 24.55 6.29 90 1825 32.85 24.19 150 30 1830 17 59 57.5 - 54,0 - 62.1 Ans 32.90 24.33 35 310 psi TANI 749 S.C. 6-13 1035 40 32.13 29.47 24,08 18 00 5 15.42 29.54 1801 23.99 1533 1836 38.20 6 700 1857 41.61 72.91 1802 99 43.15 34.43 15.33 1838 8 1803 35.32 1804 15.34 1839 44 .00 24.02 15.36 44.49 35.83 200 262 psi 1840 45 1805 10 74.09 343 psi 44.84 36.18 1810 15 184 24.13 15.47 TOU - 810' 5.6. = 6.40 47 45.11 36.45 200 1847 1819 20 1816 21 21.04 150 1843 45.3 36.65 1844 269 psi 14.48 45.4 36.75 1817 49 200 2388 20 45.50 36.84 302 psi 1845 1818 1 5 24.54 45.80 37.14 269 PSV 24 33.20 150 1850 53 1819 1855 (58) 33,52 200 TOU = 667 24.66

рессетте, Вр. Велея & GRAHAM, INC.



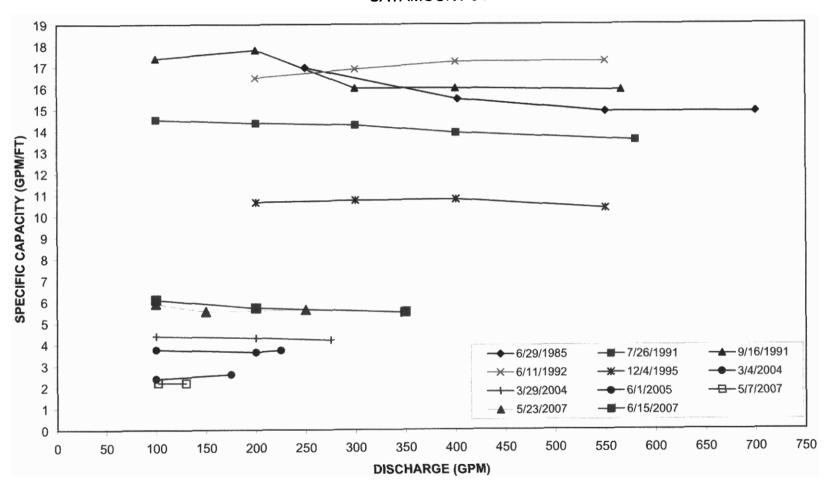
6 Arren, Mew Jerefy 07446 www.legwer 07446

						L			It'lh	24.02	20	2581	
								225	60.04	343h	19	1820	51/9
Notes (Observer, Water Quality, Weather, Discharge Quality, etc.)	Olscharge Rafe (gpm)	Drawdown (ff)	Depth to (f)	besqeili (nim) emiT	emil	Date	Notes (Observer, Water Quality, Westher, Discharge Quality, etc.)	Olschauge (mqg) ətsA	Orawdown (ft)	Depth to Water (ft)	beagel3 (nim) emiT	9miT	ased
										7	184: 21	ENTS TAKEN	MEASUREM
					State: N		OWN: RAMADO	Ĺ		202	(A) :10	TION: Stre	MELL LOCA
TO 1 5/1 9 STAD		uoj	Observat		Pumping	N ASK	WELL NUMBER: (MANOUNT						10B CODE;
			моэ.а	BGME	1.WWW.	,	RAMSEY, NEW JERSEY O'A						

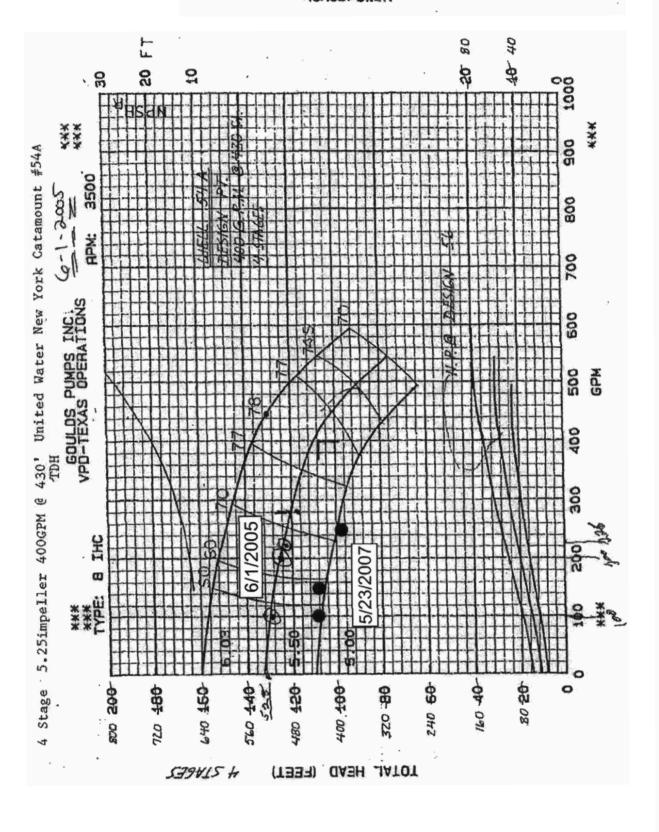
:					324 0 25	255	% 5h 18 5h 18 5h 18 5h 10 5h 10 5h 10 5h 10 5h 10 60 0h	19 15 15 15 16 15 10 15 10 15 21 15 21 05 51 8h	04 69 89 49 99 59 69 29	2061 2061 2061 2061 1061 0061 6531 8581 2581	51/9
-						325	88.84 18.84 18.84 18.84 10.84 th.74	#5.12 #4.12 #5.12 #0.12 #0.12 #0.12 #0.12 #0.12 #0.13	69 89 49 99 59 69 49	1001 1001 1001 1000 1000 1823	
:						<u>इस</u>	88.84 18.84 74.84 45.84 88.84 10.84	#3.12 #4.12 #4.12 #4.12 #0.12 #0.12	69 89 10 99 59 10 59	4061 4061 2061 1061 0061 (531	
:					750 052	225	88°Sh 18°Sh 18°Sh 18°Sh 18°Sh	#8.52 #4.52 #5.52 #5.52 #6.52 #6.52	69 89 49 99 59	4061 4061 2061 1061 0061	
					750 052	225	88°64 18°64 76°64 65°64	45°25 £4°25 £6°25	69 89 49 99	4061 4061 2061 1061	
	·				750 052	225	88.84 13.84 74.84	45°25 25°28	69 89 49	5061 5061 2061	
	·				750 0 52		13°SH	45°25	69 89	4061	
	·				750 052	260	88 8/4	45.52	69	1061	
	,					266				The second secon	
1		1				266	26.54	29'65	OŁ	3061	
					1000	000					
					750 057	400	hit hh	23.90	SE	0161	
					90'5 : 75 189 = HIL	522	hhihh	01.62	OS	5161	
					CIGI DUATHUS			41 60 60	MORN		
					7.9 Hd .95 142L			32.10		9/61	
					4 mod : 00 48:00 F			19.25	-2	1161	
								19.05	8	8/6/	
								39.21	lx	6161	
						-		386	3	0261	
								3h'6	9.	1261	
								57.6	8	2261	
								31.6	6	1261	
								81.6	01	5261	
			-								

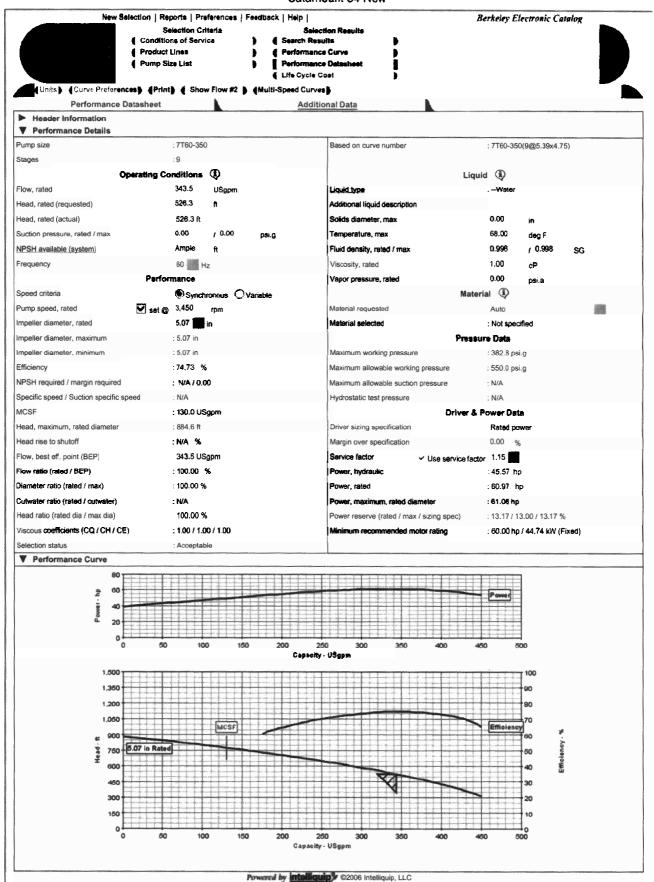
UNITED WATER NEW YORK ROCKLAND COUNTY, NEW YORK

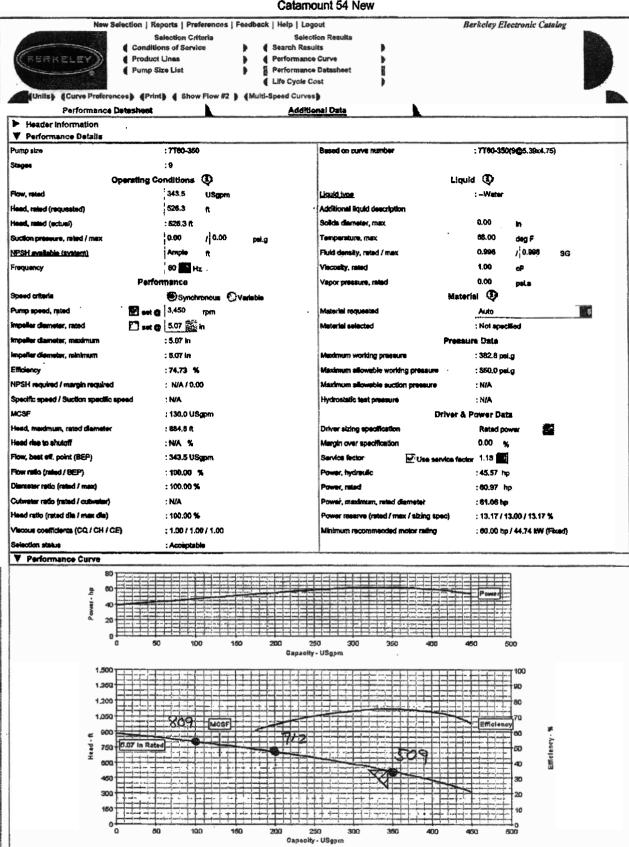
HISTORIC SPECIFIC CAPACITY CATAMOUNT 54



HORSEPOWER







Powered by intelliquip @2006 Intelliquip, LLC

REGULATORY CORRESPONDENCE

IEW YORK STATE DEPARTMENT OF HEALTH				Application of Approval of Plans for Public Water Supply Improvement
pplicant	Location of works	CV.D	County	Water District (specific area served)
Inited Water New York	Ramapo (T)		Rockland	United Water New York
ype of Ownership		Private - Other	Authority	Interstate
T Garachai	Commercial	Private - Institutional	_	International
	Water Works Corp.	Board of Education	State	Native American Reservation
Modifications to existing	system. If checked, pr	ovide PWS ID # NY	NY4303673	
New System If checked,				
this project involves a new				
igital format on CD or Flopp		arrations.		
□ Digital GIS Data Provided unding Source ☑ Priv	☐ Digital CAD Data Provi	Federal Other	ita Provided L. i.e.	ext Description Provided N/A
	hecked, please provide			
Section 1		Estimated Proj	lect Cost \$	TO COMPANY TO STATE OF THE STAT
ource	Treatment	A CARLON CONTRACTOR	Storage and the same of the	Distribution
The state of the s		Figure 1 Annual Communication	and the second second second second	
umping	Engineering		Legal/Permitting	Total Total
\$25,000. ype of Project	The second secon	Пих	· · · · · · · · · · · · · · · · · · ·	
Source	☐ Corrosion Control ☐ U.V. I. ☐ Pumping Unit ☐ Fluori		Light Disinfection	☐ Distribution ☐ Storage
Transmission				Other
Project Description	Replace pumps and flow for peak dema		ne UWNY catamount Wel	lfield (Catamount 42 and 54) to provide more
Total P	Total Population of Service Area % population actually served % population affected by project		298,000	
			90	OF NEW L
% рорг			5	LE CRANCIS CO
Latest Total	l Consumption Data (i	in MGD)		6/6 40
verage Day	Year			
31.5		2006	NYS Professional Licensed Engineer	
faximum Day	Year	2000		
dia sia na managama	ani Carifornia Sko	PARTY AND STREET	Stamp and Signature***	
eak Hour	Year	2006		
al lan kir	TCAL			
57.2		2006]	
	Robert F Raczko, P	E.		
	UWNY			
Name and Address of Desig	360 West Nyack Ro	oad		
Engineer			-	
	West Nyack, NY 10 845-623-1500 x226	0994 67 email: bob.raczko@un	itedwater.com	
Robert	Robert Rayles			6/12/2007
Signature of Applicant		Date		
OTE: All applications must be acco	ompanied by 3 sets of plans,	3 sets of specifications and an e	ngineer's report describing the p	project in detail. The project must first be discussed with
e appropriate city, county, district of	or regional public health engi	ineer. Signature by a designated	representative must be accomp	anied by a letter of authorization.
\dditional information regarding or 'Current DWSRF project listings m		found at:	http://www.health.state.ny.us/n http://www.health.state.ny.us/n	
Current Dware broject assuigs to	may no noming ac.		min y www.ncann.mate.ny.uwn	VSUUD WALCESTIALISTICS

*By affixing the stamp and signature the Design Engineer agrees that the plans and specifications have been prepared in accordance with the most recent rision of the recommended standards for water works and in accordance with the NYS Sanitary Code.

OH - 348 (02/05)



APPENDIX D SYSTEM WELL CAPACITY MODIFICATIONS

Donald F. Distante, P.E. Director, Master Planning

UNITED WATER
700 Kinderkamack Road, Oradell, NJ 07649
Tel: 201.986.4749 • Fax: 201 225.5113
donald.distante@unitedwater.com



January 19, 2007

Rockland County Department of Health Environmental Health Division Dr. Robert L. Yeager Health Center 50 Sanatorium Rd Pomona, New York 10970

Subject: System well capacity modification

Dear Dr. Miller:

Attached please find a report from Leggette, Brashears & Graham, Inc. (LBG) regarding the capacity of United Water New York's (UWNY) system wells. The existing reported capacity of the system wells is 20.5 mgd, which was based on the actual production amounts attained during the 3-day historical peak demand days in 2001. For several reasons, as explained in detail in LBG's report, we are requesting that the peak capacity be modified from 20.5 mgd to 21.5 mgd (an increase of 1.0 mgd, sustainable for two 3-day periods). Note that LBG's report/table refers to the 2001 capacity of 20.6 mgd. This was the actual system well production capacity in 2001 but it was reported as 20.5 mgd just for simplicity. The primary reasons for these modifications are as follows:

- The achieved capacity in 2001 were without the improvements made at Nanuet 13/14 and Viola 28/106
- One well (Germonds 21) was not in-service in 2001 and has since been placed into service
- Some wells were shutoff for periods of time during the August 2001 peak event (Spring Valley 3, Tappan 20 and New Hempstead 24). In reviewing the data it was determined that these wells had ample submergence to allow them to operate. In speaking to the operators about this, the issue of excessive pressure has been raised as one potential reason for shutting them down. To address this, we applied the hydraulic model using the capacities indicated in LBG's letter with an overall system demand of 47.9 mgd (i.e., the 2007 projection). The results indicate that 21.5 mgd can be produced. The model includes the recent distribution system improvements (i.e., East-to-West transmission main and the Eckerson Rd. Booster pump) as well as Letchworth WTP addition which have significantly helped transmit the water produced to the needed areas within acceptable pressures.

This requested modification is an important element of UWNY's ability to meet the June 15, 2007 volume commitment of 1.5 mgd. As you are aware, two other projects that are part of our short-term supply strategy (Sparkill Air Stripper and Blaisdell Pump Station) are still under review by RCDOH. Therefore, to assist UWNY with meeting this important water supply target,

we would greatly appreciate a determination on the attached package at your earliest convenience.

We look forward to working with you on this project. LBG and UWNY would be pleased to meet with you to discuss this further, if necessary.

Yours Truly,



Donald Distante, PE Director, Master Planning

cc:

Michael Pointing, General Manager of UWNY John Dillon, Corporate Attorney UW Michael McDonald, P.E., Program Manager, B&V Alan Weland, P.E., Director of Engineering UWNY Gary Albertson, P.E. Vice President, UW

LEGGETTE, BRASHEARS & GRAHAM, INC.

PROFESSIONAL GROUND-WATER AND ENVIRONMENTAL ENGINEERING SERVICES

6 ARROW ROAD, SUITE 103 RAMSEY, NI 07446 201-818-0700 FAX 201-818-0505 www.lbgweb.com

January 18, 2007

Mr. Don Distante, P.E. United Water New York 700 Kinderkamack Road Oradell, NJ 07649

RE: Summary of Well Supply Contribution to 2001 System

Drought Yield

United Water New York Rockland County, New York

Dear Mr. Distante:

As per your request, Leggette, Brashears & Graham, Inc. (LBG) has completed a review of the United Water New York (UWNY) well-production data for 2001, relative to system capacity during droughts. The 2001 data were utilized since it corresponds to a historical peak demand during a period of very dry weather. The purpose of the review was to evaluate overall yield contribution of the wells to the UWNY system during peak demand periods (considered over the course of three successive days) under drought conditions such as those exhibited during 2001.

Based on the available SCADA-recorded information, the peak (3-day) UWNY system demand during the 2001 drought occurred on August 8, 9, and 10. A summary of the corresponding daily pumpage ("flow" rate) data, expressed in gallons per minute (gpm), is provided as Table 1. The "Adjusted Peak Capacity" column presented in Table 1, reflects the actual average rates for each well reported for the three day period in August 2001, rounded to the nearest 5 gpm. Totaling the average flow rate for each of the UWNY Wells that were in production during the August 2001 peak demand period, yields a well production total of 20.6 million gallons per day (mgd).

The well production total of 20.6 mgd represents well-pumpage corresponding to the maximum operational system demand at that time, and does not represent the maximum cumulative well yield that is currently available. Based on the currently existing pump, treatment and well conditions, and available aquifer yield, a greater amount of cumulative yield is available. The basis for this conclusion regarding available well capacity is presented by the following discussion regarding Spring Valley Well 3, Tappan 20,

and New Hempstead 24, which were reportedly not pumped either at all or consistently during the targeted August 2001 peak period at the respective potential peak capacity.

The submergence data for Spring Valley Well 3 and Tappan 20 indicate that the utilized pumping rates over the three-day period were at some point decreased to a flow rate below the respective potential capacities, though no adverse conditions assumed as a basis for this decision were reported (see respective graphs presented in Attachment I). As such, the Adjusted Peak Capacity values indicated in Table 1 for Spring Valley Well 3 and Tappan 20, are based on the respective peak flow rate use during the corresponding three day period (145 gpm and 115 gpm, respectively).

As for New Hempstead 24, a peak flow rate of 418 gpm was noted for the August 2001 period. However, New Hempstead 18 (which with New Hempstead 24, comprise the local well field) was being pumped during this same time at 880 gpm and exhibited minimal submergence (see respective graphs presented as Attachment I). In consideration of the interference effects that have historically existed between these two wells, and the reported conditions that prevailed during the August 2001 period, the New Hempstead Well Field was capable of a cumulative yield of 1,200 gpm, which could have also been accomplished by pumping each well at approximately 600 gpm. As such, 600 gpm has been indicated for each of these wells in the Adjusted Peak Capacity column of Table 1. Further support of this capacity is that these two wells produced an average of 1,434 gpm during the 3-day peak demand event that occurred from August 3 through 5, 2005. This was also a dry period, albeit of shorter duration than that of 2001.

In addition to the three wells discussed above, three other UWNY Wells that reportedly were not utilized at the corresponding peak capacity over the three-day period in August 2001, by virtue of not being in service, are: Nanuet 14; Germonds 21 and Viola 106. Nanuet 14 was not in service during this time due to treatment system work which precluded the ability for it to be pumped. Germonds 21 was not in service as it is a relatively low-capacity well and at the time not considered crucial to have available. Viola 106 was not in service in early August 2001, due to clear-well construction activities for air removal. This well was returned to service in late August 2001.

In an effort to project the adjusted peak capacity during a drought, the flow and submergence data for Nanuet 13 and Nanuet 14 (comprising the local well field) collected in 2005 indicate average pumping rates of 390 gpm and 242 gpm, respectively, during the peak demand period in August 2005 (Attachment I). During this time frame, Nanuet 14 was not pumped consistently at 242 gpm, but at daily average rates ranging from 110 gpm to 312 gpm. The available submergence data and recorded interference effects with Nanuet 13, indicate that Nanuet 14 was capable of being pumped consistently at a rate of 312 gpm during that peak demand period. Thus, the Adjusted Peak Capacities for Nanuet 13 and 14 presented by Table 1 reflect the 2005 data, and are 390 gpm and 310 gpm, respectively, which equates to a 3-day peak demand period well-field yield of 700 gpm.

Germonds 21 was not in service in 2001, and flow data is absent for the peak demand period of August 2005. However, submergence data for this time period indicates that the well was in service. Recent well test results indicate that Germonds 21 is capable of being pumped at 150 gpm during peak

demand periods, and conservatively meeting current operating demands at 90 gpm. As such, the conservative operating demand (90 gpm) for Germonds 21 was assumed as the respective Adjusted Peak Capacity value provided in Table 1.

Submergence data for 2001 and 2005 indicate that if Viola 106 had been in service during the three-day demand period in August 2001, the well would have been capable of being pumped at 538 gpm, which is the average demand rate utilized in 2005 (see respective graphs presented as Attachment I). Thus, based on the flow rate and submergence data recorded for 2005, and the interference effects which occur between Viola 28 and Viola 106 (both comprise the local well field), the Adjusted Conservative Peak capacity of the Viola Well Field is 990 gpm (i.e., based on the 2005 data, which indicate Viola 28 at 450 gpm, and Viola 106 at 540 gpm).

Based on the available UWNY SCADA flow and submergence data, and the above discussion, the total Adjusted Peak Capacity for the active wells comprising the UWNY system during drought conditions (as exhibited in 2001 and 2005) is 21.5 mgd. This total does not include additional well capacities that may be available in the event that pump size and setting limitations relative to residual available submergence are compensated for at selected UWNY Wells.

If you have any questions or need additional information, please call.

Very truly yours,

LEGGETTE, BRASHEARS & GRAHAM, INC.

Bill Prehoda, Po

Hydrogeologist/Sr. Associate

Frank Getchell, PG

Hydrogeologist/Vice President

BP:pw Attachments

Distribution: M. McDonald; B&V

TABLE

TABLE 1
United Water New York
Rockland County, New York

System Wells Peak Flow 2001

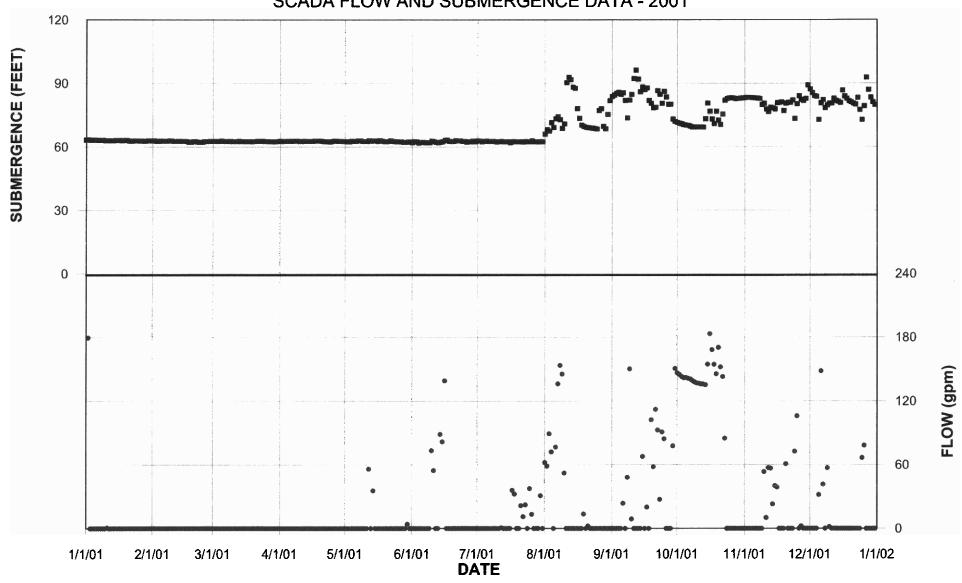
	Promure					Adjusted Peak
Well	District	8/8/2001	8/9/2001	8/10/2001	Average	Capacity ⁽¹⁾
Spring Valley 1A	10	550		ported in gallo		
Spring Valley 3	10 10	559 145	545 52	546 0	550 66	550 145
Spring Valley 4	10	107	272	245	208	210
Spring Valley 6	10	399	390	391	393	390
Spring Valley 17	10	408	400	404	404	400
SVWF Total					1621	1695
Nanuet 13	10	476	476	425	459	390
Nanuet 14	10	0	0	O		310
Nanuet Total					459	700
Blauvelt 15	10	333	332	331	332	330
Tappan 16	10	211	193	189	198	200
Tappan 20	10	53	115	101	90	115
Tappan Total					287	315
Bardonia 19	10	152	128	155	145	145
Germonds 21	10	0	0	0	24	90
Pearl River 22 New City 23	10 10	93 270	94 231	89 242	92 248	90 360
Wesel Road 32	10	178	179	171	176	250 175
Norge 64	10	363	365	319	349	350
Elmwood 66	10	.241	241	240	241	240
Lake Shore 73	10	408	353	345	369	370
West Gate 79 Grotke 83	10 10	126 149	126 147	125 147	126 148	125
		149	147	1407		150
Total PD 10: Total PD 10 (mgd)	20				4591 6.6	5025
						7.3
Saddle River 53 Pascack 65	95	437	436	413	429	430
Pinebrook 69	95 95	430 447	422 449	427 425	426 440	425 440
Birchwood 70	95	165	165	163	164	165
Eckerson 71	95	165	169	155	163	165
Eckerson 82	95	254	251	248	251	250
Total PD 95	6				1874	2875
Total PD 95 (mgd)					2.7	2.7
New Hempstead 18	20	953	775	919	882	600
New Hempstead 24	20	0	418	24	147	600
New Hempstead Total					1030	1200
Tallman 26	20	439	440	436	438	440
Ramapo 27	20	925	928	916	923	925
Ramapo29A	20	716	712	709	712	710
Ramapo Total					1635	1635
Viola 28	20	825	957	1213	998	450
Viola 106	20	0	0	0	0	540
Viola Total					998	990
Monsey 30	20	230	231	229	230	230
Monsey 31A Monsey Total	20	200	201	199	200	200
					430	430
Pomona 37 Pomona 38	20	181	179	149	170	170
Pomona 38 Pomona Total	20	292	293	291	292 462	290 460
Catamount 42A	20	100	174	170		
Catamount 42A Catamount 54A	20 20	173 218	174 218	172 216	173 217	175
Catamount Total	20	218	210	210	390	215 390
	. 20	410	411	400		
Nottingham 55 Willow Tree 56	20 20	410 661	411 655	408 650	410 655	410 655
Grandview 67 Grandview 78	20 20	194 194	194	197	195	195
Grandview Total	20	194	193	160	182 378	180 375
Cherry Lane 68	20	220	221	200		
Rustic 72	20 20	330 448	331 449	328 447	330 448	330 450
Academ 12	20	7-10	777	44)	740	430
Total PD 20	19				7604	7765
Total PD 20 (mgd)					10.9	11.2
Garnerville 46	40	0	0	0		
Thiells 50	40	37	38	38	38	40
	40	219	219	219	219	220
Thiells 51						
					257	260
Thiells 51 Total PD 40 Total PD 40 (mgd)					257 0.4	260 0.4
Total PD 40						

Note: (1) Values indicated for Spring Valley Well 3, Tappan 20, New Hempstead 18 and 24, Nanuet 13 and 14, Germonds 21, and Viola 28 and 106, are reflective of 2005 peak demand period data as discussed in the January 18, 2007 letter.

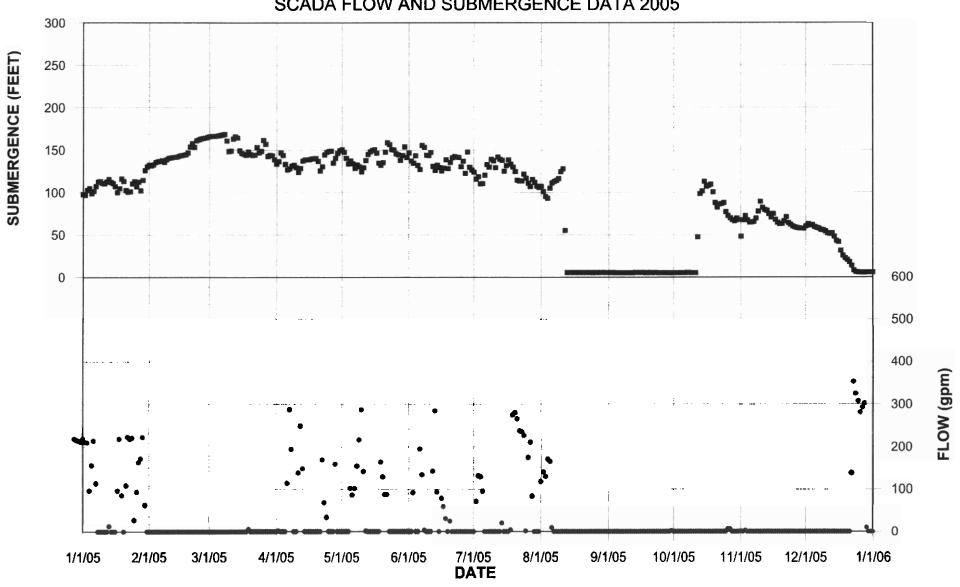
LEGGETTE, BRASHEARS GRAHAM, INC

ATTACHMENT

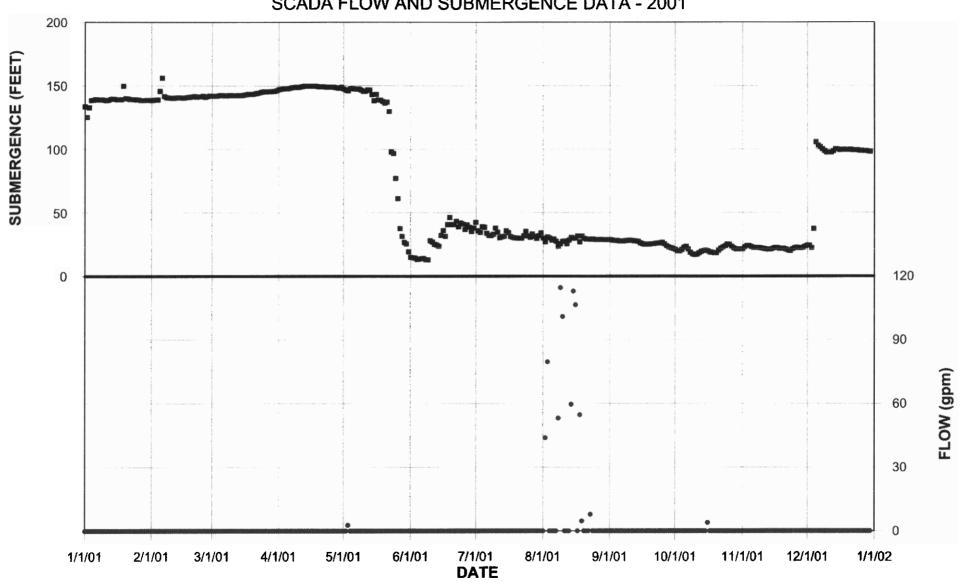
SPRING VALLEY WELL 3 SCADA FLOW AND SUBMERGENCE DATA - 2001



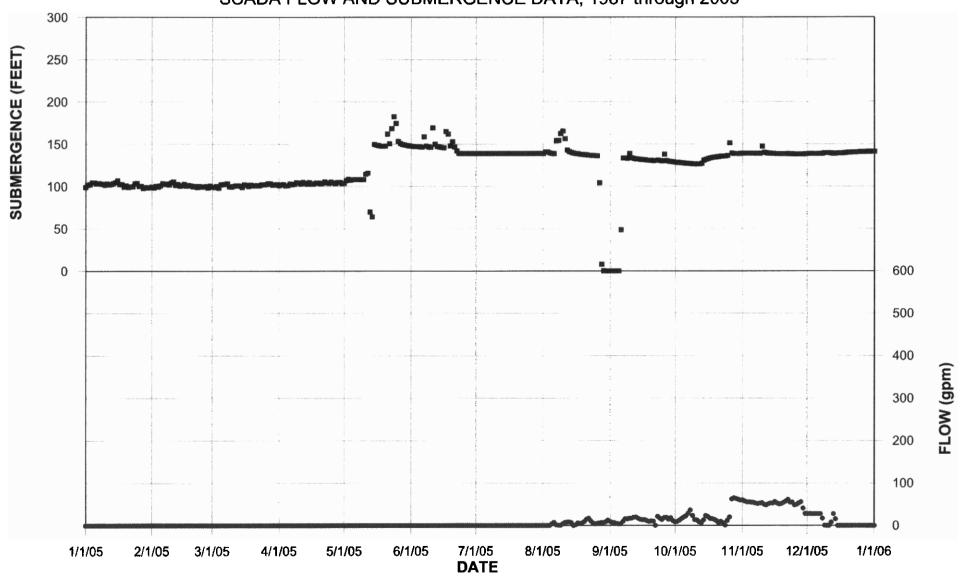
SPRING VALLEY WELL 3 SCADA FLOW AND SUBMERGENCE DATA 2005



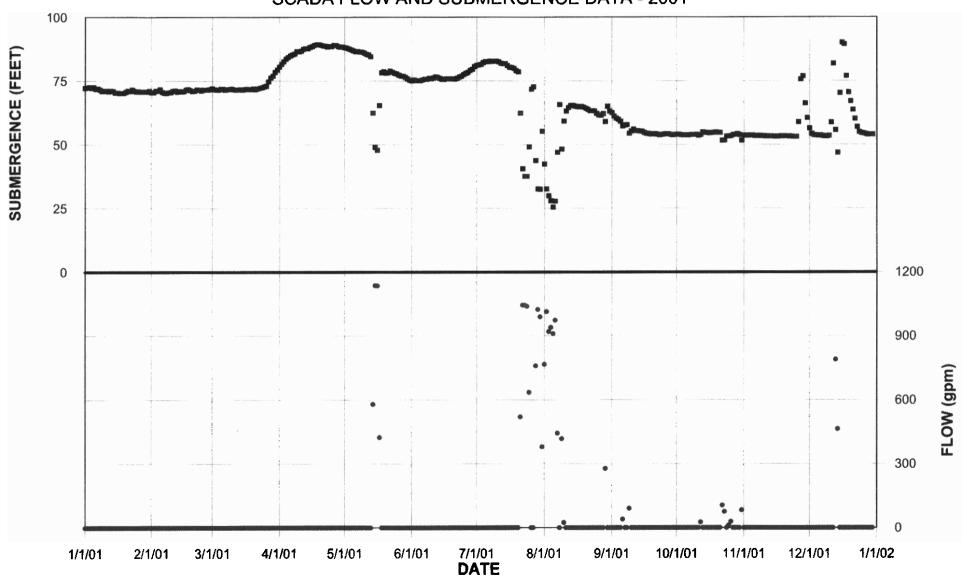
TAPPAN WELL 20 SCADA FLOW AND SUBMERGENCE DATA - 2001



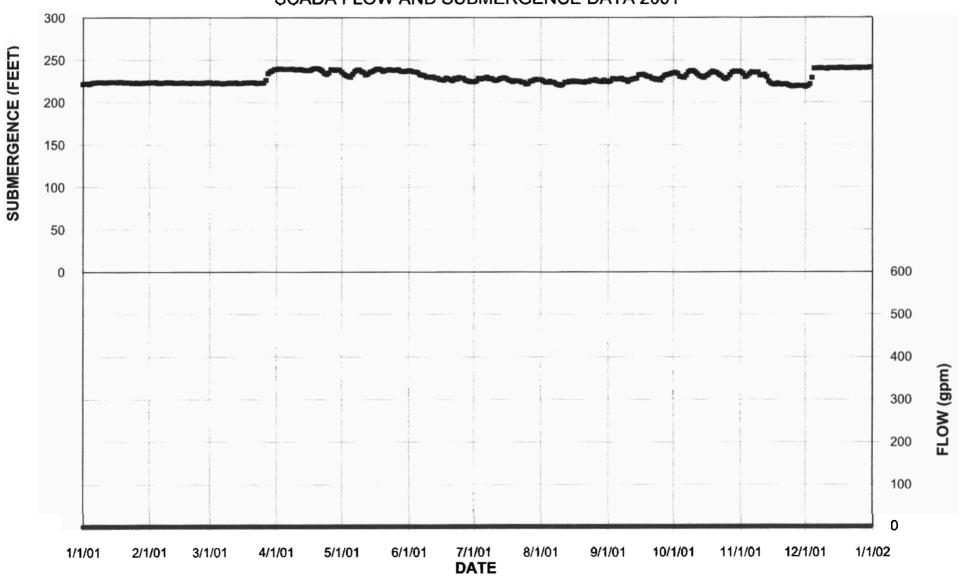
TAPPAN WELL 20 SCADA FLOW AND SUBMERGENCE DATA, 1987 through 2005



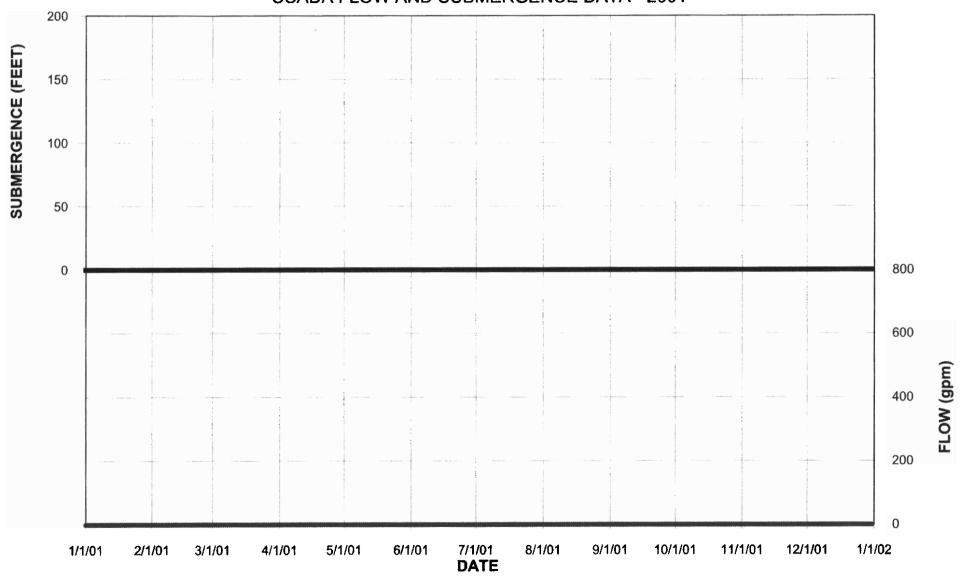
NEW HEMPSTEAD WELL 24 SCADA FLOW AND SUBMERGENCE DATA - 2001



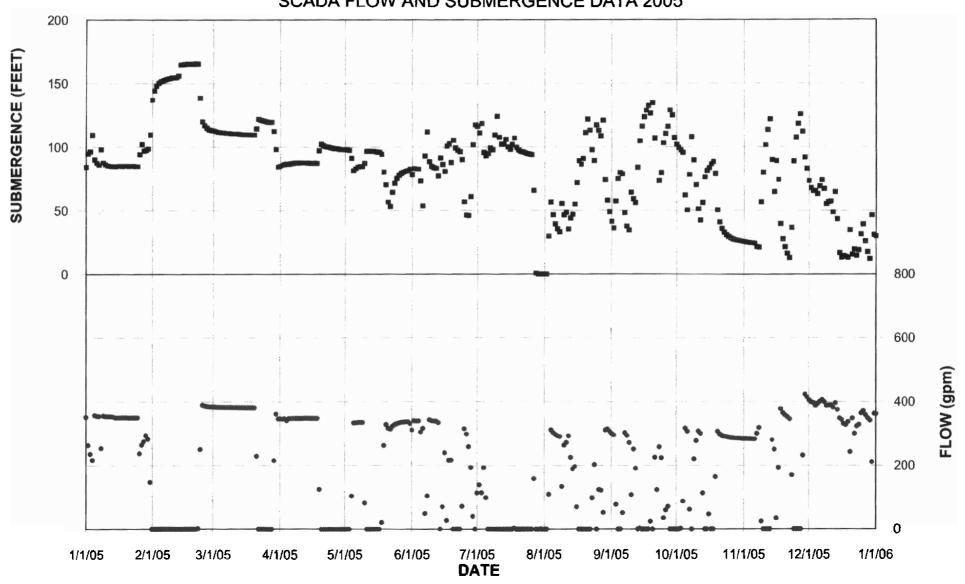
GERMONDS WELL 21 SCADA FLOW AND SUBMERGENCE DATA 2001



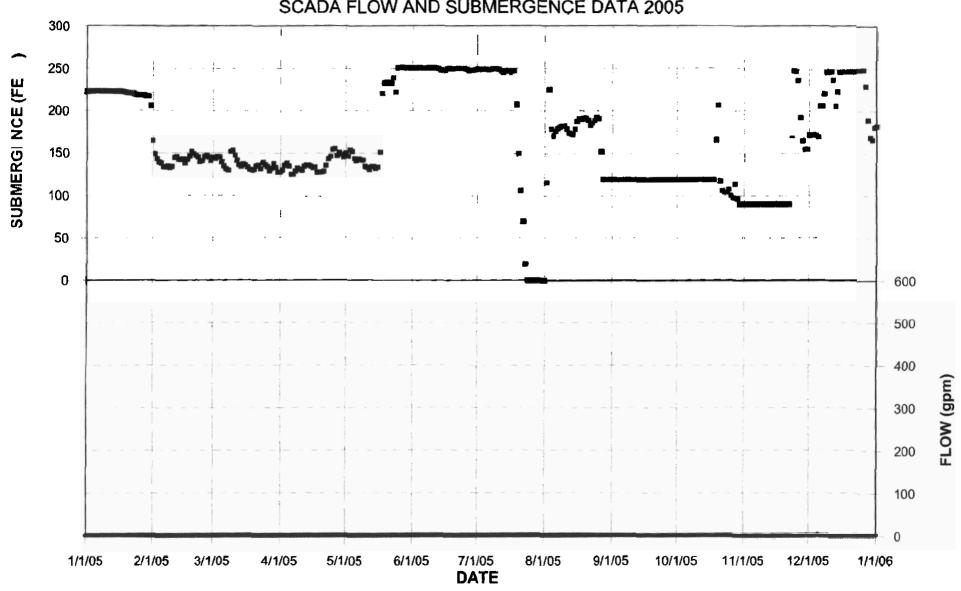
NANUET WELL 14 SCADA FLOW AND SUBMERGENCE DATA - 2001



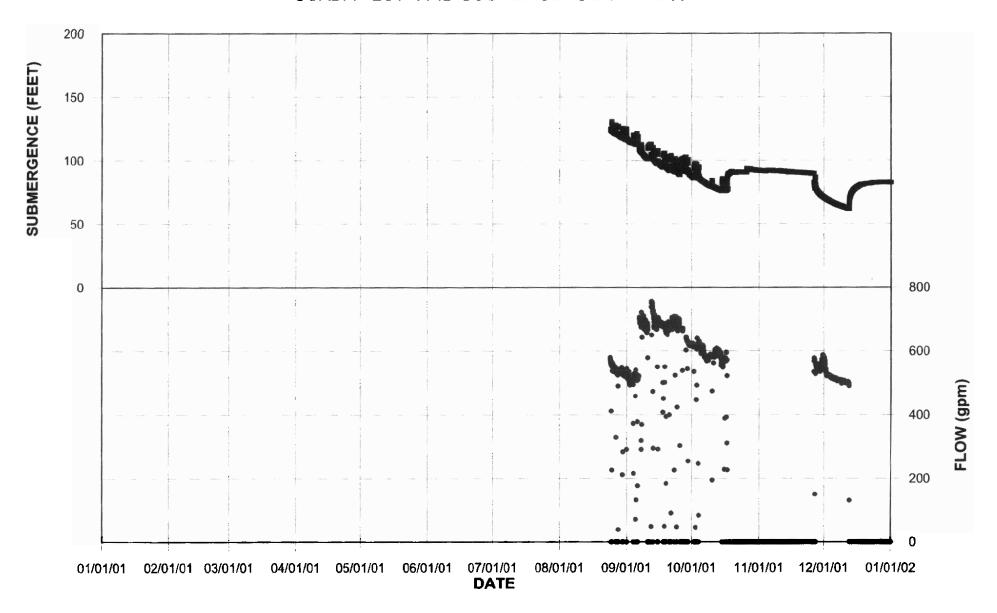
NANUET WELL 14 SCADA FLOW AND SUBMERGENCE DATA 2005



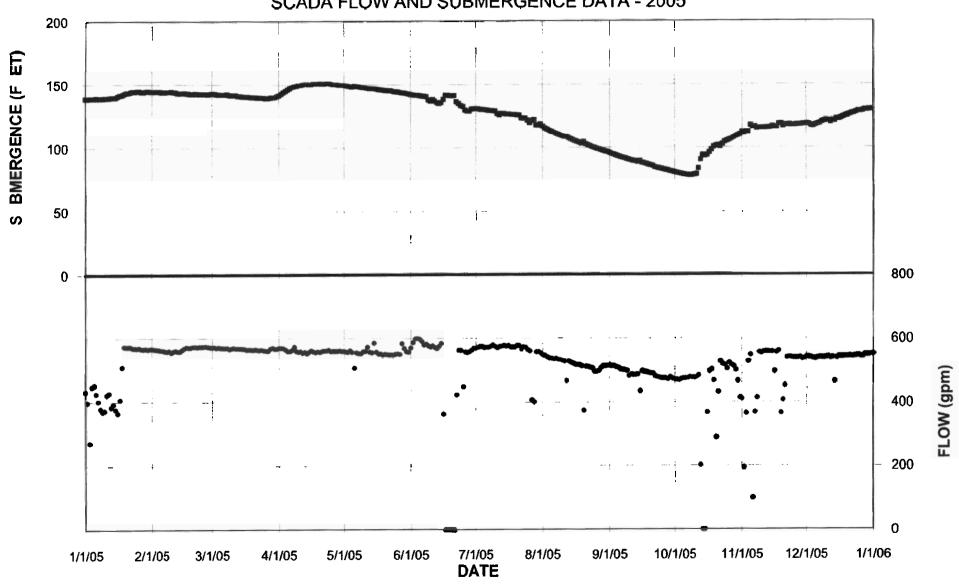




VIOLA WELL 106 SCADA FLOW AND SUBMERGENCE DATA 2001







APPENDIX E

TEST WELLS

SCHEDULE AND MILESTONES

July 2006

 A Potential Well Development and Conceptual Cost Estimates Report was prepared evaluating eight (8) potential well sites for development into production wells.

December 2006

Meeting between United Water New York (UWNY), Rockland County
Department of Health (RC-DOH) to discuss the conversion of test wells to
production wells. At this meeting, properties that meet required standards to be
constructed were discussed and a list of possible sites was developed.

January 2007

• For the potential sites, a technical package was submitted to the RCDOH for the conversion of test wells to production wells.

February 2007

- Discussed requirements for submittal of application packages with each town that the potential production wells are located.
- Discussed electrical requirements with Orange and Rockland (O&R) Utilities,
 Inc. for each site.
- PB Americas, Inc. visited all three (3) potential sites to identify wetlands, streambanks, and tree inventory for each site.

March 2007

- Conference call with New York State Department of Environmental Conservation (NYSDEC), New York State Department of Health (NYSDOH), and RCDOH to discuss the permit requirements for the construction of the test wells.
- Electrical Application for Service was submitted to O&R Utilities, Inc.
- Partial Site Plans for Strawtown Road and Long Clove Well were submitted to the Rockland County Drainage Agency (RCDA) for Jurisdictional Determination (JD).
- Partial Site Plans for Strawtown Road and Long Clove Road are submitted for Jurisdictional Determination (JD) by the NYSDEC for the construction of the test wells.
- On March 19, 2007 a meeting with the Rockland County Legislature and public
 was held to discuss the two (2) projects being considered for development. A
 presentation for information purposes only was given by UWNY and questions
 regarding the work and potential impacts to the surrounding environment are
 discussed and will be monitored during the pump test.

• A licensed surveyor began preparing a survey for the three (3) sites.

April 2007

- The licensed surveyor retained for the work completed the survey for all three (3) sites.
- In a letter dated April 9, 2007, the RCDA confirmed that they do not have jurisdiction for the sites.

May 2007

- In a letter dated May 4, 2007, the RCDOC issued the test well drilling permit for Strawtown Road, Long Clove Well, and Lime Kiln Road Wells. A copy of the approved permit follows.
- In an email dated May 2, 2007, the NYSDEC confirmed that no other permits are required for the construction of the test wells.
- On May14, 2007, letters are submitted to private well owners within a one-thousand five hundred (1,500') feet radius of the proposed wells. The letters are sent notifying all private well owners of the intention to construct new wells at the site and to offer monitoring of their wells during the pump test.
- On May 25, 2007, the Technical Advisory Committee (TAC) application was submitted to the Town of Clarkstown.
- Layne Christensen begins drilling the test wells at the Strawtown Road and Long Clove Road sites. Strawtown was air tested and the estimated capacity is between three hundred to four hundred (300 to 400) gpm. The air test at Long Clove Road results in an estimated capacity of sixty (60) gpm. The estimated capacity at Long Clove Road was approximately one hundred and fifty (150) gpm. A new test well location on the property is being considered that is closer to the existing test wells. However, this will require easement(s) from adjacent property owners. A new RC-DOH permit will be required for the new test well location.

June 2007

- A TAC hearing was held on June 13th for the Strawtown and Long Clove Test Wells with the Town of Clarkstown.
- A second Environmental Meeting of the Rockland County Legislature is currently scheduled for June 18, 2007.
- It is anticipated that the pump capacity test and monitoring of adjacent private wells will occur at the end of this month. In addition, a full water quality analysis shall be performed.
- Due to the reduced capacity encountered at the proposed Long Clove Road test well site, submit new test well location for approval for a new test well at the Long Clove Road site. Pending approval of the new location, drill new test well and air test shall be performed in June 2007.

July/August 2007

- Pending approval at the TAC hearing, it is anticipated that the design of the well house will be completed and an application for Preliminary and Final Planning Board hearings will be submitted to the Town of Clarkstown. It is anticipated that this will be scheduled to be heard in July and August 2007.
- Final applications for the NYSDOH and NYSDEC shall be applied for.
- The two (2) well projects shall be bid with pre-qualified contractors to construct the well and well house.

September/October 2007

- Pending approval at the Preliminary and Final Planning Board hearings,
 Construction Applications for the Town of Clarkstown shall be applied for.
- Pending approval of the Building, NYSDEC, and NYSDOH applications, the successful Contractor shall be given Notice to Proceed to begin construction of the production wells and above ground well houses.

November/December 2007

• Construction of the production wells shall continue and be completed by December 15, 2007.

PERMITS AND CORRESPONDENCE

The following information is included for reference and follows:

Strawtown Road Well

- 1. May 25, 2007 letter prepared by Buck, Seifert & Jost, Inc. (BS&J) addressed to the Town of Clarkstown Technical Advisory Committee (TAC), 3 pages
- 2. Strawtown PW-1 Geologic Log prepared by Leggette, Brashears & Graham, Inc. (LBG) dated May 18, 2007, 2 pages
- 3. Rockland County Department of Health Permit to Construct a Water Supply Well, Permit #07-004, dated May 4, 2007, 1 page
- 4. April 9, 2007 letter prepared by the County of Rockland Drainage Agency regarding Jurisdictional Determination (JD) for Proposed Production Well at Strawtown Site, 1 page
- 5. Construction of the Strawtown Test Well No. 2 Site Plan, Well Diagram, and Notes drawing prepared by BS&J, 1 page 11" x 17".
- 6. Construction of the Strawtown Well Cost Estimate dated March 14, 2007, 1 page.

Long Clove Well

- 1. Long Clove TW-4 Geologic Log prepared by Leggette, Brashears & Graham, Inc. (LBG) dated May 29, 2007, 2 pages
- 2. May 25, 2007 letter prepared by Buck, Seifert & Jost, Inc. (BS&J) addressed to the Town of Clarkstown Technical Advisory Committee (TAC), 3 pages
- 3. Rockland County Department of Health Permit to Construct a Water Supply Well, Permit #07-004, dated May 4, 2007, 1 page
- 4. April 9, 2007 letter prepared by the County of Rockland Drainage Agency regarding Jurisdictional Determination (JD) for Proposed Production Well at Strawtown Site, 1 page
- 5. Construction of the Long Clove Test Well No. 3 Site Plan, Well Diagram, and Notes drawing prepared by BS&J, 1 page 11" x 17".
- 6. Construction of the Long Clove Well Cost Estimate dated March 14, 2007, 1 page.



APPENDIX F
SPARKILL

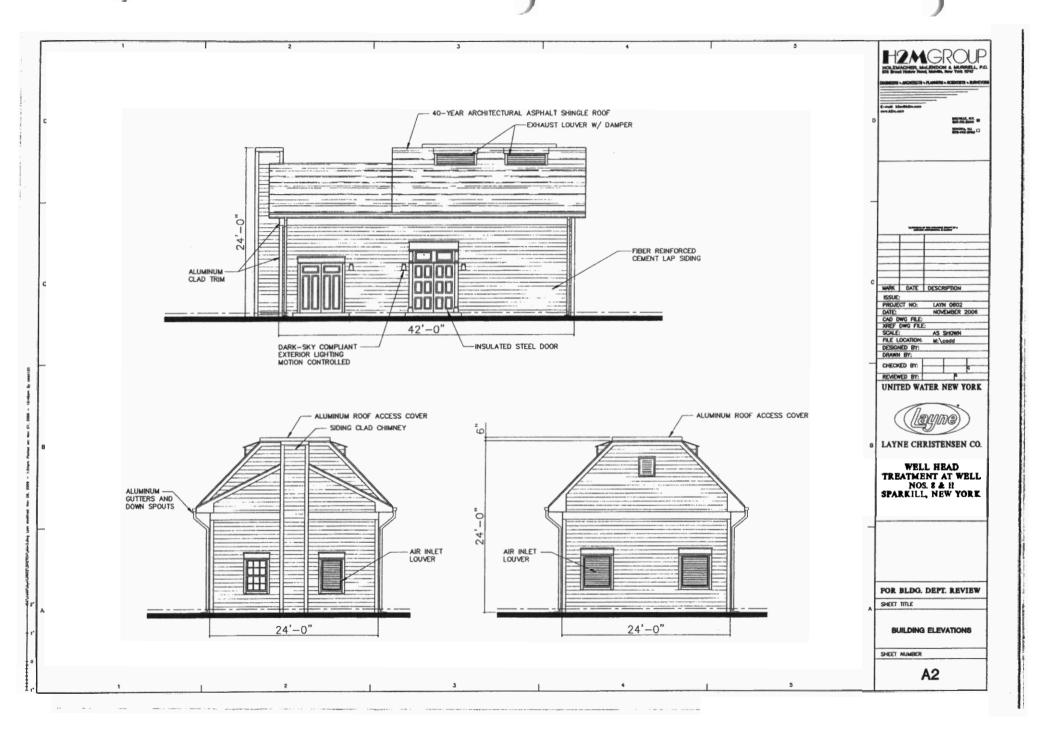
PERMITS AND CORRESPONDENCE

PERMITS AND CORRESPONDENCE

The following information is included for reference and follows:

- 1. UWNY receives final approval for the project on June 6, 2007 when the Zoning Board of Appeals (ZBA) for the Town of Orangetown denies an appeal of the Town Zoning Director's prior determination that the project does not require a use variance.
- 2. April 24, 2007 memorandum prepared by John Giardiello, P.E., Director of the Town of Orangetown Office of Building, Zoning, and Planning Administration and Enforcement addressed to the Zoning Board of Appeals, 2 pages.
- 3. Town of Orangetown Building Permit No. 36910 dated March 9, 2007 for the public water supply treatment system and building located at 59 Flitt Street, Tappan, NY, 2 pages.
- 4. November 1, 2006 memorandum prepared by John Giardiello, P.E., Director of the Town of Orangetown Office of Building, Zoning, and Planning Administration and Enforcement addressed to Cheryl Coopersmith, Chief Clerk and Debbie Arbolino, Administrative Aide, 1 page.
- 5. November 1, 2006 letter prepared by Rockland County Department of Health (RCDOH) regarding air emissions guidelines for the proposed UWNY Well Head Treatment at Well Nos. 8 and 11, Sparkill, NY, 1 page.
- 6. September 5, 2006 Denial of building permit application and referral to the ACABOR and ZBA for the proposed UWNY Well Head Treatment at Well Nos. 8 and 11, Sparkill, NY prepared by John Giardiello, P.E., Director of the Town of Orangetown Office of Building, Zoning, and Planning Administration and Enforcement, 2 pages.
- 7. Town of Orangetown Building Permit No. 36502 dated August 21, 2006 for the replacement of electrical service to the existing well house located at 59 Flitt Street, Tappan, NY, 2 pages.

SITE AND ARCHITECTURAL PLANS



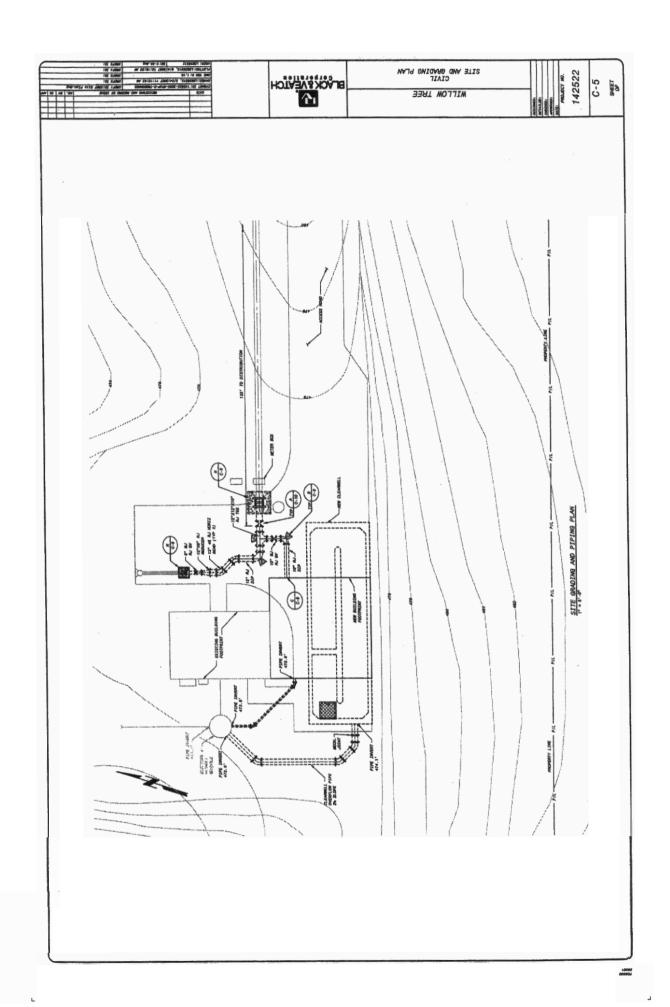
APPENDIX G BEDROCK WELL DE-AERATION

WILLOW TREE 56

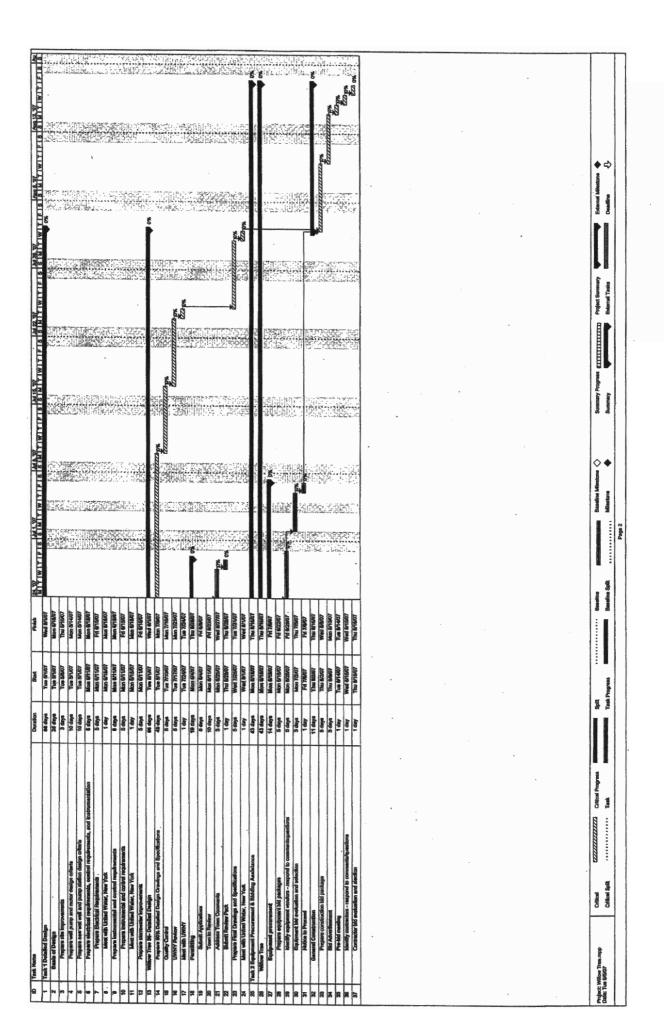
Willow Tree 56

Willow Tree 56 is permitted by NYSDEC for 1,000 gpm and taps the underlying bedrock aquifer. Recent production capacity indicates Willow Tree 56 has been utilized at rates ranging between 400 gpm and 800 gpm. Willow Tree 56 was operating at an average of 655 gpm with an excess of 50 feet of submergence above the pump intake during the August 2001 3-day system peak demand period. Current pumping test data results indicate the specific capacity of Willow Tree 56 is similar to the results of the original testing, with no significant production-limiting decline noted.

Willow Tree 56 has exhibited air in the discharge, especially at higher pumping rates, limiting production capacity. UWNY proposes to install a de-aeration basin to eliminate air entrainment into the distribution system and allow for higher discharge capacity. Based on the results of original testing, operational history, and results of current testing, Willow Tree 56 is capable of sustaining the short-term (3-day) peaking at a rate of 1,000 gpm during a drought demand period similar to August 2001. This 3-day peak pumping rate represents an increase of 345 gpm over the 2001 peak system demand operating rate.



AND THE STATE OF THE STATE
The state of the s
The state of the s
THE PARTY OF THE P
The Street of th
The stratum is a second to the strategy of the
TOTAL STATE OF THE PROPERTY OF
We word The word
The state of the s
Will bear west and the second of the second
Will Proper To Comment of the Commen
With Respond to the Water State of the Water State
New Property Will State of the Control of the Contr
The state of the s
Land France The Foundation of the Control of the Co
Land Transport of August Control of August Contr
Note the property of the prope
The filters is a distinct in the distinct in t
The state of the s
The street of th
The Wide Water Wat
The Bridge of The Bride of The Bridge of The Bridge of The Bridge of The Bridge of The
The state of the s
The bridges
Thus likely of
V
ay Ederica Michiga
Test Propres Dearline Spill Ministry American Survey



NEW HEMPSTEAD 18 AND 24

New Hempstead 18 and 24

The New Hempstead Well Field is permitted by NYSDEC at a combined capacity of 1,700 gpm. Both New Hempstead 18 and 24 tap the underlying bedrock aquifer. Recent production capacity indicates that UWNY utilizes the wells in the New Hempstead Well Field primarily on an alternating basis and not 24 hours per day. Recent New Hempstead 18 production rates range between 800 gpm and 1,000 gpm (while New Hempstead 24 is off), and recent New Hempstead 24 production rates vary between 900 and 1,200 gpm (while New Hempstead 18 is off).

The New Hempstead Well Field is typically not operated at its maximum permitted production capacity of 1,700 gpm, in part due to air entrainment at higher production rates. UWNY proposes to install a deaeration basin to eliminate air entrainment into the distribution system. The New Hempstead combined operating discharge rate was 1,030 gpm (New Hempstead 18 – 882 gpm, New Hempstead 24 – 147 gpm) during the August 2001 3-day system peak demand period. The low discharge rate of New Hempstead 24 reflects primarily only one day of usage during this period. Current pumping test data results indicate the specific capacity of both New Hempstead 18 and 24 are similar to historic specific capacity data, with no significant production-limiting decline noted at the well field.

Based on the results of original testing, mutual interference, operational history, and results of current testing, the New Hempstead Well Field is capable of sustaining the short-term (3-day) peaking at a rate of 1,200 gpm during a drought demand period similar to August 2001. This well field peaking discharge can be obtained by operating Wells 18 and 24 at 600 gpm each, or potentially operating Well 24 at a higher rate and reducing the rate of Well 18 by an equivalent amount. This 3-day peak pumping rate represents an increase of 170 gpm over the 2001 peak system demand operating rate.

PASCACK 65

Pascack 65

Pascack 65 is permitted by NYSDEC for 700 gpm and taps the underlying bedrock aquifer. Recent production capacity indicates Pascack 65 has been utilized at rates ranging between 350 gpm and 500 gpm. Pascack 65 was operating at an average of 426 gpm with an excess of 50 feet of submergence above the pump intake during the August 2001 3-day system peak demand period. Current pumping test data results indicate the specific capacity of Pascack 65 is similar to the results of the original testing, with no significant production-limiting decline noted.

Pascack 65 has historically exhibited air in the discharge, limiting production capacity especially at higher pumping rates. UWNY proposes to install a deaeration basin at Pascack 65 to eliminate air entrainment into the distribution system and allow for higher discharge capacity. Based on the results of original testing, operational history, and results of current testing, Pascack 65 is capable of sustaining the short-term (3-day) peaking at a rate of 600 gpm during a drought demand period similar to August 2001. This 3-day peak pumping rate represents an increase of 274 gpm over the 2001 peak system demand operating rate.

APPENDIX H VIOLA AND ELMWOOD INFRASTRUCTURE IMPROVEMENTS

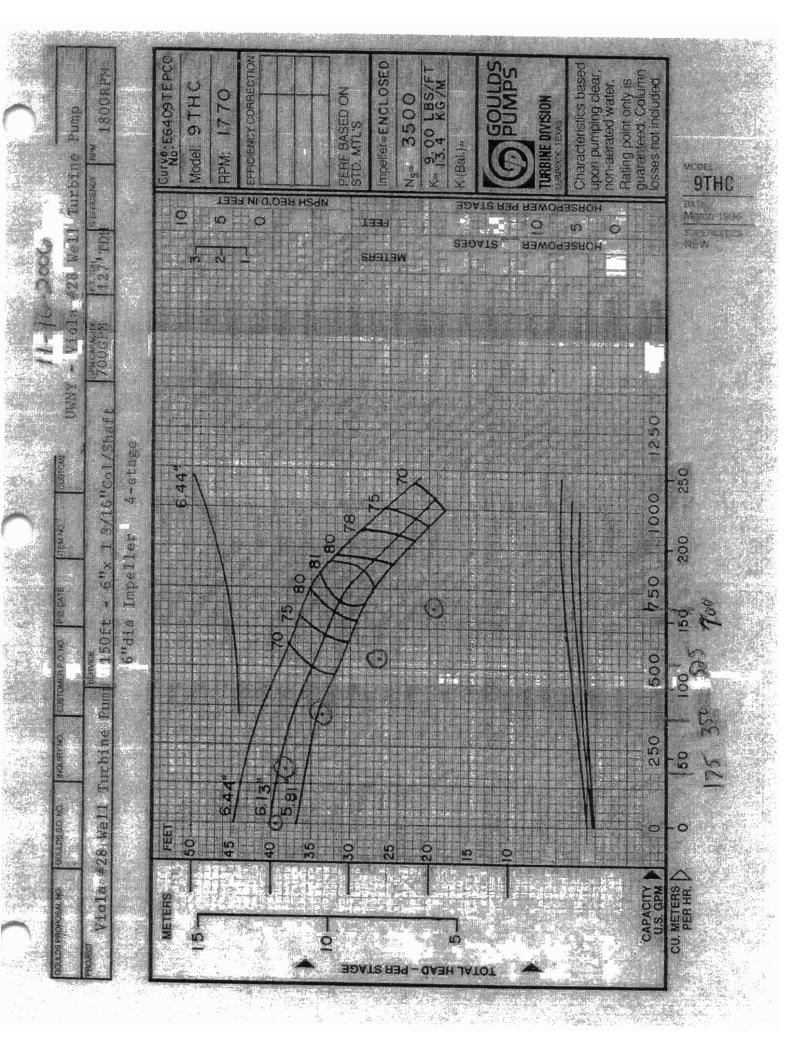
Viola 28 and 106

The Viola Well Field is permitted by NYSDEC at a combined capacity of 1,400 gpm. Both Viola 28 and 106 tap the underlying bedrock aquifer. Recent production capacity indicates that UWNY utilizes the wells in the Viola Well Field primarily on an individual basis with the recent production rates range between 500 gpm and 700 gpm total. The Viola Well Field is typically not operated at its maximum permitted production capacity of 1,400 gpm due to air entrainment at higher production rates. UWNY installed a deaeration basin in 2001 to eliminate air entrainment into the distribution system.

The Viola 28 average discharge rate was 998 gpm during the August 2001 3-day system peak demand period, without Viola 106 operating due to the dearation basin not being put online until the end of August 2001. Current pumping test data results indicate the specific capacity of both Viola 28 and 106 are similar to historic specific capacity data, with no significant production-limiting decline noted at the well field. Based on the results of original testing, mutual interference, operational history, and results of current testing, the Viola Well Field is capable of sustaining the short-term (3-day) peaking at a rate of 1,400 gpm during a drought demand period similar to August 2001. This well field peaking discharge can be obtained by operating Wells 28 and 106 at 700 gpm each. This 3-day peak pumping rate represents an increase of 402 gpm over the 2001 peak system demand operating rate.

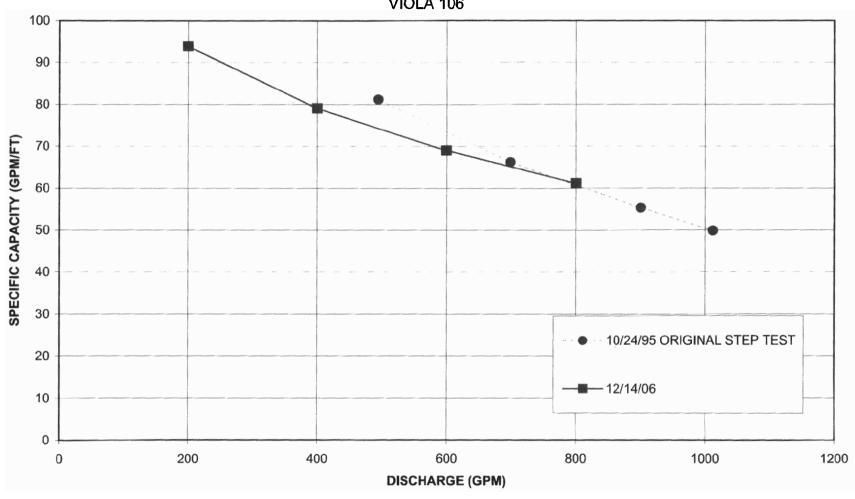
. 09

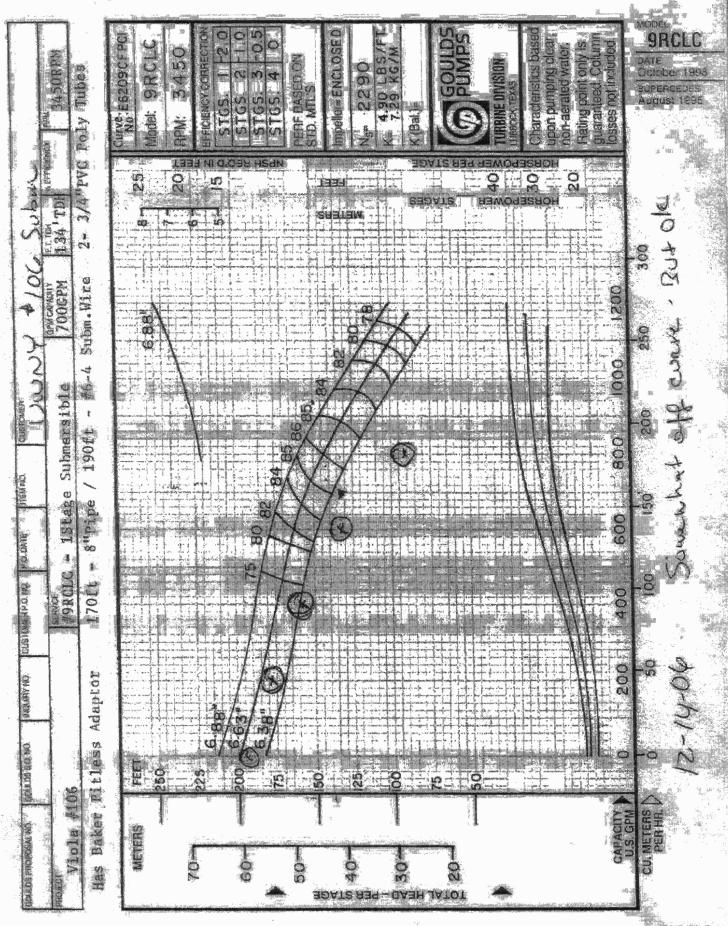
SPECIFIC CAPACITY (GPM/FT)



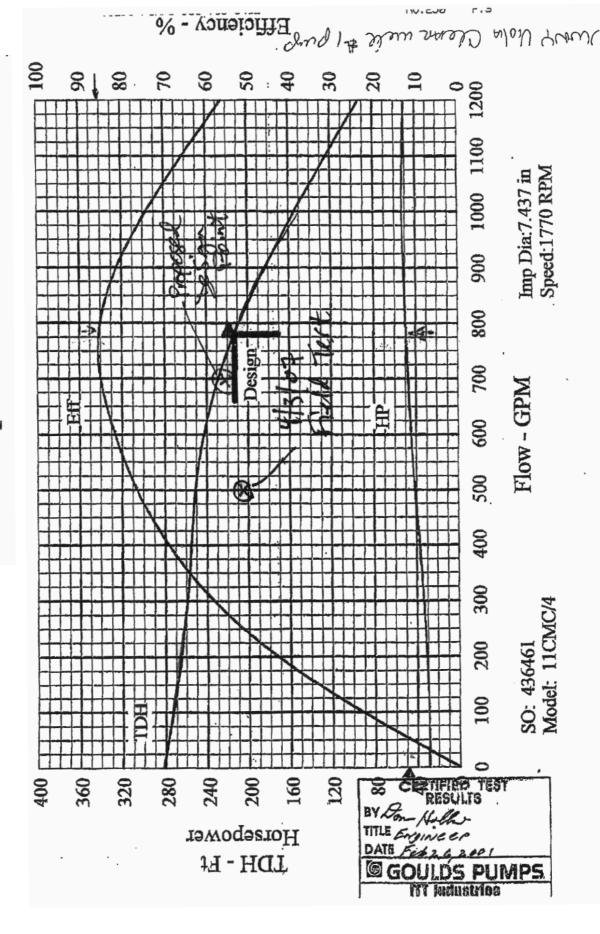
UNITED WATER NEW YORK ROCKLAND COUNTY, NEW YORK

HISTORIC SPECIFIC CAPACITY VIOLA 106





ITT - Goulds Pumps Turbine Operations

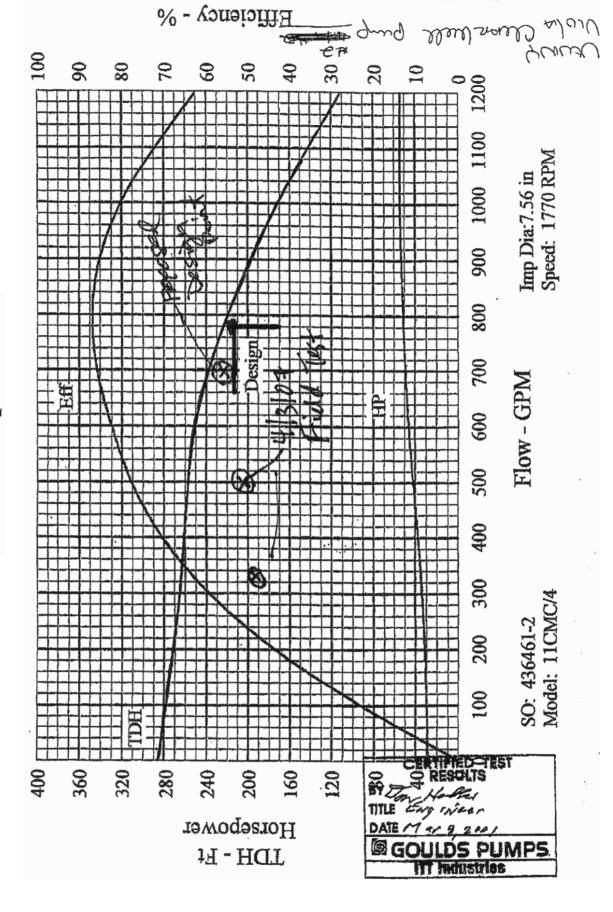


AF007 Rev 0 8/1999



ITT - Goulds Pumps Turbine Operations

7007. 3cv 0 3/1999





Hesd: 240 A Tupellens

Temperature: 60 °F Vapor pressure: 0.2563 psi a Atm pressure: 14.7 psi a

IU: KINBKAND

Name:

Date: 04/05/07

1-11-07 mun:

Size: 11CHC (4 stages)

Type: Lineshaft Synch speed: 1800 rpm

Speed: 1770 rpm Dia: 7.5625 in

Curve: E3141-2

Specific Speeds:

Ns: 2130

Pump Notes for Standard Sizes: Suction Size 8" Discharge Sizes 6" 8"

Vertical Turbine:

Bowl size: 11 in Mex lateral: 0.75 in Thrust K factor: 7 lb/ft

Search Criteria: Flow: 780 US gpm

Fluid: Water

SG: 1 Viscosity: 1.106 cP

NPSHe: -- ft

Motor:

Standard: NEMA

Size: 75 hp Speed: 1800

Sizing criterie: Max Power on Design Curve

Some conve

Pump Limits for Standard Construction: Pressure: 380 psi g

Temperature: 120 °F Sphere size: 0.68 in

> Data Point ---Flow: 780 US gons Heed: 242 ft Eff: 86%

Power, 55.3 hz NPSHr: 8,74 ft

- Design Curve -Shutoff Head: 281 ft Shutoff dP: 122 pel

Min Flow: -- US gpm

BEP: 86% eff

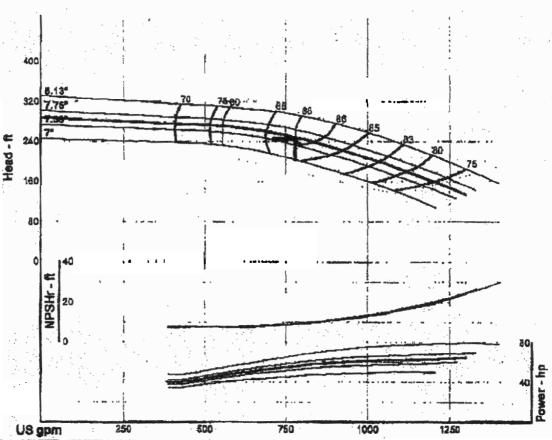
@ 806 US dbm

NOL PWT: 64 hp

@ 1299 US apm

Max Pwr. 78.6 ho

-- Max Curve --1300 US gpm



938	1770	215	84.6	59.7	11.7
780	1770	242	86	55.3	8.74
624	1770	259	82.8	49.2	7.39
468	1770	268	74,5	42.2	7
312	1770			_	

RINBRAND WELL DRILLING CO., INC. Glen Rock, NJ 074572-2831

Selected from catalog: Goulds Linesheft 80HZ Vers: 3.06

780 spu stays TDH
is Fumps 2004d GOHP needed Selected from
VIU/A 28 pumpd1+2 Booster VIO/A

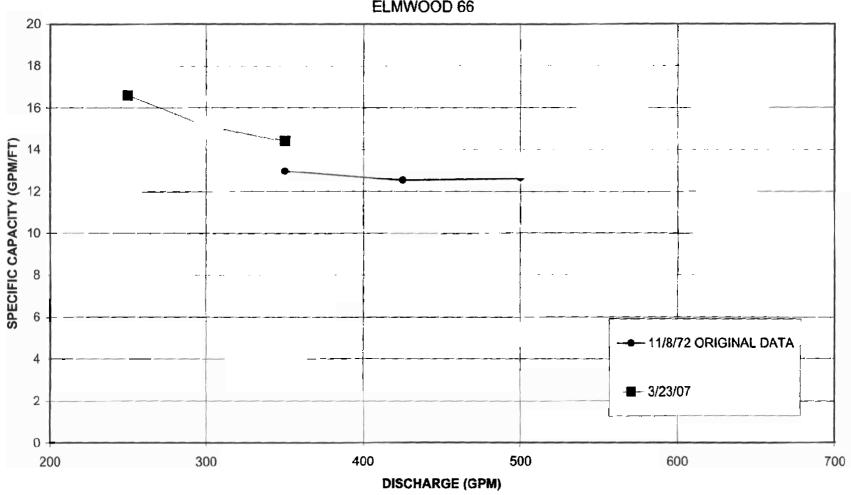
Elmwood 66

Elmwood 66 is permitted by NYSDEC at a capacity of 350 gpm and taps the underlying bedrock aquifer. Recent production capacity indicates that UWNY utilizes Elmwood 66 at a production rate up to 300 gpm, but not typically operated at its maximum permitted production capacity of 350 gpm. Elmwood 66 has a Lowery treatment system installed primarily for treatment of VOCs.

The Elmwood 66 average discharge rate was 241 gpm during the August 2001 3-day system peak demand period. Current pumping test data results indicate the specific capacity of Elmwood 66 is similar to historic specific capacity data, with no significant production-limiting decline noted at the well field. Based on the results of original testing, operational history, and results of current testing, Elmwood 66 is capable of sustaining the short-term (3-day) peaking at a rate of 350 during a drought demand period similar to August 2001. This 3-day peak pumping rate represents an increase of 109 gpm over the 2001 peak system demand operating rate.

UNITED WATER NEW YORK ROCKLAND COUNTY, NEW YORK

HISTORIC SPECIFIC CAPACITY ELMWOOD 66



MODEL EFFICIENCY CORRECTION Characteristics based 8RJHO **8RJHO** guarantaed. Column losses not included. upon pumping clear, non-aerated water. 3105 Impeller= A8155 Rating point only is 1.760RPM 1760 PERF BASED ON STD. MTL'S DATE 2825 May 2, 1988 SUPERCEDES Curve No. July 22,1977 5.7 5.3 K (Bal.) Model: RPM 75HP NPSH REC'D IN F 0 3 UINTED WATER NEW YORK STAGES HOUSEPOWER 62 TDH 3-23-07 6"Tail Pipe (5"Impeller 350GPM í 10ft CUSTOME જ 400 Shaft 3stage ITEM NO. بح 300 6"x1"Column 1 PO DATE SPRINCE #8RJHO CUSTOMER P.O. NO. 200 1 85ft 00 99# INCLIMITY NO. PUMP GOLALDS S.O. NO. ELYWOOD WELL FEET 3-39-07 CU. METERS CAPACITY U.S. GPM GOULDS PROPOSAL NO. METERS T 8 0 TOTAL HEAD - PER STAGE

(Dochle Eran Impellens MODEL EFFICIENCY CORRECTION Characteristics based 8RJH0 8RJHO guaranteed. Column losses not included. Curve No. 3105 Impeller= A8155 LBS/FT KG/M upon pumping clear non-aerated water. 1760RPM Rating point only is PERF BASED ON STD. MTL'S DATE 1760 2825 E DIVISIO May 2, 1988 LUBBOCK, TEXAS SUPERCEDES July 22,1977 5.3 K (Bal.)= Model RPM. UNTED WATER NEW YORK 75HP BUN Prehada 낖 X EFFICIENCY NPSH REQ'D IN FEET HORSEPOWER PER STAGE 0 S 0 62 TDH STAGES HORSEPOWER L"Tail Pipe SSOCPM (FImpeller) 500 & 30ft CUSTOMER ST. 1 2. 8 400 6"x1"Column & Shaft 3stage 73 900 P.O. DATE SERVICE #8R JHO Lo. 20-707 50 200 00 99# Rowls Nov 2006 Lyrellens PUMP WELL ELMWOOD CAPACITY U.S. GPM CU. METERS Original 1994 0 8 9 S PROJECT TOTAL HEAD - PER STAGE New C8RJHO

MODEL 8RJLO EFFICIENCY CORRECTION Characteristics based Impeller A8154 guaranteed. Column Curve No. 3003 upon pumping clear osses not included. Rating point only is 8RJLO non-aerated water. 3600RPM DATE March 1995 3500 PERF BASED ON STD. MTL'S K= 5.3 LBS/F E PASSO 2625 SUPERCEDES May 2, 1988 **BBOCK, TEXAS** Model: K (Bal.)= HPM KINBKAND WELL DRILLINGCOL, DKC. 14 Waldron Avenue Glen Rock, NJ 09452,2831 % EFFICIENCY HOUSEPOWER NECH BEOLD IN E 20 0 0 ŝ 0 5.T. TOH 380 'TDH STAGES HORSEPOWER 100 230/460V/3PH York 50 Cosmon United Water New 350GPM 009 125 50HP same Curve. 4.68Jmp 4-52-A 500 Column&Shaft 5 stare 00 Model#8RJL0-5Stage stage 400 DUCTILE IRON IMPELLERS would be ON MET 75 x 13/16" 4 68" 300 Despublish Booston Pup 6"x setting 6", Impellers 50 200 Short s Bronze Pump 8 INCURNY NO. Booster GOULDS S.O. NO. 0 FEET 99# METERS PER HR. E] mwood **30ULDS PROPOSAL NO.** METERS 50-40-30-0 20-ਲੋ **JOATS READ - DER STAGE**

C8RJLO.1



► Acknowledgements

A great deal of resources and expertise have contributed to the assessment and implementation of means and methods for increasing supply to meet STWS goals. The effort has been a collaboration of UWNY's engineering and operations staff, monitored by way of weekly progress meetings, and UWNY's management, which has been meeting every two weeks to gage progress. A strong contingent of consulting expertise has been enlisted and working closely with UWNY to support the program.

The following individuals and organizations have provided consistent support for the program and will continue to do so through 2008:

- Bob Raczko, P.E. Senior Engineer; UWNY
- Vito Spadavecchia Production Manager; UWNY Operations
- Michael Joosten Senior Operations Staff; UWNY Operations
- Bill Prehoda, P.G. Hydrogeologist; Leggette, Brashers and Graham, Inc.
- Jonas Hollis, P.G. Hydrogeologist; Leggette, Brashers and Graham, Inc.
- Michael Johnson, P.E. Engineer; Buck, Seifert and Jost, Inc.
- Michael McDonald, P.E. Program Manager; Black & Veatch, Inc.